Prepared for:

# Sapphos Environmental Inc.

430 North Halstead Street Pasadena, California 91107

In support of:

# Southern California Gas Company 555 West 5<sup>th</sup> Street, GT14E7 Los Angeles, California 90013

# PRELIMINARY GEOTECHNICAL STUDY Southern California Gas Company North-South Project San Bernardino and Riverside Counties, California

Prepared by:

# Geosyntec Consultants, Inc.

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Geosyntec Project Number: SC0763

July 2015



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20 July 2015

Ms. Marie Campbell Sapphos Environmental, Inc. 430 North Halstead Street Pasadena, California 91107

Subject: Preliminary Geotechnical Study Southern California Gas Company North-South Project San Bernardino and Riverside Counties, California

Dear Ms. Campbell:

Geosyntec Consultants Inc. (Geosyntec) is pleased to present Sapphos Environmental, Inc. (Sapphos) this report presenting the results of our preliminary geotechnical study report for the Southern California Gas Company (SoCalGas) and San Diego Gas & Electric Company (SDG&E) proposed North-South Project (Proposed Project) located in San Bernardino and Riverside Counties, California.

This report details Geosyntec's review of available information used to evaluate potential geotechnical hazards and geotechnical conditions to support preparation of the Environmental Impact Report/Environmental Impact Statement for the Proposed Project. Our scope included a site reconnaissance, public records search and review of: publically available aerial imagery and geologic information; publically available geotechnical borings performed for projects by others near the Proposed Project alignment; and an environmental assessment and a geologic hazards evaluation performed for the Proposed Project.

Geosyntec appreciates this opportunity to assist Sapphos and SoCalGas with this important project. Please contact us at (858) 674-6559 if you have any questions, comments, or if you need additional information.

Sincerely,

Alexander Greene, C.E.G. Exp. 12-31-10 Senior Engineering Geolog OFCAL

cc: Ron Bott and Jessica Kinnahan, SoCalGas

Jennifer Nevius, G.E. 282. Senior Engineer



Geosyntec Project No. SC0763

engineers | scientists | innovators



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# Geosyntec<sup>></sup>

# LIST OF ACRONYMS AND ABBREVIATIONS

APM	Applicants' Proposed Measures
Applicants	SoCal Gas and SDG&E
Caltrans	California Department of Transportation
CBC	California Building Code
CBSC	California Building Standards Code
CCR	California Code of Regulations
CDMG	California Division of Mines and Geology
CEQA	California Environmental Quality Act
CGS	California Geological Survey
CPUC	California Public Utilities Commission
DOGGR	California Department of Conservation, Division of Oil, Gas & Geothermal Resources
ECSZ	Eastern California Shear Zone
EFZ	Earthquake Fault Zone
EIR	Environmental Impact Report
EIS	Environmental Impact Statement
FEMA	Federal Emergency Management Agency
FIRM	Flood Insurance Rate Maps
ft	feet
g	units of gravity
Geosyntec	Geosyntec Consultants, Inc.
IBC	International Building Code
km	kilometer
LCI	Lettis Consultants International, Inc.
LiDAR	Light Detection and Ranging
MP	Mile Post
m	meter
Μ	Moment Magnitude
mm	millimeter
MSL	Mean Sea Level



NEPA	National Environmental Policy Act
NRCS	Natural Resources Conservation Service
PEA	Proponent's Environmental Assessment
PDCC	Pipeline Design and Construction Corridor
PGA	Peak Ground Acceleration
SBNF	San Bernardino National Forest
SDG&E	San Diego Gas & Electric Company
SEI	Sapphos Environmental, Inc.
SoCalGas	Southern California Gas Company
SWRCB	State Water Resources Control Board
UCERF3	Uniform California Earthquake Rupture Forecast Version 3
USDA	United States Department of Agriculture
USFS	United States Forest Service
USGS	United States Geological Survey
yr	year

# **EXECUTIVE SUMMARY**

Geosyntec Consultants, Inc. (Geosyntec) prepared this Preliminary Geotechnical Study to inform the environmental review process for the Southern California Gas Company (SoCalGas) and San Diego Gas & Electric Company (SDG&E), collectively "Applicants," proposed North-South Project (Proposed Project). The primary components of the Proposed Project include constructing a 36-inch-diameter natural gas transmission pipeline along a 65-mile-long alignment between Adelanto and Moreno Valley, and rebuilding the Adelanto Compressor Station. The Proposed Project is located in San Bernardino and Riverside Counties, California.

This Preliminary Geotechnical Study is based on a thorough review of available information to characterize the geology and soils of the proposed alignment, assess the potential for risk and hazards to people and property from the Proposed Project, and identify design efforts required to remediate risks to people and property to an acceptable level. The review included the "Proponent's Environmental Assessment, North-South Project" (PEA), dated 6 June 2014, prepared by Applicants; a report for the Proposed Project titled "Proposed North-South Pipeline Alignment, Geologic Hazards Evaluation," dated 29 June 2015, prepared by Lettis Consultants International, Inc. (LCI) for SoCalGas and publically available regional and geological information. Indirect and direct public records searches for geotechnical data (specifically boring logs and plans identifying boring locations) along the Proposed Project alignment. This Preliminary Geotechnical Study will be augmented with the results of soil borings along the Proposed Project alignment planned in the summer of 2015.

As detailed by LCI [2015], the Proposed Project alignment crosses several active or potentially active faults strands. The potential for surface rupture associated with a seismic event on either the San Andreas or San Jacinto Fault Zones is considered high, requiring the implementation of mitigation measures (i.e., safety measures and geotechnical investigation and standard project design features detailed herein and in the Applicants' PEA) to avoid or reduce impacts from the Proposed Project.

As detailed by LCI [2015], as a result of its location in a seismically active region, the Proposed Project area would likely experience moderate to severe ground shaking in response to a large-magnitude earthquake occurring on a local or more distant active fault during the expected life of the Proposed Project, thus requiring the preparation of site-specific geotechnical recommendations to inform the design of the proposed pipeline and safety features in the event of seismic ground-shaking.

As detailed by LCI [2015], the Proposed Project alignment has been routed around major landslide hazard areas and is predominantly located within existing utility

corridors, existing roads, and valley bottoms that demonstrate low landslide hazard potential. However, potential landslide hazard conditions exist within the Cajon Pass and Loma Linda Hills area that may result in previously unmapped landslides due to strong seismic shaking, adverse structural conditions (out of slope bedding), or oversaturation of hill slope deposits during years of above-normal wet weather.

As detailed by LCI [2015], approximately 23 miles of the 65-mile Proposed Project alignment are located in areas mapped as having soil liquefaction potential, primarily in the vicinity of the Santa Ana River Basin and Loma Linda in San Bernardino County and Moreno Valley in Riverside County. The potential for liquefaction ranges from low to very high along the Proposed Project alignment, with several miles of the alignment mapped with significant liquefaction hazard requiring site-specific geotechnical recommendations to inform the design of the proposed pipeline, and safety features in the event of seismically induced liquefaction.

Other potential geologic hazards, such as expansive and collapsible soils, subsidence, hazardous materials, volcanism, flooding, tsunamis, seiches, radon gas, and naturally occurring asbestos, were evaluated and considered to have a moderate to very low hazard potential for the Proposed Project.

A site-specific, design-level geotechnical investigation would be conducted prior to final Proposed Project design and include subsurface explorations and laboratory testing at selected locations along the Proposed Project alignment and at other Proposed Project components to collect data for detailed evaluation of potential geologic hazards and geotechnical considerations.

The most significant geologic hazard impacts identified by this Preliminary Geotechnical Study include fault rupture, seismic shaking, landslides, and liquefaction and its secondary effects, with less significant geologic hazard impacts associated with subsidence, flooding, and soil erosion. However, with site-specific investigation and standard project design features, these potential geologic hazards would be reduced to below the level of significance, and the potential for substantially adverse hazards would be mitigated to an acceptable level.

# **1.0 INTRODUCTION**

This report documents findings from a Preliminary Geotechnical Study for the Proposed Project. This report was prepared by Geosyntec under contract to Sapphos Environmental, Inc. (SEI) on behalf of SoCalGas.

The Proposed Project is located in San Bernardino and Riverside Counties, California, and a 13.0-mile pipeline portion within the San Bernardino National Forest (SBNF). The pipeline would be largely located within existing SoCalGas right-of-way or public right-of-way; however, Proposed Project construction would require temporary access roads, staging areas, and work areas that may extend beyond the existing right-of-way. This area, which includes temporary work space, is known as the Pipeline Design and Construction Corridor (PDCC).

# 1.1 <u>Proposed Project Description and Location</u>

The primary components of the Proposed Project include construction and installation of a 36-inch-diameter natural gas transmission pipeline and rebuilding the Adelanto Compressor Station. The Proposed Project also includes installation of additional pressure-limiting equipment at the Moreno, Whitewater, and Shaver Summit Pressure Limiting Stations and upgrades to the existing pressure-limiting equipment at the Desert Center Compressor Station.

As presented on Exhibit 1 (Appendix A), the proposed pipeline extends approximately 65 miles, beginning at the Adelanto Compressor Station in the City of Adelanto and proceeds in a southerly direction through unincorporated San Bernardino County and the City of Victorville. The alignment then runs along Interstate 15 through the Cajon Pass and the SBNF, through urbanized San Bernardino and Moreno Valley, and terminates at the Moreno Pressure Limiting Station in the City of Moreno Valley.

# 1.2 <u>Regulatory Authority</u>

The Proposed Project alignment lies within two counties and seven cities: San Bernardino and Riverside Counties and the Cities of Adelanto, Victorville, San Bernardino, Highland, Colton, Loma Linda, and Moreno Valley. A portion of the Proposed Project alignment is located on lands administered by the SBNF. Table 1 summarizes the length of the Proposed Project pipeline within each jurisdiction. Environmental review for the Proposed Project includes the preparation of an Environmental Impact Report/Environmental Impact Statement (EIR/EIS) in accordance with the California Environmental Quality Act (CEQA) and the National Environmental Policy Act (NEPA). The federal and state lead agencies for the

environmental review process are the SBNF and the California Public Utilities Commission (CPUC), respectively.

To facilitate the evaluation of the Proposed Project, the discussion of the pipeline alignment has been broken into four segments. These alignment segments are based on existing land use characteristics or regulatory jurisdiction. Proposed Project Segment 1 encompasses the portion of the alignment that traverses the high desert area in San Bernardino County; Segment 2 traverses the SBNF; Segment 3 traverses metropolitan San Bernardino; and Segment 4 traverses Riverside County.

# **1.2.1** Segment 1 – High Desert

Segment 1 is an approximately 14.0-mile segment extending from the Adelanto Compressor Station to the SBNF boundary traversing the Cities of Adelanto and Victorville and a portion of unincorporated San Bernardino County. This segment is located between milepost [MP] 0.0 and MP 14.0 of the Proposed Project.

# 1.2.2 Segment 2 – San Bernardino National Forest

Segment 2 is an approximately 13.0-mile segment extending from the northern SBNF administrative boundary to the southern SBNF administrative boundary, including a portion of unincorporated San Bernardino County. This segment is located from MP 14.0 to MP 27.0, approximately 10.2 miles under SBNF jurisdiction and 2.8 miles within unincorporated areas of San Bernardino County.

# **1.2.3** Segment 3 – San Bernardino Urbanized Area

Segment 3 is an approximately 24.3-mile segment, traversing a portion of unincorporated San Bernardino County and the Cities of San Bernardino, Highland, Loma Linda, and Colton. This segment extends from the southern administrative boundary of SBNF to the County of San Bernardino/County of Riverside boundary. This segment is located between MP 27.0 and MP 51.3 of the Proposed Project.

# **1.2.4** Segment 4 – Riverside County

Segment 4 is an approximately 13.7-mile segment of the Proposed Project in Riverside County. For this segment, the Proposed Project alignment extends from Reche Canyon Road at the County of San Bernardino/County of Riverside boundary to the Moreno Pressure Limiting Station. This segment extends through unincorporated Riverside County and the City of Moreno Valley and is located between MP 51.3 and MP 65.0 of the Proposed Project.

# **1.2.5** Other Proposed Project Components

Other components of the Proposed Project include planned upgrades at the Whitewater Pressure Limiting Station, Shaver Summit Pressure Limiting Station, and Desert Center Compressor Station located along the Interstate 10 corridor in unincorporated Riverside County at respective distances of approximately 29, 80, and 101 miles east of the Proposed Project alignment.

Exhibits 1 and 2a through 2d (Appendix A) present the location of the Proposed Project alignment, and Exhibit 2e (Appendix A) presents the location of other Proposed Project components.

# 1.3 <u>Purpose and Scope of Study</u>

The purpose of this Preliminary Geotechnical Study was to prepare a technical report to support preparation of an EIR/EIS. The Geosyntec scope of work included review and compilation of multiple data sources and evaluating available data to identify geologic and soils resources and potential hazards in the Proposed Project area. Data sources utilized to prepare this Preliminary Geotechnical Study report include publically available aerial imagery and geologic information, publically available geotechnical borings performed for projects by others near the Proposed Project alignment, and an environmental assessment and a geologic hazards evaluation performed for the Proposed Project. Design-level geotechnical investigation and preparation of design recommendations were not included and will be performed during later stages of the Proposed Project.

# 1.4 <u>Regulatory Setting</u>

This Preliminary Geotechnical Study was performed for the Proposed Project in consideration of applicable regulatory setting, specifically applicable federal and state regulations, jurisdictional planning documents, and CEQA significance criteria.

# **1.4.1 Federal Regulations**

# International Building Code (IBC)

The IBC is published by the International Code Council and forms the basis for California's building code and other state-specific building codes in the United States. The IBC has been adopted by the California Legislature to address the specific building conditions and structural requirements for California and provide guidance on foundation design and structural engineering for different soil types. The IBC also provides seismic design guidelines for building structures and infrastructure improvements.



# USDA Natural Resources Conservation Service (NRCS)

The NRCS maps soils and farmland uses to provide comprehensive information necessary for understanding, managing, conserving, and sustaining the nation's limited soil resources. In addition to many other natural resource conservation programs, the NRCS manages the Farmland Protection Program, which provides funds to help purchase development rights to keep productive farmland in agricultural uses. Working through existing programs, the USDA joins with state, tribal, and local governments to acquire conservation easements or other interests from landowners.

#### SBNF Land Management Plan

Within the Cajon Pass, the Proposed Project (Segment 2) crosses through the SBNF. The SBNF Land Management Plan provides regulatory guidance for projects within the SBNF, including the Proposed Project. Part 2 of this plan [United States Forest Service (USFS), 2005], titled "San Bernardino National Forest Strategy," contains program strategies and tactics related to geology and soils within the National Forest. Program strategy and tactic AM 2, titled "Forest-wide Inventory," states:

**AM 2 - Forest-wide Inventory.** Develop and maintain the capacity (processes and systems) to provide, store, and analyze the scientific and technical information needed to address agency priorities including...(the identification of) geologic hazards (i.e., seismic activity, landslides, land subsidence, flooding and erosion) through landscape and watershed planning, sediment placement site planning, engineering design, reclamation and maintenance as part of landscape or project assessment.

# **1.4.2 State Regulations**

# CPUC

The CPUC has primary state jurisdiction over the Proposed Project by virtue of its discretionary approval authority over construction, operation, and maintenance of public utility facilities. Because local governments generally do not have discretionary authority over projects within CPUC jurisdiction, such projects are generally exempt from local land use and zoning regulations and permitting. However, as part of the CEQA impact analysis, SoCalGas considered local and state land use plans and policies.

# Alquist-Priolo Earthquake Fault Zoning Act (Alquist-Priolo Act)

The Alquist-Priolo Act provides policies and criteria to assist cities, counties, and state agencies in the development of structures for human occupancy across the trace of active faults. The Alquist-Priolo Act was intended to provide the citizens of the state

with increased safety and to minimize the loss of life during and immediately following earthquakes by facilitating seismic retrofitting to strengthen buildings, including historical buildings, against ground shaking.

The Alquist-Priolo Act requires that special geologic studies be conducted to locate and assess any active fault traces in and around known active fault areas prior to development of structures for human occupancy. This state law was a direct result of the 1971 San Fernando Earthquake, which was associated with extensive surface fault ruptures that damaged numerous homes, commercial buildings, and other structures. The Alquist-Priolo Act's main purpose is to prevent the construction of buildings used for human occupancy on the surface trace of active faults. This Act addresses only the hazard of surface fault rupture and is not directed toward other earthquake hazards.

#### California Building Code (CBC)

The CBC is another name for the body of regulations contained in Title 24, Part 2, of the California Code of Regulations (CCR), which is a portion of the California Building Standards Code (CBSC). Title 24 is assigned to the California Building Standards Commission, which, by law, is responsible for coordinating all building standards [CBC, 2013]. The CBC incorporates by reference the IBC published by the International Code Council [IBC, 2012] with necessary California amendments. The CBC has been tailored for California earthquake conditions.

#### California Department of Transportation (Caltrans) Regulations

Caltrans' jurisdiction includes rights-of-way of state and interstate routes within California. Work within the right-of-way of a federal or state transportation corridor is subject to Caltrans' regulations governing allowable actions and modifications to the right-of-way. Caltrans issues permits to encroach on land within their jurisdiction. The encroachment permit requirement applies to persons, corporations, cities, counties, utilities, and other government agencies. A permit is required for specific activities including opening or excavating a state highway for any purpose, constructing or maintaining road approaches or connections, grading within rights-of-way on any state highway, or planting or tampering with vegetation growing along any state highway. The encroachment permit application requirements relating to geology, seismicity, and soils include information on road cuts, excavation size, engineering and grading cross-sections, hydraulic calculations, and mineral resources approved under the Surface Mining Area Reclamation Act.

# Seismic Hazards Mapping Act

The Seismic Hazards Mapping Act of 1990 addresses non-surface fault rupture earthquake hazards, including liquefaction and seismically induced landslides. The purpose of the Act is to protect the public from the effects of strong ground shaking, liquefaction, landslides, or other ground failure, and other hazards caused by earthquakes. The program and actions mandated by the Seismic Hazards Mapping Act closely resemble those of the Alquist-Priolo Act.

# Southern California Catastrophic Earthquake Preparedness Plan

The Southern California Catastrophic Earthquake Preparedness Plan, adopted in 2008, examines the initial impacts, inventories resources, cares for those wounded and homeless, and develops a long-term recovery process. The process of Long-Term Regional Recovery provides a mechanism for coordinating federal support to state, tribal, regional, and local governments; nongovernmental organizations; and the private sector to enable recovery from long-term consequences of extraordinary disasters. The Long-Term Regional Recovery process accomplishes this by identifying and facilitating the availability and use of recovery funding sources, and providing technical assistance (such as impact analysis) for recovery and recovery planning support. "Long term" refers to the need to reestablish a healthy, functioning region that will sustain itself over time. The Long-Term Regional Recovery's three main focus areas are housing, infrastructure (including transportation), and economic development.

# **1.4.3** County Jurisdictional Regulations

# San Bernardino County

The County of San Bernardino 2007 Developmental Code (as amended 15 January 2015) details the Geologic Hazard Overlay and includes the requirements of Amended County Ordinance 4067, related to the evaluation of geologic hazards. The Geologic Hazard Overlay established by Sections 82.01.020 (Land Use Plan and Land Use Zoning Districts) and 82.01.030 (Overlays) was created to provide greater public safety by establishing investigation requirements for areas subject to potential geologic problems, including active faulting, landsliding, debris flow/mud flow, rockfall, liquefaction, seiche, and adverse soil conditions [County of San Bernardino, 2015]. Proposed Project Segments 1, 2, and 3 are located in San Bernardino County.

# Riverside County

The County of Riverside Code of Ordinances, Title 15, Chapters 15.60 (Earthquake Fault Area Construction Regulations) and 15.80 (Regulating Special Flood Hazard Areas and Implementing the National Flood Insurance Program) regulate construction

within areas with elevated hazards related to earthquakes and flooding [County of Riverside, 2015]. Chapter 15.60 is enforced pursuant to County of Riverside Ordinance 547, which implements the Alquist-Priolo Act, whereas Chapter 15.80 implements Ordinance 458, "An Ordinance of the County of Riverside Regulating Special Flood Hazard Areas and Implementing the National Flood Insurance Program." Proposed Project Segment 4 is located within the County of Riverside.

# **1.4.4 Municipal Regulations**

Proposed Project construction will occur across various municipal jurisdictions including the Cities of Adelanto, Victorville, Highland, San Bernardino, Loma Linda, Colton, and Moreno Valley. Municipal regulations and/or geologic hazard information for these areas are included in various city ordinances, codes, or hazard mitigation plans. Excluding the unincorporated area of Phelan, information was available for each municipality. A summary of these resources includes the following:

- *City of Adelanto*: Adelanto North 2035 Sustainable by Design Technical Report [City of Adelanto, 2012]. Includes a general list of potential hazards for new development in the Adelanto North Planning Area. Relevant hazards included geologic and seismic hazards and flooding hazards. A portion of Segment 1 of the Proposed Project alignment is located in the City of Adelanto.
- *City of Victorville*: Code of Ordinances, Title 16 Developmental Code, Chapter 5 Building and Fire Regulations [City of Victorville, 2014]. These sections include ordinances related to the prevention of loss of life and property resulting from floods/inundation and seismic events. A portion of Segment 1 of the Proposed Project alignment is located in the City of Victorville.
- *City of San Bernardino*: Chapter 15, Buildings and Construction [City of San Bernardino, 2009]. Referred to as the City of San Bernardino Building Code, includes requirements for evaluating and mitigating liquefaction with the focus on reducing property damage and loss of life during earthquake events. A portion of Segment 3 of the Proposed Project alignment is located within the City of San Bernardino.
- *City of Highland*: Municipal Code, Title 15 Building and Construction, and Title 16 Land Use and Development [City of Highland, 2015]. These sections include ordinances related to the adoption of Appendix Chapter M of the CBC, establishing a seismic hazards identification program and establishing standards of construction in all areas of special flood hazard (respectively). A portion of Segment 3 of the Proposed Project alignment is located in the City of Highland.
- *City of Loma Linda*: Local Hazard Mitigation Plan [City of Loma Linda, 2011]. The plan was prepared pursuant to the Stafford Act, as amended by the Disaster

Mitigation Act of 2000. This document includes a hazard assessment for several naturally occurring and man-made hazards. Pertinent hazards discussed include earthquakes, flooding (including inundation), and slope failure. A portion of Segment 3 of the Proposed Project alignment is located within the City of Loma Linda.

- *City of Colton*: Title 15 Buildings and Construction [City of Colton, 2014]. Includes provisions and minimum standards for structural seismic resistance and floodplain management regulations. A portion of Segment 3 of the Proposed Project alignment is located within the City of Colton.
- *City of Moreno Valley*: Local Hazard Mitigation Plan [City of Moreno Valley, 2011]. The plan was prepared pursuant to the requirements of the Robert T. Stafford Disaster Relief and Emergency Assistance Act (Stafford Act), as amended by Section 322 of the Disaster Mitigation Act of 2000 and the 44 Code of Federal Regulations Part 201 Mitigation Planning. The plan provides a summary and risk evaluation of specific naturally occurring and man-made hazards. Pertinent hazards discussed include earthquakes, landslides, flooding, and dam failure/inundation. A portion of Segment 4 of the Proposed Project alignment is located within the City of Moreno Valley.

# 1.4.5 CEQA Significance Criteria

# Section VI – Geology and Soils

The CEQA significance criteria used to evaluate Proposed Project impacts are defined in *Appendix G, Section VI - Geology and Soils*, of the state CEQA Guidelines (Title 14 of the CCR, Section 15000 et seq.). A significant impact would occur if the Proposed Project would:

- a. Expose people or structures to potential substantial adverse effects, including the risk of loss, injury, or death involving:
  - (i) Rupture of a known earthquake fault, as delineated on the most recent Alquist-Priolo Earthquake Fault Zoning Map issued by the State Geologist for the area or based on other substantial evidence of a known fault? Refer to Division of Mines and Geology Special Publication 42.
  - (ii) Strong seismic ground shaking?
  - (iii) Seismic-related ground failure, including liquefaction?
  - (iv) Landslides?
- b. Result in substantial soil erosion or the loss of topsoil?

- c. Be located on a geologic unit or soil that is unstable, or that would become unstable as a result of the project, and potentially result in on- or off-site landslide, lateral spreading, subsidence, liquefaction, or collapse?
- *d.* Be located on expansive soil, as defined in Table 18-1-B of the Uniform Building Code (1994), creating substantial risks to life or property?
- e. Have soils incapable of adequately supporting the use of septic tanks or alternative waste water disposal systems where sewers are not available for the disposal of waste water?

#### Section VIII – Hazards and Hazardous Materials

For the purpose of this report, the applicable CEQA significance criteria used to evaluate Proposed Project impacts are defined in *Appendix G, Section VIII – Hazards and Hazardous Materials*, of the state CEQA Guidelines (Title 14 of the CCR, Section 15000 et seq.). A significant impact would occur if the Proposed Project would:

g. Be located on a site which is included on a list of hazardous materials sites compiled pursuant to Government Code Section 65962.5 and, as a result, would it create a significant hazard to the public or environment?

#### Section IX – Hydrology and Water Quality

For the purpose of this report, the applicable CEQA significance criteria used to evaluate Proposed Project impacts are defined in *Appendix G, Section IX – Hydrology and Water Quality*, of the state CEQA Guidelines (Title 14 of the CCR, Section 15000 et seq.). A significant impact would occur if the Proposed Project would:

- *h. Place within a 100-year flood hazard area structures which would impede or redirect flood flows?*
- *j. Inundation by seiche, tsunami, or mudflow?*

# 2.0 METHODS

This report section outlines the methods used by Geosyntec to conduct this Preliminary Geotechnical Study, including site reconnaissance, review of Proposed Project information, and review of public information.

# 2.1 <u>Site Reconnaissance</u>

Geosyntec and SEI personnel attended a site reconnaissance for the Proposed Project on 30 April 2015 in conjunction with a site meeting conducted by SoCalGas with attendees from CPUC, SBNF, and other members of the SoCalGas team of consultants. After introductions and a safety briefing, attendees met at two locations along the Proposed Project alignment near the intersection of Interstate 15 and Highway 138. Subsequently, the members of the SEI consultant team (including Geosyntec) performed visual site reconnaissance, walking and driving along selected segments of the Proposed Project alignment.

# 2.2 <u>Review of Proposed Project Information</u>

Geosyntec performed a review of information prepared for the Proposed Project to support this Preliminary Geotechnical Study, including the PEA [SoCalGas and SDG&E, 2014] and a focused geologic hazards evaluation [LCI, 2015] as discussed in Section 2.2 herein. Our scope included review of the data presented, identification of data gaps, and assessment of the evaluation methodology and conclusions but did not include performing or checking detailed calculations.

# 2.2.1 PEA

The Applicants prepared a PEA dated 6 June 2014 [SoCalGas and SDG&E, 2014]. This PEA presents an environmental impact assessment for the Proposed Project, including evaluation regarding geology, soils and seismicity. The PEA characterized the level of significance for applicable CEQA criterion and provided a rationale for Applicants' Proposed Measures (APMs) to reduce the identified impacts to less than significant for the Proposed Project. As summarized below and presented in detail in Table 2, the PEA presented the following APMs related to geology, soils, and seismicity, designated APM-GEO-1 through APM-GEO-8.

- APM-GEO-1, Geotechnical investigation;
- APM-GEO-2, Determination of active or potentially active faults;
- APM-GEO-3, Appropriate design ground motion values;
- APM-GEO-4, Appropriate design features to prevent or limit liquefaction;

- APM-GEO-5, Appropriate design features to prevent or limit landslide/slope instability;
- APM-GEO-6, Soil erosion or loss of topsoil;
- APM-GEO-7, Appropriate design features to prevent or limit damage to the pipeline and appurtenant structures on unstable geologic unit or soil; and
- APM-GEO-8, Appropriate design and construction recommendations to prevent or limit expansive material damage to the pipeline and appurtenant structures.

The APMs outlined in the PEA provided mitigation related to site-specific investigations (APM-GEO-1; APM-GEO-2, APM-GEO-6) and appropriate design features (APM-GEO-3, APM-GEO-4, APM-GEO-5, APM-GEO-7, and APM-GEO-8) to mitigate potential hazards relative to geology, soils, and seismicity. Site-specific investigations (including soil borings) to evaluate potential hazards and standard project design features to avoid/limit these hazards would be part of the Proposed Project. Therefore, the APMs from the PEA are not used in this study.

# 2.2.2 Geologic Hazards Evaluation

Under contract to SoCalGas, LCI prepared a report for the Proposed Project titled "Proposed North-South Pipeline Alignment, Geologic Hazards Evaluation," dated 29 June 2015 [LCI, 2015]; a copy of this evaluation report is presented in Appendix B. In general, the LCI report presents the results of a focused geologic hazards evaluation for the Proposed Project pipeline alignment, specifically addressing fault rupture, liquefaction, and landslide hazards. The LCI report did not evaluate hazards associated with other Proposed Project components at the Desert Center Compressor Station and the Whitewater and Shaver Summit Pressure Limiting Stations. LCI performed a data review consisting of compiling and evaluating regional geologic data along the Proposed Project alignment, including:

- Quaternary faults published by the California Geological Survey (CGS);
- Alquist-Priolo Earthquake Fault Hazard Zones (AP-EFZ) and active faults;
- Quaternary faults published by the U.S. Geological Survey (USGS);
- Modeled displacement estimates from ShakeOut;
- Distribution of ShakeOut displacement estimates in the Cajon Pass area;
- Geomorphic offset measurements compiled in Uniform California Earthquake Rupture Forecast, version 3 (UCERF3);

- AP Fault Investigation Reports and subsurface investigations therein;
- Available historic (1930s era) topographic maps;
- Available Dibblee geologic maps;
- Available USGS geologic maps;
- Available CGS geologic maps;
- Available historical (pre-1945) aerial photographs;
- Available Light Detection and Ranging (LiDAR) data; and
- Liquefaction hazards maps.

The LCI report comprises a screening level compilation, review, and interpretation of published information and models for selected geologic hazards, supported by field reconnaissance performed by LCI personnel.

# 2.3 <u>Review of Available Public Information</u>

# 2.3.1 Regional Information

Geosyntec reviewed publically available regional geologic information to support this Preliminary Geotechnical Study. Such information includes, but is not limited to, aerial imagery, geologic maps, hazard maps, and technical publications on geologic hazards pertaining to faulting, landslides, and liquefaction prepared by the CGS, USGS, and others. Geosyntec also reviewed publically available EIR/EIS documents from other large infrastructure projects within the Proposed Project area such as the Calnev Pipeline Expansion Project [Bureau of Land Management, 2012] and the BNSF Cajon Third Main Track Summit to Keenbrook Project [URS Corporation, 2007].

# 2.3.2 Public Records

Geosyntec conducted indirect and direct public records searches for geotechnical data (specifically subsurface exploration logs and plans identifying exploration locations) along the Proposed Project alignment. The search focused on select locations considered to have a higher potential for publically available geotechnical information (i.e., roadway intersections and larger developed commercial/municipal properties as identified from aerial imagery). Geosyntec requested a search of public records from the following jurisdictions and municipalities:

- SBNF;
- Caltrans District 8;



- City of Adelanto;
- City of Colton;
- City of Moreno Valley;
- Phelan, Unincorporated
- City of San Bernardino; and
- County of San Bernardino.

The SBNF provided regional geologic reference information but did not locate publically available project information in close proximity to the Proposed Project alignment. The Caltrans records search was still in progress at the time of this report. Limited information was available as a result of the records search for the selected parcels at the noted municipalities. Two municipalities provided geotechnical information for specified parcels along the Proposed Project alignment; Table 3 presents a summary of these sites and the geotechnical data obtained. Exhibits 2a through 2d (Appendix A) present the site locations for which geotechnical data was obtained by Geosyntec for this study. Copies of selected borings logs and boring location maps, where available, are provided in Appendix C.

Geosyntec also directly searched the State Water Resources Control Board (SWRCB) database for available geotechnical information. GeoTracker is the SWRCB system for managing sites that impact groundwater, especially those that require groundwater cleanup and permitted facilities such as those with operating underground storage tanks and land disposal sites [SWRCB, 2010, 2015]. GeoTracker provides most of the public record for these sites through its Document Manager Module, including regulatory communication with responsible parties, regulatory actions, and data and documents submitted by the responsible party. As such, the SWRCB GeoTracker website (http://geotracker.waterboards.ca.gov/) provides a repository of a variety of environmental data for regulated facilities in California. These data include boring logs, primarily for borings advanced for subsurface characterization, monitoring well installation, or remediation system installation.

Geosyntec identified eleven sites near the Proposed Project alignment where the GeoTracker website provided geotechnical information; Table 4 presents a summary of these sites and the geotechnical data obtained. Exhibits 2a through 2d (Appendix A) present the site locations for which geotechnical data was obtained by Geosyntec for this study. Copies of selected borings logs and boring location maps, where available, are provided in Appendix C. Additional information regarding these sites can be

obtained from the GeoTracker website, searching by site identification criteria (i.e., site name, identification number, or address).

The boring logs presented in Appendix C were performed for other projects by other consultants and are provided for information only. Geosyntec makes no representation regarding the information prepared by others.

# 3.0 RESULTS

This discussion of Preliminary Geotechnical Study results is based on limited site reconnaissance and our review of available information provided by others as discussed herein. At the time of the preparation of this report (June 2015), no site-specific field explorations have been performed for the Proposed Project.

# 3.1 <u>Environmental Setting</u>

# 3.1.1 Topography

As presented on Exhibit 2a through 2d (Appendix A), the Proposed Project pipeline alignment traverses varied and complex terrain from its northernmost point at the Adelanto Compressor Station in the Mojave Desert to the Moreno Pressure Limiting Station in Moreno Valley at its southernmost point.

Segment 1 (MP 0.0 to MP 14.0)

Segment 1 of the Proposed Project alignment encounters low-relief terrain incised by numerous drainages extending to the north from the adjacent topographic highlands. Based on GoogleEarth<sup>TM</sup>, elevations within Segment 1 of the Proposed Project alignment range from 2,962 feet (ft) above Mean Sea Level (MSL) at the Adelanto Compressor Station to 4,083 ft above MSL near MP 13.

Segment 2 (MP 14.0 to MP 27.0)

Segment 2 of the Proposed Project alignment encounters a series of low-lying channels and washes surrounded by the topographic highlands of the San Bernardino and the San Gabriel Mountains. Based on GoogleEarth<sup>™</sup>, elevations within Segment 2 of the Proposed Project alignment range from 4,490 ft above MSL near MP 15 at the SBNF northern boundary to 2,303 ft above MSL near MP 27.

Segment 3 (MP 27.0 to MP 51.3)

Segment 3 of the Proposed Project alignment transverses terrain of moderate to low relief incised by numerous drainages that generally flow to the south within the San Bernardino Valley. Based on GoogleEarth<sup>TM</sup>, elevations within Segment 3 of the Proposed Project alignment range from 2,303 ft above MSL near MP 27 to 1,311 ft above MSL near MP 51.

Segment 4 (MP 51.3 to MP 65.0)

Segment 4 of the Proposed Project alignment transverses through hilly terrain within the Loma Linda Hills and terminates within areas of low relief in the Moreno Valley. Based

on GoogleEarth<sup>TM</sup>, elevations within Segment 4 of the Proposed Project alignment range from 2,446 ft above MSL near MP 56.6 to 1,558 ft above MSL near MP 65 at the Moreno Pressure Limiting Station.

### Other Proposed Project Components

As presented on Exhibit 2e (Appendix A), other Proposed Project components at the existing Whitewater and Shaver Summit Limiting Stations and the Desert Center Compressor Station are located east of the Proposed Project alignment. The three other Proposed Project component sites are located on relatively flat to gently sloping ground that has previously been graded as part of the original project component construction. Based on GoogleEarth<sup>TM</sup>, respective elevations at the Whitewater and Shaver Summit Limiting Stations and the Desert Center Compressor Station are 1,055, 1,690, and 880 feet above MSL, respectively.

#### 3.1.2 Physiographic Setting

The Proposed Project alignment traverses through the Mojave Desert, Transverse Ranges, and Peninsular Ranges physiographic provinces as presented in Exhibit 3 (Appendix A).

#### Segment 1 (MP 0.0 to MP 14.0)

The proposed pipeline originates and continues south through the southern Mojave Desert Province for the initial 12 miles of the alignment. The Mojave Desert Province extends north from the Cajon Pass, north of the San Andreas Fault zone, and eastward to the Nevada state line as presented in Exhibit 3 (Appendix A). The majority of Segment 1 of the Proposed Project alignment (approximately MP 0.0 to MP 12.0) is located within this Province. The Mojave Desert is a broad interior region of Southern California characterized by isolated north-trending mountain ranges separated by broad expanses of alluvial-filled desert plains [CGS, 2002]. The Province is bounded on the southwest by the San Andreas Fault and the Transverse Ranges, on the north and northeast by the Garlock Fault, and to the east by the Basin and Range Province near the California-Nevada state line. The Mojave Desert is a late Tertiary- and Quaternary-aged in-filled basin comprised of mostly aggrading surfaces receiving nonmarine continental deposits from adjacent uplands. Segment 1 of the Proposed Project alignment is situated within the western edge of the Mojave Desert between the City of Adelanto and the SBNF northern boundary.

Between MP 12.0 and MP 14.0, the proposed alignment continues southward into the east-west trending Transverse Range Province at the Cajon Pass between the San Gabriel and San Bernardino Mountains. This Province is an east-west trending

physiographic-structural controlled feature bounded by the Mojave Desert to the north and the Peninsular Ranges and Colorado Desert to the south and southeast as presented in Exhibit 3 (Appendix A). The San Gabriel Mountains are situated in the center of the Transverse Ranges, with the San Bernardino Mountains bounding the eastern portion. Within its eastern extension, the San Bernardino Mountains have been displaced to the south along the San Andreas Fault, and the Province is subjected to intense north-south compression as a result of the left bend on the San Andreas Fault [CGS, 2002].

#### Segment 2 (MP 14.0 to 27.0)

Segment 2 of the Proposed Project alignment extends through the east-west trending Transverse Ranges Province, as described for Segment 1, for approximately 13 miles.

#### Segment 3 (MP 27.0 to 51.3)

The northern portion of Segment 3 between MP 27.0 and MP 38.0 also extends through the east-west trending Transverse Ranges Province, as described for Segment 1, for approximately 11 miles. Between MP 38.0 and MP 51.3, Segment 3 lies within the western flank of the Peninsular Ranges Province, a northwest-oriented physiographic structure of blocks separated by similarly trending faults as presented in Exhibit 3 (Appendix A). The Peninsular Ranges extend south of the San Gabriel and Santa Monica Mountains into Mexico, forming the Baja California peninsula [CGS, 2002]. The Province extends west offshore to the continental margin and east to the western edge of the Salton Trough. The Peninsular Range is a complex mixture of Jurassic and Cretaceous-age igneous and metamorphic rocks associated with the Nevadan plutonism [Walawender, 2000].

#### Segment 4 (MP 51.0 to 65.0)

Segment 4 of the alignment extends through the Peninsular Ranges Province, as described for Segment 3.

#### Other Proposed Project Components

The Shaver Summit Limiting Station and Desert Center Compressor Station are located within the southern portion of the Mojave Desert with comparable physiographic setting to that described for Segment 1.

The Whitewater Limiting Station is located in the southeastern portion of the Transverse Ranges Province within the Coachella Valley, with comparable physiographic setting to that described for Segment 1.

### 3.1.3 Geologic Setting

The Proposed Project pipeline alignment extends through an area of complex geologic structure that lies along the transform boundary between the North American and Pacific continental plates and marks the convergence of several structural blocks bounded by faults. Geological conditions encountered by the alignment are highly varied as a result of complex strike-slip faulting, thrust faulting, and deformation. The surface expression of these complex geologic conditions range from bare bedrock and soil-mantled bedrock in areas of greater relief to unconsolidated to partially lithified alluvial deposits in canyon and valley floors.

# Segment 1 (MP 0.0 to MP 14.0)

This segment is located in the Antelope Valley, a broad, triangular-shaped alluvial valley bounded by the Garlock Fault to the north and the San Andreas Fault to the south. With the immediate vicinity of Segment 1, the Proposed Pipeline alignment is underlain by Quaternary-age alluvial fan and wash deposits. At the southern end of Segment 1, the Proposed Project alignment is bordered by Cretaceous-age granitic crystalline rock in areas of greater relief associated with the San Gabriel Mountains. The mapped geologic units encountered within Segment 1 are presented in Figure 17 of the LCI report (Appendix B).

# Segment 2 (MP 14.0 to 27.0)

This segment is characterized by highly folded and faulted sedimentary rock associated with the active San Andreas Fault. Within the immediate vicinity of Segment 2, the Proposed Project alignment is underlain by geologic units associated with the Mioceneage Cajon Valley Formation and Quaternary-age alluvial fan and wash deposits. Older bedrock units exposed adjacent to the alignment near the San Andreas Fault include Cretaceous-age granitic and metamorphic bedrock. The mapped geologic units encountered within Segment 2 are presented in Figures 17 and 18 of the LCI report (Appendix B).

#### Segment 3 (MP 27.0 to 51.3)

This segment is located adjacent to the foothills of the San Bernardino Mountains and traverses the Santa Ana River drainage. Within the immediate vicinity of Segment 3, the Proposed Project alignment is underlain by geologic units associated with Quaternary-age alluvial fan and wash deposits. The mapped geologic units encountered within Segment 3 are presented in Figure 19 of the LCI report (Appendix B).

### Segment 4 (MP 51.3 to 65.0)

Within the immediate vicinity of Segment 4, the Proposed Project alignment is underlain by Quaternary-age alluvial fan and wash deposits. Cretaceous-age granitic rocks are encountered adjacent to Segment 4 in Reche Canyon within the Loma Linda Hills and are characterized by shallow bedrock or boulders weathered from bedrock. Within the Moreno Valley, the alignment is underlain by Quaternary-age alluvium. The mapped geologic units encountered within Segment 4 are presented in Figure 20 of the LCI report (Appendix B).

#### Whitewater Limiting Station

The Whitewater Limiting Station is located within the western portion of the Coachella Valley. The Coachella Valley is an alluvial valley that extends southeast from the San Gorgonio Pass to the north end of the Salton Sea. This region is traversed by multiple strands of the San Andreas Fault and is underlain by a thick sequence of alluvial sediments being shed off of the surrounding granitic and metamorphic bedrock within the topographic highlands of the San Bernardino Mountains to the north and San Jacinto Mountains to the south. The Whitewater Limiting Station is located along the eastern margin of the Whitewater River drainage and is underlain by Quaternary-age alluvium [California Division of Mines and Geology (CDMG), 1965].

#### Shaver Summit Limiting Station

The Shaver Summit Limiting Station is located just west of Chiriaco Summit within an east-west trending alluvial valley. The site area is bounded to the north by granitic bedrock exposed in the Eagle Mountains and to the south by Cretaceous-age granitic and pre-Cambrian-age metamorphic bedrock exposed in the Orocopia Mountains. The Shaver Summit site is underlain by Quaternary-age alluvium [CDMG, 1967].

#### Desert Center Compressor Station

The Desert Center Compressor Station is located just east of the town of Desert Center along the southwestern flank of the Chuckwalla Valley. To the south, the general site area is bounded by the Chuckwalla Mountains composed of Cretaceous-age granitic and pre-Cambrian-age metamorphic bedrock. The Desert Center site is underlain by Quaternary-age alluvial fan deposits [CDMG, 1967].

# **3.1.4 Geologic Materials**

This discussion of geologic materials was prepared based on information presented by LCI [2015] as presented in the geologic hazards evaluation report for the Proposed

Project (Appendix B). Table 6 [LCI, 2015] summarizes the surficial geologic materials along the Proposed Project alignment.

# Segment 1 (MP 0.0 to MP 14.0)

Segment 1 of the Proposed Project alignment encounters two geologic units both consisting of Quaternary alluvial deposits. Figure 17 in the LCI report [2015] depicts simplified geology after Bedrossian et al. [2012] along Segment 1 of the Proposed Project alignment; Figure 21 [LCI 2015] presents a legend of units depicted on the map. The younger Quaternary alluvial materials make up approximately 86% of materials along Segment 1, while the older Quaternary deposits of the Manzanita Wash make up approximately 14%.

# Segment 2 (MP 14.0 to 27.0)

Segment 2 of the Proposed Project alignment encounters a variety of geologic materials including Quaternary alluvial valley and fan deposits, Tertiary and Quaternary bedrock, granitic and other intrusive crystalline rock, and artificial fill. Figures 17 and 18 in the LCI report [2015] depict simplified geology after Bedrossian et al. [2012] along Segment 2 of the Proposed Project alignment; Figure 21 [LCI 2015] presents a legend of units depicted on the maps. Materials along Segment 2 consist of approximately 87% Quaternary alluvial deposits, 6% coarse-grained Tertiary bedrock, 5% artificial fill, 1% granitic and other intrusive crystalline rock, and 1% Quaternary coarse-grained bedrock.

# Segment 3 (MP 27.0 to 51.3)

Segment 3 of the Proposed Project alignment excavation encounters Quaternary alluvial valley and fan deposits, Tertiary and Quaternary bedrock, granitic and other intrusive crystalline rock, and metamorphic rock of sedimentary and volcanic origin. Figures 18 and 19 in the LCI report [2015] depict simplified geology after Bedrossian et al. [2012] along Segment 3 of the Proposed Project alignment; Figure 21 [LCI 2015] presents a legend of units depicted on the maps. Materials along Segment 3 consist of approximately 87% Quaternary alluvial deposits, 5% Quaternary coarse-grained bedrock, 4% metamorphic rock, 2% coarse-grained Tertiary bedrock, and 2% granitic and other intrusive crystalline rock.

# Segment 4 (MP 51.3 to 65.0)

Segment 4 of the Proposed Project alignment excavation encounters Quaternary alluvial deposits, older Quaternary alluvial fan deposits, Quaternary bedrock, and granitic and other intrusive crystalline rock. Figures 19 and 20 in the LCI report [2015] depict simplified geology after Bedrossian et al. [2012] along Segment 4 of the Proposed Project alignment; Figure 21 [LCI 2015] presents a legend of units depicted on the

maps. Materials along Segment 4 consist of approximately 80% Quaternary alluvial and alluvial fan deposits, 18% granitic and other intrusive crystalline rock, and 2% Quaternary coarse-grained bedrock.

### Other Proposed Project Components

The Whitewater and Shaver Summit Limiting Stations and the Desert Center Compressor Station are underlain by Quaternary alluvial deposits.

#### 3.1.5 Faults and Seismicity

Fault maps, which present the San Andreas Fault system and the Eastern California Shear Zone (ECSZ), are presented on Exhibit 4 (Appendix A) and Figures 1 through 8b of the LCI [2015] report (Appendix B).

#### San Andreas Fault System

Segment 2 of the Proposed Project alignment traverses through the complex neotectonic structure of the active San Andreas Fault System in the Cajon Pass region as presented on Exhibit 1 and 2b (Appendix A). The dominant and most well-known geologic structures are the northwest-oriented right lateral strike-slip fault zones of the San Andreas, San Jacinto, and Elsinore Fault Zones. The right-slip displacements of these fault zones have contemporaneous compressional and uncommon extensional faults, such as the Cucamonga Fault Zone and the left-slip Cleghorn Fault, respectively [Morton and Miller, 2006]. Along Segment 3 of the Proposed Project alignment in the San Bernardino Valley area, the extensional San Bernardino basin, filled by Quaternary-age deposits, occurs between the San Andreas and San Jacinto Fault Zones [Morton and Miller, 2006].

Within the eastern Transverse Ranges Province, the San Andreas Fault Zone is a wellexpressed continuous fault zone with active and older abandoned fault strands. Older fault zones within the Transverse Ranges Province proximal to the Proposed Project considered inactive by Morton and Miller [2006] include the San Gabriel, Punchbowl, and the north branch of the San Bernardino fault segment. However, these fault zones may still be potentially active and/or active according to the Earthquake Fault Zone (EFZ) maps in the Southern California region [Dibblee and Minch, 2003].

The San Bernardino (north and south) segments of the San Andreas Fault, also referred to as the south branch, are considered to be the active section of the San Andreas Fault between the Salton Sea and the Cajon Pass. The Working Group on California Earthquake Probabilities estimated that the southern San Andreas Fault has a 59% probability of an earthquake of at least a moment magnitude (**M**, defined as a measurement of the size of an earthquake in terms of energy released) greater than

**M**6.7 to occur during the 30-year period between 2007 and 2037. Estimated slip rate for the San Andreas Fault system is presented in Section 3.1.5.3.

# ECSZ

The ECSZ is an important component of the Pacific-North American plate boundary. This region of active, predominantly right-lateral strike-slip deformation east of the San Andreas Fault is thought to accommodate approximately 20% to 25% of total relative motion between the Pacific and North American plates [Frankel et al., 2008]. This area of active deformation extends northward from eastern end of the "Big Bend" in the San Andreas Fault near Palm Springs northward through the Mojave Desert and along the east side of Sierra Nevada and into western Nevada. The Mojave Desert portion of ECSZ is bounded to the west by the Helendale-South Lockhart Fault Zone and to the east by the Calico-Hidalgo and Pisgah-Bullion Fault Zones. Fault zones within the ECSZ (west to east) include the northwest striking Helendale-South Lockhart, Lenwood-Lockhart, and Harper-Camp Rock Fault Zones.

The ECSZ is considered a highly seismically active region. Recent work in the Mojave Desert section of the ECSZ indicates the total long-term slip rate across this fault is on the order of 5 to 7 millimeters/year (mm/yr) [Oskin et al., 2006, 2007; and Frankel et al., 2008], suggesting a pronounced strain transient across the Mojave section of the ECSZ. Several faults classified as potentially active (including sections of the Johnson Valley, Homestead, Emerson, and Camp Rock Faults) within the southern ECSZ ruptured during the 1992 M7.3 Landers earthquake [Southern California Earthquake Center (SCEC), 2013]. Rupture along the Lavic Lake Fault and the central Bullion Fault also occurred as a result of the 1999 M7.1 Hector Mine earthquake [SCEC, 2013].

# Regional Historic Events and Recurrence

Several paleoseismic sites have been investigated along the San Andreas Fault Zone proximal to the Proposed Project alignment, including Wrightwood, Pitman Canyon, and Plunge Creek, which are situated approximately 2 to 20 miles from the Cajon Pass crossing area within Segment 2 of the Proposed Project alignment [Weldon et al., 2013]. Previous geologic hazard studies performed at these sites identified 15 separate seismic events at the Wrightwood young sedimentary site, 14 events at the Wrightwood old sedimentary section, 7 events at Pitman Canyon, and 3 events at Plunge Creek site, ranging in age from 2915 BC to AD 1857 based on age dating of stratigraphic offset across the fault zone, as summarized in Table 4 of the LCI report (Appendix B). These studies suggest large surface-rupturing earthquakes occur on the San Andreas Fault at approximately 100- to 200-year intervals. The most recent earthquake includes the 1857 Fort Tejon earthquake which occurred on the north San Bernardino segment of the San

Andreas Fault. Based on the evaluation performed by LCI [2015], which considered these previous paleoseismic studies, an estimated slip rate for the primary strand of the San Andreas Fault was assigned a value of 20 to 30 mm/yr.

Located on the San Jacinto Fault, the Mystic Lake paleoseismic site is situated approximately 2 miles southeast of the Moreno Pressure Limiting Station at the southern end of Segment 4 of the Proposed Project alignment. This site shows evidence of seven earthquakes since AD 579 with a recurrence interval of approximately 181 years [Onderdonk et al., 2013; and Weldon et al., 2013]. Similar to the evaluation performed on the San Andreas Fault Zone, LCI [2015] developed an estimated slip rate for the San Jacinto Fault based on the findings of the paleoseismic studies and assigned a value of 2 to 10 mm/yr for the primary strand of the fault.

Exhibit 4 (Appendix A) presents historic earthquake epicenter map for historical earthquake events within a 100-kilometer radius of the Proposed Project based on data from SCEC [2015].

# 3.2 Geologic Hazards

# 3.2.1 Fault Rupture

Fault rupture hazard was evaluated to assess the exposure to people or structures to substantial adverse effects, including the risk of loss, injury, or death per Appendix G – Section VI Geology and Soils, subsections a(i), of the state CEQA Guidelines. The potential for fault surface rupture is generally considered to be significant along "active" faults (defined as exhibiting surface rupture within the past 11,000 years) and to a lesser degree along "potentially active faults" (exhibiting surface rupture within the past 1.6 million years). As illustrated on Figures 1 through 5b, 8a and 8b prepared by LCI [2015] (Appendix B), a review of published geologic maps and previous geotechnical and geologic hazard investigations identified that Segments 2 and 3 of the Proposed Project alignment cross the San Andreas and San Jacinto Fault Zones, respectively. LCI [2015] identified "preferred" crossings, defined as the most likely fault crossing, based on an evaluation of existing data and additional interpretation. Both the San Andreas and San Jacinto Fault Zones have complex fault traces with multiple strands that have been zoned as active Earthquake Fault Hazard Zones by the State of California's Alquist-Priolo Act. In addition to the San Andreas and San Jacinto Faults, the Proposed Project alignment crosses active or potentially active faults strands associated with the Punchbowl and Cleghorn Faults within the Cajon Pass area of Segment 2, and the Loma Linda Fault within the Loma Linda Hills of Segment 3.

Active faults do not cross Segments 1 and 4 and, therefore, fault rupture hazard is not considered to pose a threat as a result of the Proposed Project within these segments.

Based on a preliminary review of available fault maps, the Shaver Summit Limiting Station and Desert Center Compressor Station are not situated within close proximity of active faults. The Whitewater Limiting Station is located approximately 2 miles south of the San Bernardino Section of the San Andreas Fault and 0.4 miles south of the Garnet Hill Fault. However, the site is not located within an Alquist-Priolo Earthquake Fault Zone. Therefore, fault rupture hazard is not considered to pose a threat as a result of the Proposed Project at these other Proposed Project components.

#### Segment 2 - Fault Rupture Hazard

LCI [2015] presents a detailed evaluation of deterministic fault displacement estimates utilizing multiple approaches and datasets to characterize potential future displacements on the San Andreas Fault Zone and Cleghorn Fault within Segment 2 of the Proposed Project alignment. Estimates of potential displacement were developed by calculating average and maximum displacement for deterministic earthquake scenarios using Wells and Coppersmith [1994], model-derived displacement values from the USGS ShakeOut Scenario for the San Andreas [Jones et al., 2008], observed displacements from past earthquakes compiled and reassessed in UCERF3 [Madden et al., 2013], and strain accumulation based on geologic slip rates versus elapsed time since last rupture. Utilizing an estimated M7.8 value on the San Andreas Fault, best-estimate mean surface displacement near Cajon Pass and the Proposed Project alignment range from 1.0 to 4.4 meters (m) (3.2 to 14.4 ft) with an uncertainty ranging from 0.4 to 5.1 m (1.3 to 16.7 ft) as presented on Figure 7 in Appendix B. Historic displacement data is not available for the Cleghorn Fault; therefore, magnitude-average displacement relationships from Wells and Coppersmith were used for all slip types to estimate potential future displacements on this fault. Based on a rupture area of 401 square kilometers taken from UCERF3, a moment magnitude estimate of M6.6, an average displacement of 0.6 m (2.0 ft) and maximum displacement of 0.9 m (3.0 ft) was estimated for the Cleghorn Fault.

The potential for surface rupture associated with a seismic event on the San Andreas Fault Zone is considered high, requiring the consideration of mitigation measures to avoid or reduce impacts from the Proposed Project. However, with site-specific investigation, standard project design features, and consideration of mitigation measures requiring specialized design features for fault crossings to avoid or reduce impacts, fault rupture hazard would not be likely to represent a significant or substantially adverse hazard, as a result of the Proposed Project.



#### Segment 3 - Fault Rupture Hazard

LCI [2015] presents a detailed evaluation of deterministic fault displacement estimates utilizing multiple approaches and datasets to characterize potential future displacements on the San Jacinto Fault Zone within Segment 3 of the Proposed Project alignment. Estimates of potential displacement were developed by calculating average and maximum displacement for deterministic earthquake scenarios using Wells and Coppersmith [1994], model-derived displacement values from the USGS ShakeOut Scenario for the San Andreas [Jones et al., 2008], observed displacements from past earthquakes compiled and reassessed in UCERF3 [Madden et al., 2013], and strain accumulation based on geologic slip rates versus elapsed time since last rupture. Based on the event histories developed at the Mystic Lake and Hog Lake paleoseismic studies, Rockwell et al. [2014] suggests rupture of the Claremont section of the San Jacinto Fault would result in a M7.1 event. LCI [2015] suggests rupture along the Claremont (northern) and Clark (central) sections of the San Jacinto Fault would produce M7.4 event, providing a maximum displacement of 2.3 and 4.0 m for both scenarios. The potential for surface rupture associated with a seismic event on the San Jacinto Fault Zone is considered high, requiring the consideration of mitigation measures to avoid or reduce impacts from the Proposed Project. However, with site-specific investigation, standard project design features, and consideration of mitigation measures requiring specialized design features for fault crossings to avoid or reduce impacts, fault rupture hazard would not be likely to represent a significant or substantially adverse hazard as a result of the Proposed Project.

#### 3.2.2 Seismic Shaking

# Segments 1 through 4 and Other Proposed Project Components – Seismic Shaking Hazard

Seismic shaking hazard was evaluated to assess the exposure to people or structures to substantial adverse effects, including the risk of loss, injury, or death per *Appendix G* – *Section VI Geology and Soils, subsections a(ii)*, of the state CEQA Guidelines. The study area is situated within a seismically active region, and the entire Proposed Project alignment and the other three Proposed Project component sites would likely experience moderate to severe ground shaking in response to a large-magnitude earthquake during the expected life of the Proposed Project. The potential for significant seismically induced ground shaking in response to an earthquake occurring on a nearby active fault, such as the San Andreas or San Jacinto Fault Zones, or a regional fault, such as the ECSZ, is relatively high within the Proposed Project area. Exhibit 5 (Appendix A) presents a regional map of Peak Ground Acceleration (PGA) with a 2% probability of exceedance in 50 years (2,475-year recurrence interval) as mapped by the USGS

[Peterson et al., 2015]; the estimated PGA values range between 0.6g (units of gravity) and 1.3g, with the highest in the Cajon Pass area. These values are presented for impact assessment only and should not be used for engineering design.

The potential for strong seismic shaking is considered high, requiring the consideration of mitigation measures to avoid or reduce impacts from the Proposed Project. However, with site-specific investigation and standard project design features addressing seismic shaking, this hazard would not be likely to represent a significant or substantially adverse hazard as a result of the Proposed Project.

# 3.2.3 Landslides

Landslide hazard was evaluated to assess the exposure to people or structures to substantial adverse effects, including the risk of loss, injury, or death per *Appendix G* – *Section VI Geology and Soils, subsections a(iv) and c*, of the state CEQA Guidelines. The Proposed Project alignment traverses several areas with the potential for landslide hazards due to the steep topography and the underlying geologic formations susceptible to mass wasting. LCI [2015] compiled landslide hazards from various published maps and supplemented their evaluation with localized mapping of potential landslides. LCI [2015] identified two primary areas along the alignment subject to potential landslide hazards, including Segment 2 within the Cajon Pass in the San Gabriel Mountains and Segment 4 within the Loma Linda Hills near Loma Linda, California, requiring the consideration of mitigation measures to avoid or reduce impacts to people and property from the Proposed Project. Landslides do not pose a threat as a result of the Proposed Project in Segments 1 and 3 and at the three other Proposed Project component sites to the east.

# Segment 2 - Landslide Hazard

Through the Cajon Pass, Segment 2 of the Proposed Project alignment is predominantly located within existing utility corridors, existing roads, and valley bottoms that demonstrate low landslide hazard potential. Near the intersection of Interstate 15 and California Highway 138 in the Cajon Pass area, the Proposed Project alignment crosses a mountainous region with potential landslide hazards as a result of the area's high relief and localized dip-slope conditions on the east and northeast facing slopes. LCI [2015] mapped 25 potential landslides within the Cajon Pass region (as presented in Figure 3 in Appendix B). Many of the mapped landslides are adjacent to the Proposed Project alignment; however, the alignment does not cross these landslides. Landslides located near the Proposed Project alignment and Highway 138 are concentrated in east-to northeast-dipping Tertiary-age coarse-grained sedimentary formations consisting predominantly of sandstone and conglomerate. LCI [2015] also evaluated potential

landslides as mapped by the CGS [Bedrossian et al., 2012], who mapped 27 landslides within 0.5 miles of the Proposed Project alignment, one of which is intersected by the Proposed Project alignment between approximately MP 21.5 and MP 21.8. However, the location of the Proposed Project alignment where it intersects this previously mapped landslide is situated within an area of low topographic relief along the eastern margin of the landslide and therefore does not represent a significant risk.

The Proposed Project alignment has been routed around major landslide hazard areas within Segment 2. However, potential landslide hazard conditions exist in the Cajon Pass area that may result in previously unmapped landslides due to strong seismic shaking, adverse structural conditions (out of slope bedding), or oversaturation of hill slope deposits during years of above-normal wet weather. Therefore, landslides present a potential hazard within Segment 2 of the Proposed Project alignment requiring the consideration of mitigation measures to avoid or reduce impacts from the Proposed Project. However, with site-specific investigation and standard project design features addressing landslides, this hazard would not be likely to represent a significant or substantially adverse hazard as a result of the Proposed Project.

#### Segment 4 - Landslide Hazard

Several landslides are mapped within the northeastern margin of the Loma Linda Hills [Dibblee and Minch, 2003] within Segment 4 of the Proposed Alignment and are likely the result of mass wasting along out of slope dipping bedding planes. However, none of these mapped slides intersect the Proposed Project alignment.

The Proposed Project alignment has been routed around major landslide hazard areas within Segment 4. However, potential landslide hazard conditions exist in the Loma Linda Hills area between approximately MP 51.0 and MP 58.0 that may result in previously unmapped landslides due to strong seismic shaking, adverse structural conditions (out of slope bedding), or oversaturation of hill slope deposits during years of above-normal wet weather. Therefore, landslides present a potential hazard within Segment 4 of the Proposed Project alignment requiring the consideration of mitigation measures to avoid or reduce impacts from the Proposed Project. However, with site-specific investigation and standard project design features addressing landslides, this hazard would not be likely to represent a significant or substantially adverse hazard as a result of the Proposed Project.

#### 3.2.4 Liquefaction and Secondary Effects

Liquefaction, and secondary effects associated with liquefaction, were evaluated to assess the exposure to people or structures to substantial adverse effects, including the risk of loss, injury, or death per *Appendix G* – *Section VI Geology and Soils, subsections* 

a(iii) and c, of the state CEQA Guidelines. Seismically induced soil liquefaction can be described as a significant loss of strength and stiffness due to cyclic pore water pressure generation from seismic shaking or other large cyclic loading. The material types considered most susceptible to classical liquefaction are saturated, loose to medium dense granular soils, and low-plasticity fine grained soils. Although not considered classically liquefiable, clay materials can also exhibit strength loss (cyclic softening) due to seismic shaking. The potential for liquefiable soil and the potential impacts to the Proposed Project from liquefaction and secondary effects, including loss of bearing capacity below foundations, increased lateral and uplift pressures on buried structures, total and differential vertical settlement, cyclic softening of clays, and horizontal movement and instability in areas of sloping ground (landslides and lateral spreading), are not readily quantifiable at this level of study.

Lateral spreading (defined as finite, lateral movement of gently to steeply sloping, saturated soil deposits caused by earthquake-induced liquefaction) is a function of liquefaction susceptibility and localized site conditions including ground slope and distance to a free face. Localized liquefaction-induced slope instability and lateral spreading hazards would likely be adjacent to drainages where slopes are steeper and water accumulates.

In addition to high seismic shaking levels, two other key conditions conducive to liquefaction, shallow groundwater and cohesionless sands, likely exist along Segments 3 and 4 of the Proposed Project alignment. Clay soils subject to potential cyclic softening from earthquake shaking are anticipated to exist in lesser quantities along the Proposed Project alignment. As presented in Appendix B, LCI [2015] performed a preliminary evaluation of liquefaction hazard potential and an analysis of liquefaction-induced settlement for the Proposed Project. The LCI [2015] evaluation concluded that the expected level of seismic shaking in the Proposed Project area is high enough to initiate liquefaction, requiring the consideration of mitigation measures to avoid or reduce impacts from the Proposed Project. Figure 15 in the LCI [2015] report illustrates areas with mapped liquefaction potential. Due to the anticipated lack of shallow groundwater, the potential for liquefaction does not pose a threat as a result of the Proposed Project within Segments 1 and 2 of the Proposed Project alignment.

#### Segment 3 - Liquefaction Hazard

As illustrated on Figure 15 of the LCI [2015] report, mapped liquefaction hazard exists along approximately 12 miles of Segment 3 of the Proposed Project alignment primarily in the Cajon Wash, within the Santa Ana River basin, and to a lesser extent below the drainage of the San Bernardino. LCI [2015] estimated that the magnitude of liquefaction-induced settlement could range between 0 and 4.3 inches based on

published regional geotechnical exploration data in the San Bernardino Valley, requiring the consideration of mitigation measures to avoid or reduce impacts from the Proposed Project. However, with site-specific investigation and standard project design features addressing liquefaction, this hazard would not be likely to represent a significant or substantially adverse hazard as a result of the Proposed Project.

#### Segment 4 - Liquefaction Hazard

As illustrated on Figure 15 of the LCI [2015] report, mapped liquefaction hazard exists along approximately 11 miles of Segment 4 of the Proposed Project alignment primarily in the Moreno Valley, and to a lesser extent within the localized alluvial drainages in the Loma Linda Hills. The potential for liquefaction ranges from low to moderate within Segment 4 of the Proposed Project alignment. Due to the similar geologic setting of the area, LCI [2015] estimated that like Segment 3, the magnitude of liquefaction-induced settlement could range between 0 and 4.3 inches based on published regional geotechnical exploration data, requiring the consideration of mitigation measures to avoid or reduce impacts to from the Proposed Project. However, with site-specific investigation and standard project design features addressing liquefaction, this hazard would not be likely to represent a significant or substantially adverse hazard as a result of the Proposed Project along Segment 4.

#### Other Proposed Project Components

Liquefaction hazard may exist at the Whitewater and Shaver Summit Limiting Stations and at the Desert Center Compressor Station given the loosely consolidated alluvial material and susceptibility to strong seismic ground shaking from nearby seismic sources, but groundwater depths at these sites were not available. However, with sitespecific investigation and standard project design features addressing liquefaction, this hazard, if encountered, would not be likely to represent a significant or substantially adverse hazard as a result of the Proposed Project at the other Proposed Project component sites.

#### 3.2.5 Expansive and Collapsible Soils

Expansive and collapsible soils were evaluated to assess the exposure to people or structures to substantial adverse effects, including the risk of loss, injury, or death per *Appendix G – Section VI Geology and Soils, subsection d*, of the state CEQA Guidelines. Some soils and bedrock formations that contain clay minerals are susceptible to expansion under wetting conditions and contraction under drying conditions. Depending on the type and amount of clay present in a geologic deposit, the volume change from expansion and contraction (swell and shrink) can damage pipelines, slabs, foundations, and hardscape. Collapse (also called hydrocompaction)

can occur in dry soils that have an unstable soil structure due to deposition processes, typically with a skeletal structure that is weakly cemented by soluble salts or clay. Increases in moisture can cause the inter-particle cementation to reduce, causing changes in volume (collapse) especially when loaded.

Expansive soils are not mapped at the regional level within Segments 1 through 4 of the Proposed Project alignment or at the Whitewater, Shaver, or Desert Center sites. Expansive soils are not anticipated to require the consideration of mitigation measures to avoid or reduce impacts to people and property from the Proposed Project. This is due to the majority of the Proposed Project alignment and associated Proposed Project component sites being underlain by granular alluvial fan, wash, and sedimentary deposits. Potential impacts from expansive soils are considered a more significant hazard to rigid structures at or near the ground surface (i.e., features at the Adelanto and Desert Center Compressor Stations and Moreno, Whitewater, and Shaver Summit Pressure Limiting Stations). However, with site-specific investigation and standard project design features addressing expansive soil, this hazard, if encountered, would not be likely to represent a significant or substantially adverse hazard as a result of the Proposed Project alignment or associated Proposed Project components.

The potential for collapsible soil is not readily quantifiable at the current level of study. However, with site-specific investigation and standard project design features addressing collapsible soil, this hazard, if encountered, would not be likely to represent a significant or substantially adverse hazard as a result of the Proposed Project along the Proposed Project alignment or other Proposed Project components.

#### 3.2.6 Regional Subsidence

Regional subsidence was evaluated to assess the exposure to people or structures to substantial adverse effects, including the risk of loss, injury, or death per *Appendix G* – *Section VI Geology and Soils, subsection c*, of the state CEQA Guidelines. Subsidence is the gradual settling of the ground surface with little to no horizontal movement and can be caused by many factors such as fluid (i.e., oil or groundwater) extraction, mining operations, or karst terrain. Within Southern California, extraction of large fluid volumes (such as water, oil, or gas) from thick layers of poorly consolidated sediments is the principal cause of subsidence. The potential for subsidence due to karst, pseudo karst, or mining features is considered very low in relation to the geologic setting and absence of large or commercial subsurface mining within the Proposed Project area.

Subsidence hazard is considered very low within Segment 2 of the Proposed Project alignment through the Cajon Pass area due to the presence of relatively shallow bedrock and formational deposits. Along Segment 4, a review of Riverside County mapped areas

susceptible to subsidence and areas of documented subsidence (primarily young alluvial deposits) suggest that no portion of the Proposed Project alignment traverses these areas of mapped subsidence [County of Riverside, 2014]. Based on this information, the potential for regional subsidence does not pose a threat, as a result of the Proposed Project, within Segments 2 and 4 of the Proposed Project alignment. Given the geologic conditions and remote and relatively undeveloped nature of the other Proposed Project component sites at the Whitewater and Shaver Summit Limiting Stations and the Desert Center Compressor Station, the presence of other factors which typically result in subsidence are not anticipated. Therefore, subsidence hazard does not pose a threat as a result of the Proposed Project at the other Proposed Project component sites.

#### Segment 1 - Subsidence Hazard

Within Antelope Valley north of the Cajon Pass, significant fluctuations of groundwater have been documented within the Mojave River groundwater basin due to domestic, agricultural, and municipal water consumption between 1992 and 2002 [USGS, 2015a]. Although significant subsidence has not been documented, increased demand on local water supplies has resulted in overdraft conditions within the Mojave River groundwater basin, which may lead to land subsidence. Subsidence hazard due to groundwater extraction is considered low to medium within Segment 1 of the Proposed Project alignment, which crosses through the Mojave River groundwater basin, requiring the consideration of mitigation measures to avoid or reduce impacts to people and property from the Proposed Project. However, with site-specific investigation and standard project design features addressing subsidence, this hazard would not be likely to represent a significant or substantially adverse hazard to people and property as a result of the Proposed Project.

#### Segment 3 - Subsidence Hazard

Within the Bunker Hill groundwater basin in San Bernardino County, groundwater elevations in production wells were noted to have dropped as much as 70 feet locally during the period from fall of 2012 to 2013 due to recent drought conditions [San Bernardino Valley Water Conservation District (SBVWCD), 2014]. Although subsidence within the Bunker Hill groundwater basin has not been documented recently, historic subsidence of up to a foot may have occurred within the City of San Bernardino as result of groundwater pumping. However, within the San Bernardino area, the potential for subsidence has been significantly reduced since 1972, when the San Bernardino Municipal Water District began to maintain groundwater levels to allow recharge of the underlying alluvial deposits. If this current practice were to change due to increased demands on water during drought conditions, such groundwater fluctuations may once again lead to localized subsidence. Subsidence hazard due to groundwater extraction is considered low to medium within Segment 3 of the Proposed

Project alignment, which crosses through the Bunker Hill groundwater basin, requiring the consideration of mitigation measures to avoid or reduce impacts to people and property from the Proposed Project. However, with site-specific investigation and standard project design features addressing subsidence, this hazard would not be likely to represent a significant or substantially adverse hazard as a result of the Proposed Project.

#### 3.2.7 Hazardous Materials

Hazardous materials sites were evaluated to assess the impact to the public or environment per Appendix G – Section VIII Hazards and Hazardous Materials, subsection g, of the state CEQA Guidelines. Although evaluation of hazardous materials is not covered under Section VI – Geology and Soils of the state CEQA Guidelines, naturally occurring hazardous materials as a result of regional geologic conditions are present within the Proposed Project area. Segments 1 through 4 of the Proposed Project alignment do not traverse areas of hazardous materials including methane gas, hydrogen-sulfide gas, and tar seeps [USGS, 2011]. Based on available data from the California Department of Conservation, Division of Oil, Gas & Geothermal Resources (DOGGR) [2014], only one active well was identified within 300 ft of the Proposed Project centerline. Based on a review of well construction log for this location, the constructed well depth exceeds 5,000 ft below the ground surface. Assuming that the constructed well depth coincides with the most productive oil/gas bearing zones, it is unlikely that Proposed Project construction would encounter hazardous materials associated with these operations. However, monitoring for hazardous materials should be performed during pipeline construction within 300 ft of the active and inactive wells for worker safety and potential impacts to sensitive receptors, specifically within excavations or other confined spaces. Although not anticipated to be encountered along the Proposed Project alignment, with site-specific investigation and standard projectdesign features addressing hazardous materials, this hazard would not be likely to represent a significant or substantially adverse hazard as a result of the Proposed Project.

Hazardous materials are not anticipated to be encountered at Proposed Project components for the Whitewater and Shaver Summit Limiting Stations and Desert Center Compressor Station due to the geologic conditions and remote and relatively undeveloped nature of those site areas. Therefore, hazardous materials would not be likely to represent a significant or substantially adverse hazard as a result of the Proposed Project at the other Proposed Project component locations.

#### 3.2.8 Volcanic Eruptions

Based on the geographic setting of the Proposed Project alignment and other Proposed Project components, the distance to active or other non-active volcanoes, and review of the Volcano Hazards Program maps and current activity alerts [USGS, 2015b], the potential for volcanic eruptions within the vicinity of the Proposed Project is very low. Therefore volcanic eruption would not be likely to represent a significant or substantially adverse hazard as a result of the Proposed Project.

#### 3.2.9 Flooding

Flooding was evaluated to assess the Proposed Project impacts to flood hazard areas per Appendix G – Section IX Hydrology and Water Quality, subsection g and h, of the state CEQA Guidelines. Additional evaluation of flood hazard will be described in a separate specific Hydrology Report, including a scour study. At stream and river crossings with the potential for scour erosion as a result of seasonal flooding or other causes, the pipeline would be protected by deep burial. The burial depth will be based on the results of the hydrologic analyses. The Federal Emergency Management Agency (FEMA) presents flood hazard evaluation as part of their Flood Insurance Rate Maps (FIRM). As presented on Exhibit 6 in Appendix A, the FIRM maps indicate that localized portions of the Proposed Project alignment traverse or parallel special flood hazard areas subject to inundation by flooding with a 1 percent probability of occurrence in a given year (100-year recurrence interval). Segments 1, 2, and 4, in addition to the Whitewater and Shaver Summit Limiting Station and Desert Center Compressor Station, are not located within a mapped special flood hazard area. Therefore, with standard project design features addressing flooding and scour potential, flooding would not be likely to represent a significant or substantially adverse hazard as a result of the Proposed Project.

#### Segment 3 – Flood Hazard

Based on a review of available FIRM maps, Segment 3 of the Proposed Project crosses special flood hazards areas within the Old Waterman Canyon drainage (MP 38) and the Santa Ana River drainage (MP 45). The Proposed Project also parallels special flood hazard areas within the Cajon Creek drainage (MP 28 to MP 32). However, considering the planned pipeline burial depths, the potential for flooding to adversely affect the Proposed Project, or result in impacts to impede or redirect flood flows, is very low. With standard project design features addressing the potential for flooding and scour, flooding would not be likely to represent a significant or substantially adverse hazard as a result of the Proposed Project.

#### 3.2.10 Tsunami and Seiche Inundation

Tsunami and seiche inundation was evaluated to assess the exposure to people or structures to substantial adverse effects, including the risk of loss, injury, or death per *Appendix G – Section IX Hydrology and Water Quality, subsection j*, of the state CEQA Guidelines. Additional evaluation of tsunami and seiche inundation hazard will be described in a separate specific Hydrology Report. Based on the physiographic setting of the study area, the distance to the ocean, the alignment elevation, and review of California Tsunami Inundation Maps [State of California, 2009], the potential for flooding from seismically induced tsunamis is very low.

The closest large water bodies, Silverwood Lake and Lake Perris, are located several miles from the Proposed Project alignment as illustrated on Exhibit 1 (Appendix A). Silverwood Lake is situated approximately 6 miles east of the Proposed Project alignment near Cajon Pass and approximately 8 miles north of the Proposed Project alignment in San Bernardino. Lake Perris is situated approximately 3.5 miles southwest of the Moreno Pressure Limiting Station. Topographic highlands within the San Bernardino Mountains separate the Proposed Project alignment from Silverwood Lake and Lake Perris. The potential for a seismically induced seiche to adversely affect, or result in impacts to, the Proposed Project is very low. Therefore, inundation resulting from a tsunami or seiche would not be likely to represent a significant or substantially adverse hazard as a result of the Proposed Project.

#### 3.2.11 Radon-222 Gas

Radon-222 gas is radioactive, colorless, and odorless and forms from the radioactive decay of small amounts of naturally occurring uranium and thorium present in rocks and soil. Certain rock types common in California, such as black (organic-rich) shales, and some granitic rocks, and rhyolites can have uranium and thorium in amounts higher than is typical of the earth's crust. Breathing air with a concentrated level of radon gas can result in an increased risk of developing lung cancer. Radon-222 is the isotope of most concern to public health because it has a much longer half-life (3.8 days) than other radon isotopes [CGS, 2013]. The longer half-life allows randon-222 to migrate further through rock and soil through micro-fractures and through pore-spaces between mineral grains. Radon gas moves from the soil into buildings and enters cracks in building slabs or basement walls and concentrate in a building indoor air. Based on a review of mapped radon zones in California [Environmental Protection Agency (EPA), 2013], the risk of naturally occurring randon-222 gas along the Proposed Project alignment and at the other Proposed Project component sites is considered to be very low. Therefore, potential hazards resulting from Radon-222 would not be likely to

represent a significant or substantially adverse hazard as a result of the Proposed Project.

#### 3.2.12 Naturally Occurring Asbestos

Chrysotile and amphibole asbestos (such as tremolite) occur naturally in certain geologic units in California, most commonly associated with ultramafic rocks along associated faults [CGS, 2000]. Asbestos is a well-known carcinogen, and inhalation can result in the development of lung cancer or mesothelioma. Based on a review of areas likely to contain ultramafic rocks [CGS, 2000], natural occurrences of asbestos are not anticipated to be present along the Proposed Project alignment or at the other Proposed Project component sites. Therefore, potential hazards resulting from naturally occurring asbestos would not be likely to represent a significant or substantially adverse hazard as a result of the Proposed Project.

#### 3.3 <u>Geotechnical Considerations</u>

#### **3.3.1** Construction Blasting

Based on the anticipated geologic conditions and construction depths along the Proposed Project alignment, construction blasting is not anticipated to be required to install the majority of the pipeline. Construction blasting may be required in limited areas to remove boulders or excavate trenches for pipeline construction in unweathered rock at the ground surface. If required, blasting would be planned in accordance with applicable regulations and would be performed during daylight hours. A blast plan would be developed to address specifications for the following items: use of explosives; blasting; notification; transportation of blasting material; methods for limiting ground vibrations, air-overpressure levels, records requirements, and safety and warning programs; and vibrations predictions based on project parameters. Additionally, the U.S. Department of Labor's Occupational Safety and Health Administration has detailed safety requirements that would be followed for each blasting event to ensure worker safety. In addition to the site safety (existing site improvements and public and worker safety), the effects of construction blasting on subsurface aquifers would be evaluated prior to construction blasting for the Proposed Project. The distance from the construction blasting to the existing aquifer, the nature of the rock to be blasted, and the potential for soil or subsurface water impacts would be considered prior to construction blasting. However, with site-specific investigation and standard project design features, construction blasting, if required, would not be likely to represent a significant or substantially adverse hazard as a result of the Proposed Project.

#### 3.3.2 Soil Erosion and Loss of Topsoil

The Proposed Project alignment generally traverses areas that have already been disturbed, such as roadways or existing utility corridors. However, temporary disturbance of soils would occur as a result of construction activities, making soil erosion, as a result of wind and water exposure, a potential hazard that would require the consideration of mitigation measures to avoid or reduce impacts from the Proposed Project. However, with site-specific investigation and standard project design features, soil erosion and loss of topsoil would not be likely to represent a significant or substantially adverse hazard as a result of the Proposed Project.

Furthermore, standard erosion and sediment control Best Management Practices as required by a project-specific Storm Water Pollution Prevention Plan per the California Construction General Permit (CGP - Order No. 2009-009-DWQ) would prevent soils disturbance leading to erosion.

#### 3.4 <u>Soils Incapable of Adequately Supporting Septic Tanks or Alternative</u> <u>Waste Water Disposal Systems</u>

The Proposed Project would not require the use of septic tanks or alternative waste water disposal systems.

#### 3.5 <u>Conclusions</u>

As part of a design-level geotechnical investigation and exploration program, subsurface explorations and laboratory testing would be performed at selected locations along the Proposed Project alignment to collect site-specific data for more refined evaluation of potential geologic hazards and geotechnical considerations and to support development of associated design recommendations.

The most significant potential geologic hazard impacts identified are fault rupture, seismic shaking, landslides, and liquefaction and its secondary effects. Less significant potential geologic hazard impacts are associated with subsidence, flooding related to scour, and soil erosion. However, with site-specific investigation, evaluation, standard project design features addressing potential geologic hazards and geotechnical considerations, and specialized design for fault rupture, the identified geologic hazards would not represent a significant or substantially adverse hazard as a result of construction and operation of the Proposed Project.

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## **TABLES**

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#### **Table 1. Affected Jurisdictions**

Segment	Affected Jurisdiction	Length <sup>a</sup> (miles)
	City of Adelanto	2.2
Segment 1: High Desert (MP 0.0-14.0)	City of Victorville	2.0
(111 0.0-14.0)	Unincorporated San Bernardino County	9.8
	Subtotal Segment 1	14.0
Segment 2: San Bernardino National Forest <sup>b</sup>	San Bernardino National Forest	10.2
(MP 14.0-27.0)	Unincorporated San Bernardino County <sup>c</sup>	2.8
	Subtotal Segment 2	13.0
	City of San Bernardino	14.7
	City of Highland <sup>d</sup>	0.0
Segment 3: San Bernardino Urbanized Area (MP 27.0-51.3)	City of Loma Linda	0.8
	City of Colton	2.2
	Unincorporated San Bernardino County	6.6
	Subtotal Segment 3	24.3
Segment 4: Riverside County	City of Moreno Valley	6.8
(MP 51.3-65.0)	Unincorporated Riverside County	6.9
	Subtotal Segment 4	13.7
	TOTAL	65.0

Notes:

- a. Miles are approximate and rounded to the nearest tenth of a mile.
- b. Segment 2 covers the 13.0-mile portion of the Proposed Project located within the administrative boundary of the San Bernardino National Forest (SBNF), which includes unincorporated territory of San Bernardino County.
- c. Within unincorporated San Bernardino County, the limits of the SBNF extend approximately 13 miles; however, due to private holdings within the SBNF, only 10.2 miles are under the jurisdiction of the SBNF.
- d. The Proposed Project is within the City of Highland for approximately 0.04 mile, which, when rounded, is less than 0.1 mile. However, 1.3 miles of the alignment abuts the City of Highland boundary.

APM No.	Applicants' Proposed Measures					
APM-GEO-1	<b>Geotechnical Investigation.</b> One or more project-specific geotechnical investigations conducted under the most current state and county guidelines will be completed by a California-licensed geotechnical engineer and California-certified engineering geologist. The investigation will address the Proposed Project design to minimize effects from: adverse soil conditions including any liquefiable or otherwise unstable/consolidation-prone soils; bedrock characteristics; subsidence; earthquake ground shaking; slope instability; subsurface gas; groundwater; fault rupture; and/or other geotechnical and engineering geologic hazards. The design and construction recommendations will be incorporated in to the foundation, structural, and pipeline design of Proposed Project components, implemented in accordance with the design, and subjected to inspection by the relevant entities/agencies. Grading/building inspectors would perform site inspections to assure construction occurs in accordance with any building permits and plans.					
APM-GEO-2	<b>Fault Rupture.</b> It will be necessary to determine each location where the pipeline crosses an active or potentially active fault. For each fault crossing location, determination will be made as to the estimate fault rupture characteristics, such as movement direction and amount, likely intervals between movements, and the width of the zone that may experience movement. Design recommendations will be incorporated to establish block valve locations.					
APM-GEO-3	<b>Strong Seismic Ground Shaking.</b> The geotechnical investigation required by APM-GEO-1 will provide appropriate design ground motion values that will assist in the design to prevent or limit damage during earthquake events that may impact specific sections of the pipeline, compressor station, and appurtenant structures.					
APM-GEO-4	Seismic-Related Ground Failure, Including Liquefaction. The engineering geologic investigations and reports will map the locations and define the nature of any areas that may experience seismic-related ground failure, including liquefaction. Reports will provide appropriate hazard information that will assist in the design to prevent or limit liquefaction damage to the pipeline, compressor station, and appurtenant structures during earthquake events.					
APM-GEO-5	Landslides. The engineering geologic and geotechnical investigations will map the locations and define the nature of any landslides or landslide-prone areas that could impact the Proposed Project. Reports will provide appropriate hazard information to prevent or limit landslide/slope instability damage to the pipeline.					

#### Table 2. Applicants' Proposed Measures Related to Geology, Soils, and Seismicity

## Geosyntec<sup>▷</sup>

consultants

#### Table 2. Applicants' Proposed Measures Related to Geology, Soils, and Seismicity (Continued)

APM No.	Applicant's Proposed Measures
APM-GEO-6	<b>Soil Erosion or Loss of Topsoil.</b> The geotechnical investigations will evaluate the erosion characteristics of soils and geologic formations/sub-units along the length of the Proposed Project. Reports will provide appropriate construction, design, and operational measures to prevent or limit surface erosion due to the pipeline construction and due to runoff from conditions near the Proposed Project.
APM-GEO-7	<b>Unstable Geologic Unit or Soil – Off-site Landslide, Lateral Spreading, Subsidence Liquefaction or Collapse</b> . The engineering geologic and geotechnical investigations will map the locations and define the nature of any areas of unstable geologic units or soils that could affect off-site areas or be affected by off-site areas. Considerations include, but are not limited to, landslides, lateral spreads, subsidence, and soil collapse. Reports will provide appropriate design measures to prevent or limit damage to the pipeline and appurtenant structures due to these conditions.
APM-GEO-8	<b>Expansive Soil.</b> The engineering geologic and geotechnical investigations will define formations that contain sufficient clay materials to be considered sufficiently expansive to affect the pipeline and appurtenant facilities. These formations shall be mapped and analyzed (sampled and tested) to determine the degree of expansion that may be expected. Reports shall provide appropriate design and construction measure to prevent or limit expansive material damage to the pipeline and appurtenant structures.

#### Table 3. Summary of Geotechnical Data Obtained from Municipal Sources

Approximate Mile Post <sup>a</sup>	Site Name <sup>b</sup>	Site Address	Year(s) Drilled	Number of Available Boring Logs/Test Pits	Investigator	Exploration Identification
1.0	Adelanto Industrial Park III	Industrial Way at Koala Road (multiple addresses)	1989	18	American Engineering Laboratories, Inc.	Test Pit No. 1 through Test Pit No. 18
62	Residential Development Tract Nos. 31268 and 31269	Redlands Boulevard and Cottonwood Avenue (multiple addresses)	2003	12	Petra Geotechnical, Inc.	Borings B-1 through B-12

Notes:

- a. Approximate Proposed Project mile post from GIS data provided by SoCalGas.
- b. Excerpts, including site plans and boring/well logs retrieved from GeoTracker website presented in Appendix C1.

#### Table 4. Summary of Geotechnical Data Obtained from SWRCB GeoTracker Website

Approximate Mile Post <sup>a</sup>	Site Name <sup>b</sup>	GeoTracker Case Identification <sup>c</sup>	Site Address	Year(s) Drilled	Number of Available Boring Logs	Investigator	
18.8	Circle K Station 5961	T0607151980	8324 Highway 138, Phelan 92371	2007	5	TRC	C
29.0	Glen Helen Regional Park	T0607100575	2555 Glen Helen Pkwy., San Bernardino	2004, 2007	2	Geo-Cal, Inc.	N
29.0	Maintenance Facility	1060/1005/5	92407	2009	1	ERRG	N
41.8	Mahila #19 Starling Aug	T0607100246	25600 E Deceline St. Highland 02410	1992, 1996	19	Irwin Environmental	B B
41.8	Mobile #18 Sterling Ave T0607100246 25		25699 E Baseline St., Highland 92410	1997, 1998, 2001, 2002, 2005, 2009	15	Kleinfelder	В 8,
43.2	Iskandar Texaco	T0607100550	24914 E 5th St., San Bernardino 92410	2008	8	Ami Adini & Associates, Inc.	s
				1999, 2000	9	Converse Consultants	В
46.5	Bear Oil Co./ Former Texaco	Texaco T0607100598	T0607100598 24913 Redlands Blvd., Loma Linda 92354	2003	3	Kleinfelder	В
46.2	Former M&M Smog and Muffler	T1000003588	1915 East Tippecanoe Ave., San Bernardino 92410	2011, 2012, 2013	18	Stantec	G 1. to
46.3	Equilon Enterprises/ Shell	T0607100504	1973 Tippecanoe Ave., San Bernardino 92408	2005	4	Miller Brooks Environmental, Inc.	C
46.5	Unocal #2417	T0607100008	24891 W Redland Blvd., Loma Linda 92408	2009	3	Conestoa-Rovers & Associates, Inc.	С
				2005, 2007	10	TRC	S
47.2	ARCO #5214	T0607100180	305 Redlands Blvd., San Bernardino 92408	2007	3	Stratus Environmental, Inc.	C
47.8	Eric Realty Property	T10000001230	495 Commercial Road East, San Bernardino 92408	2002, 2003, 2004, 2008	15	Advanced GeoEnvironmental, Inc.	P M
48.1	Food N Fuel	T0607100528	2649 S. Waterman Ave., San Bernardino 92409	2001, 2004, 2005, 2006, 2008, 2012	21	Alta EM, Inc.	B M

Notes:

a. Approximate Proposed Project mile post from GIS data provided by SoCalGas.

b. Excerpts, including site plans and boring/well logs retrieved from GeoTracker website are presented in Appendix C2.

c. Site location and identification information from the SWRCB GeoTracker website (<u>http://geotracker.waterboards.ca.gov/</u>).

d. Data summarized represents geotechnical information retrieved from SWRCB GeoTracker website for selected locations near the Proposed Project alignment.

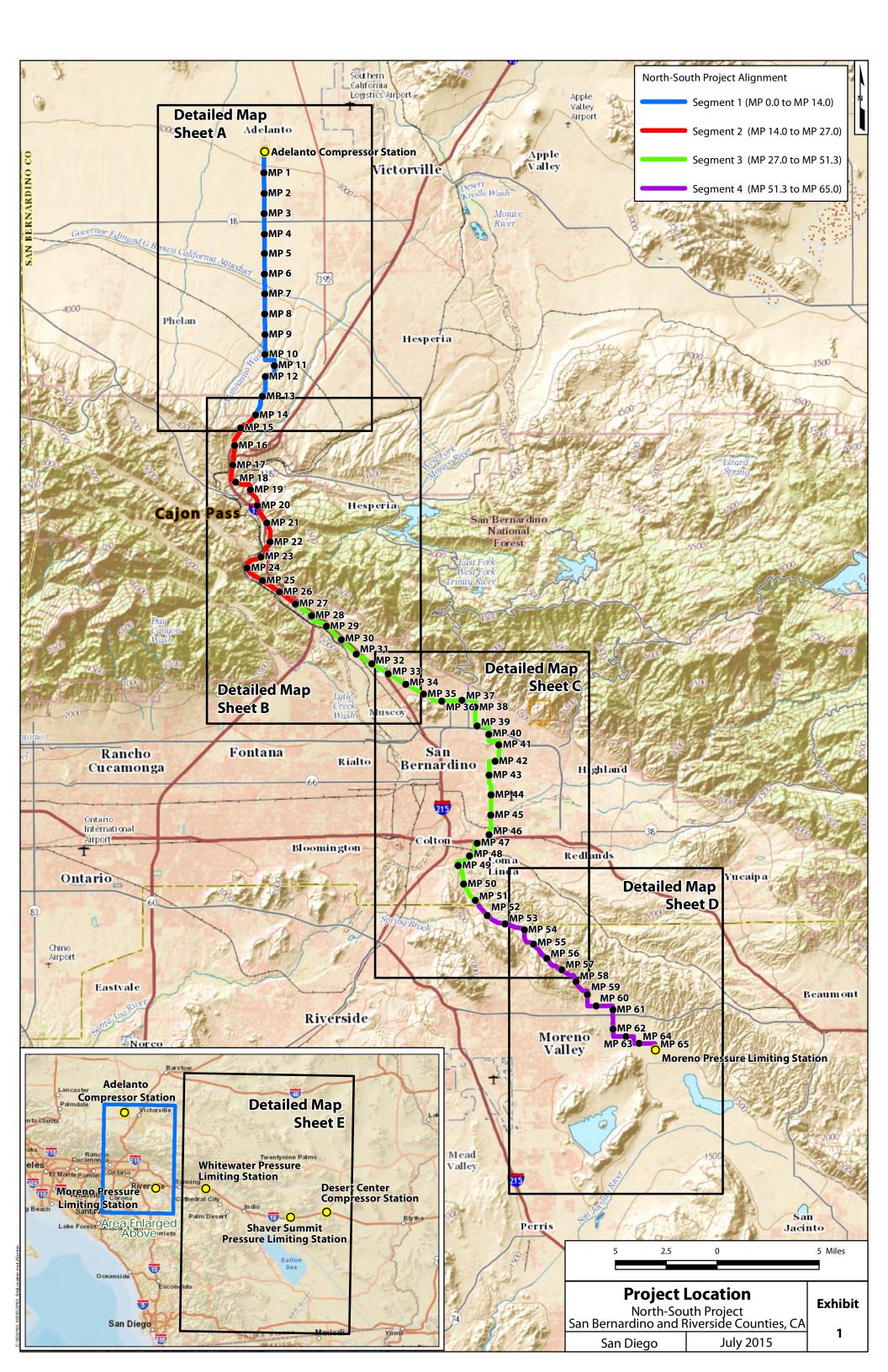
# Geosyntec<sup>></sup>

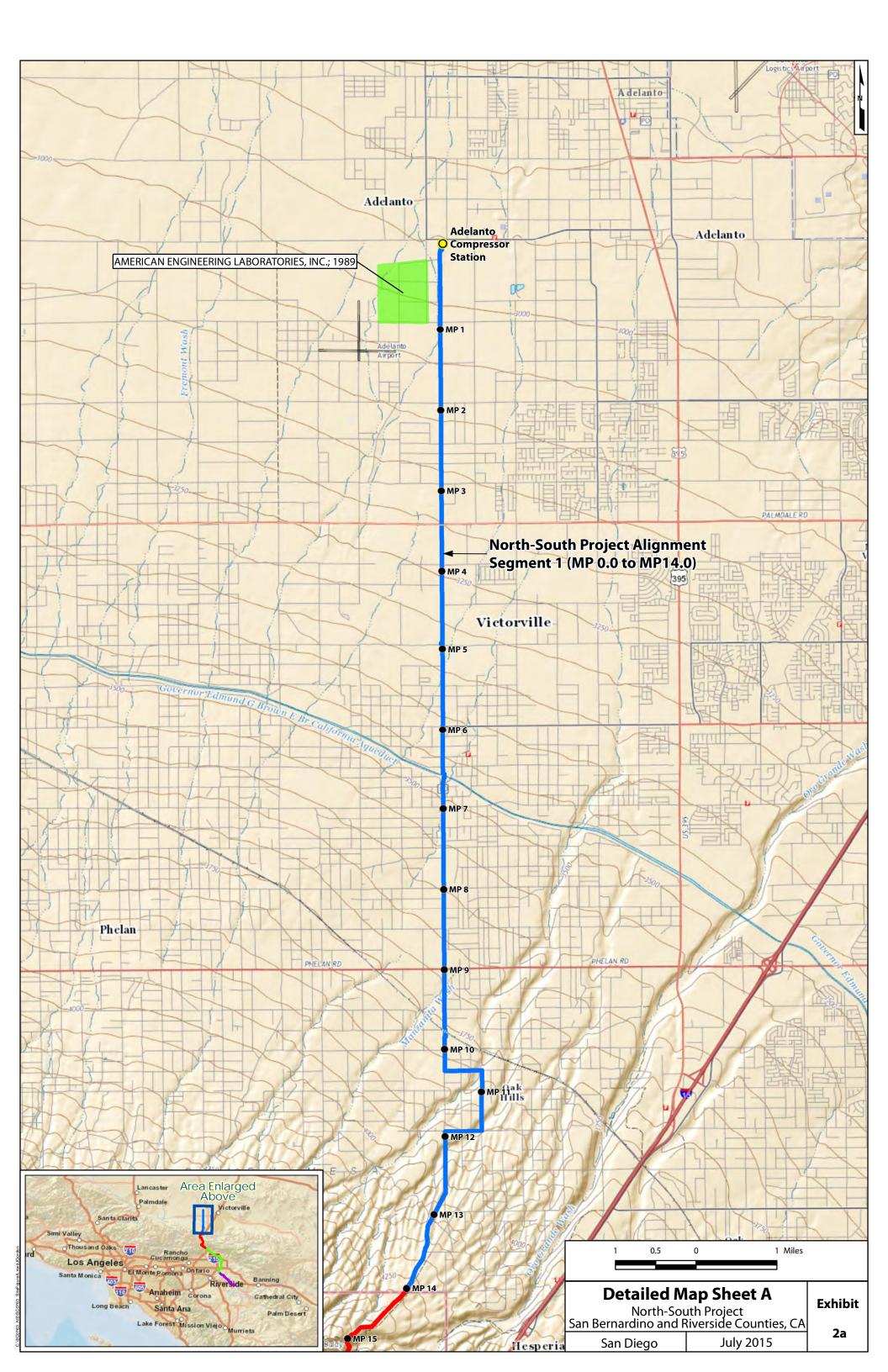
	Boring Identification <sup>d</sup>
CB-	1 to CB-5
MW	7-4, CB-1
MW	7-5
	o B4, B5-VEW1, B6 toB13, B14-VEW2, B16, -MW1, B17, B18, B20, VEW-2A
	to B26, MW2 to MW-5, MW-5A, MW-6 to MW- IW-8A
SB1	to SB8
BH-	1 to BH-9
BH-	10, BH-12, BH-13
15/0	1 toGB-3, SB-12, SB-13, SB-14/GW-6, SB- GW-7, SB-16/GW-8, SB-17, SB-18/GW-10, SB-19 B-22, SVE-1-SVE-3, VW-4
CB-	1 to CB-4
CB-	10B, CB-11, CB-12
SB-	1 to SB-7, MW-21, MW-22, MW-23
CB-	3, CB-4, CB-5
	to PB7, B8, MW1/VW1, MW2/VW2, /3/VW3, MW4, MW5 to MW9, MW12-MW17
	ing A to Boring E, CB1, CB2, CB3, FVW, MW1 to 76, MW6A, MW7 to MW11

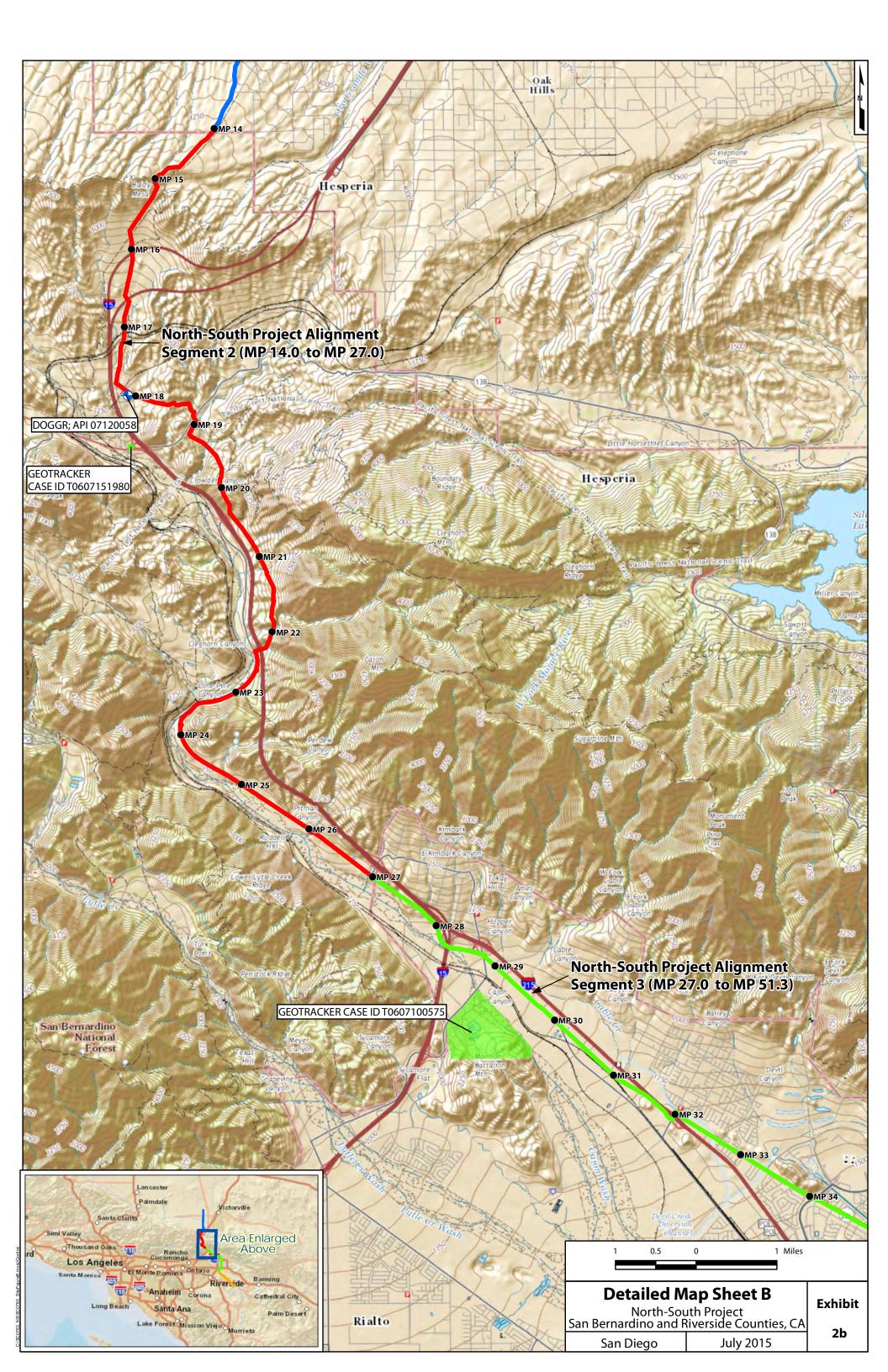


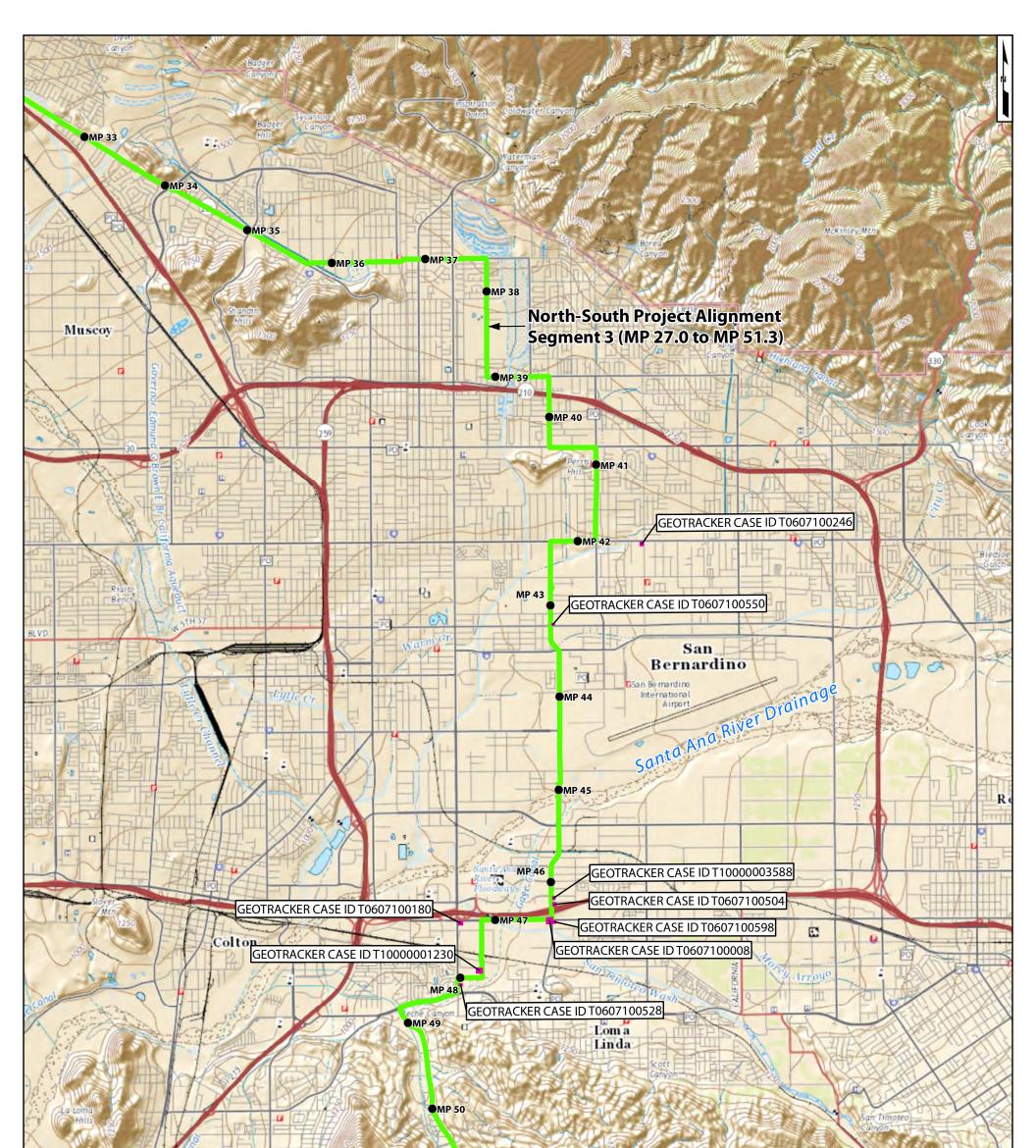
## **APPENDIX** A

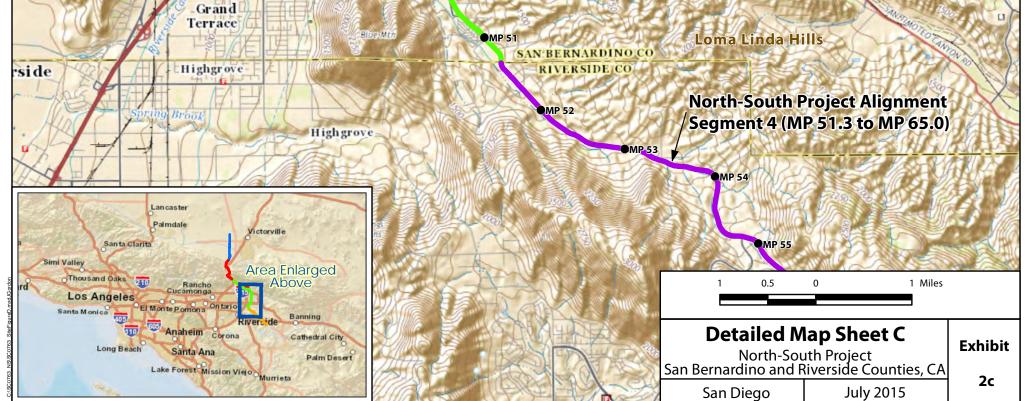
**Exhibits** 

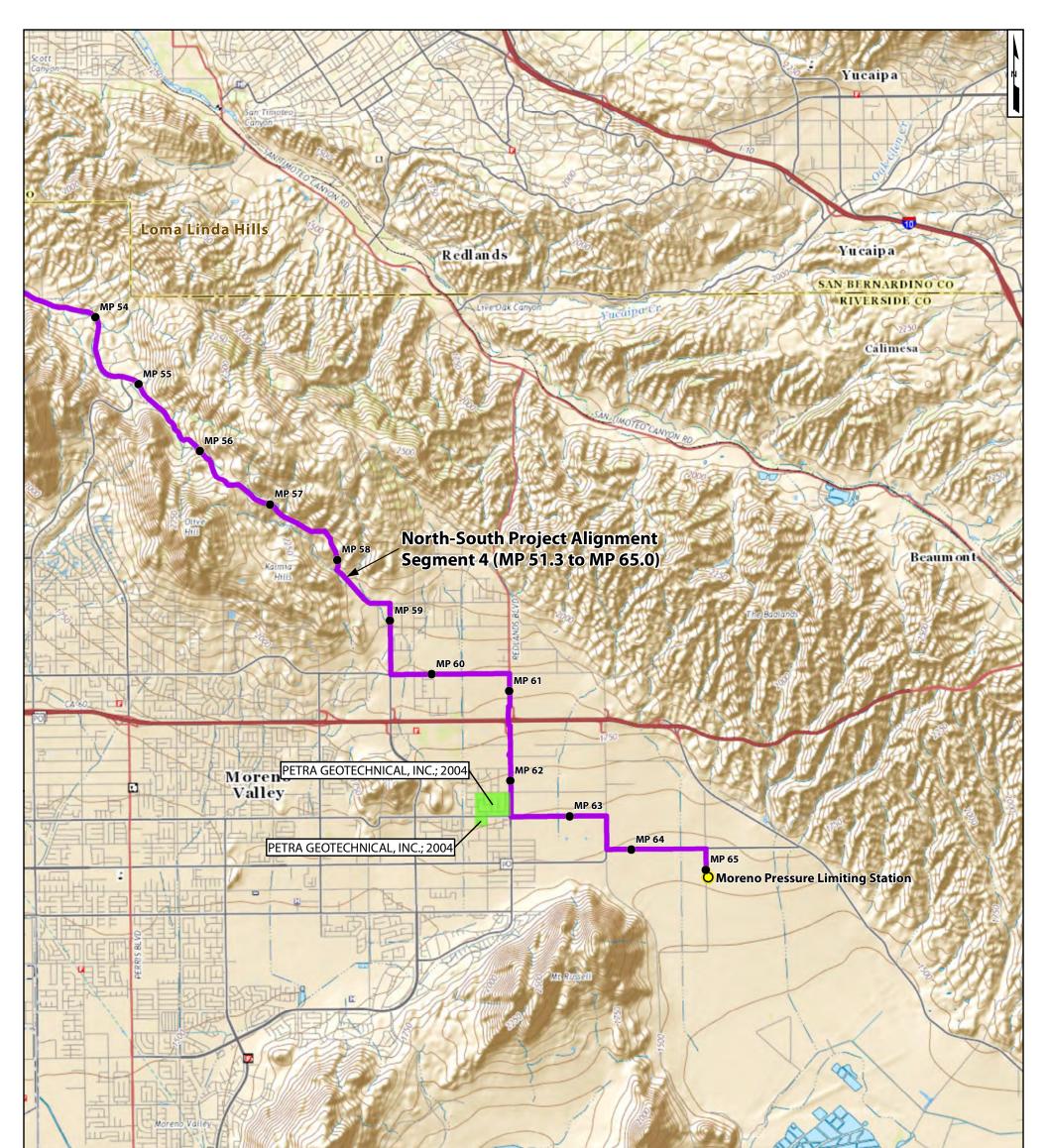


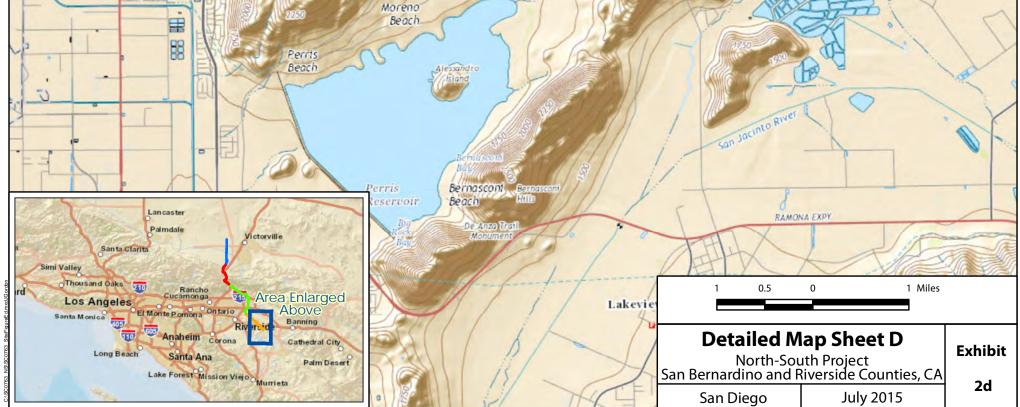


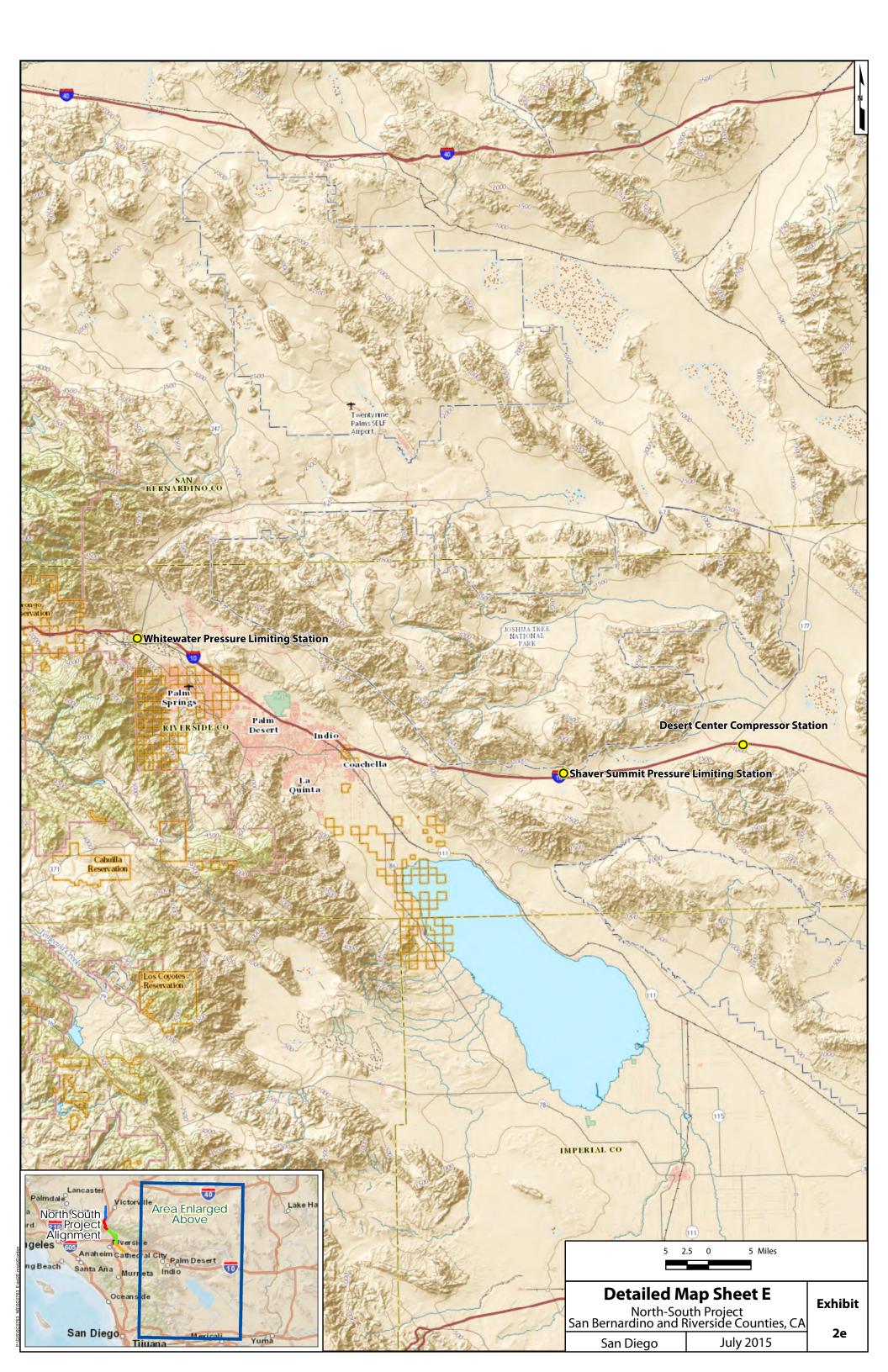


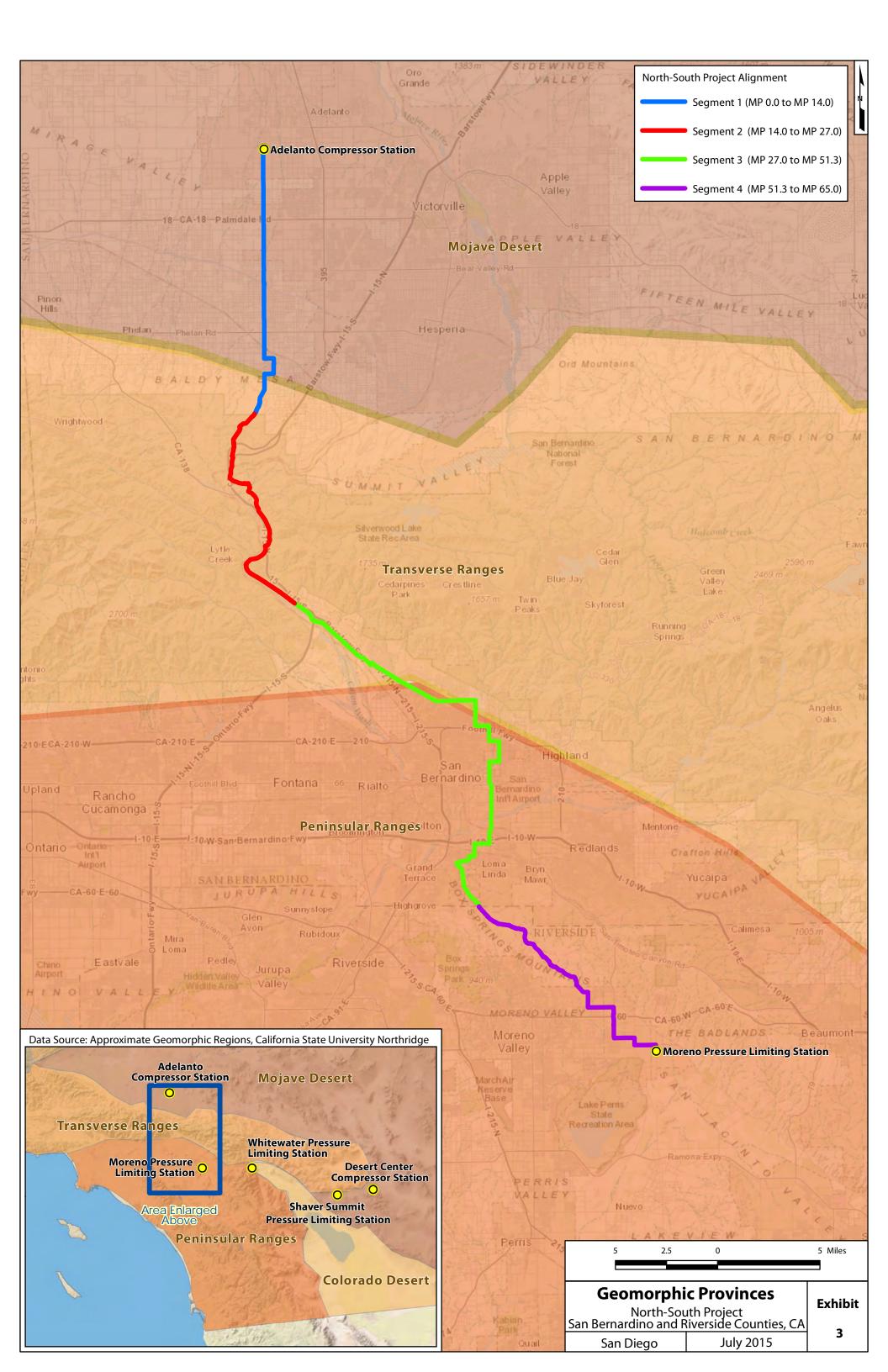


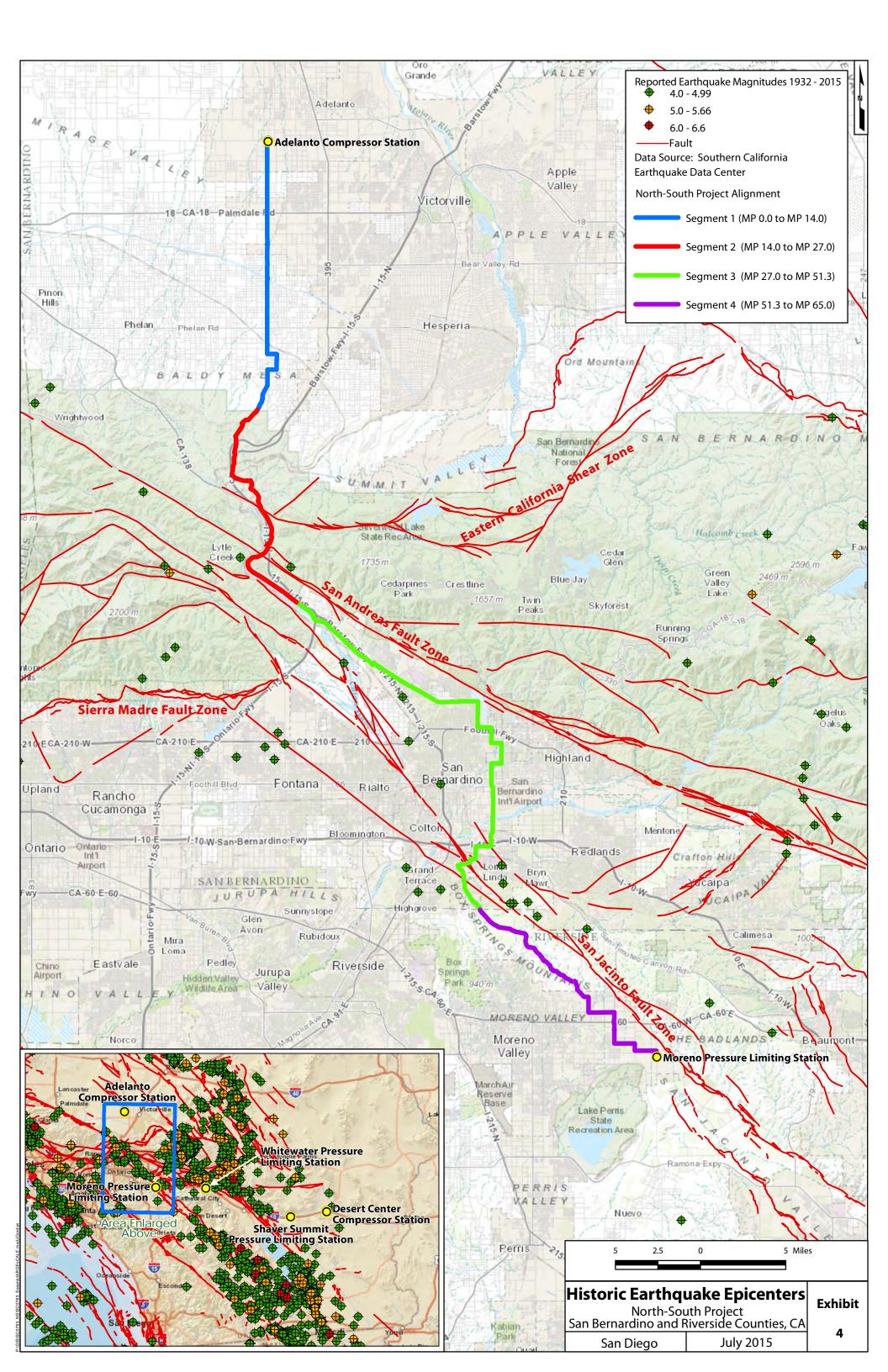












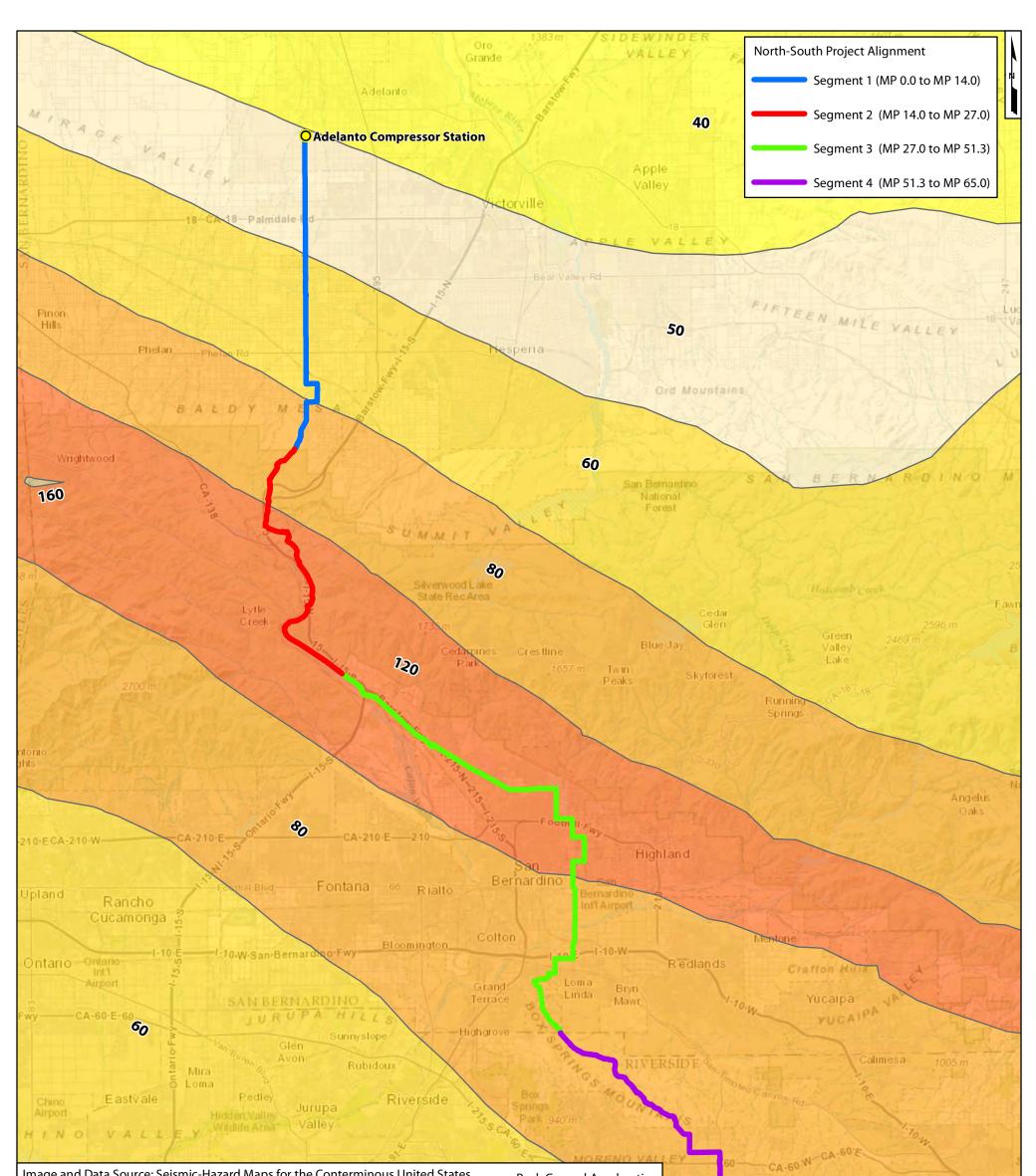
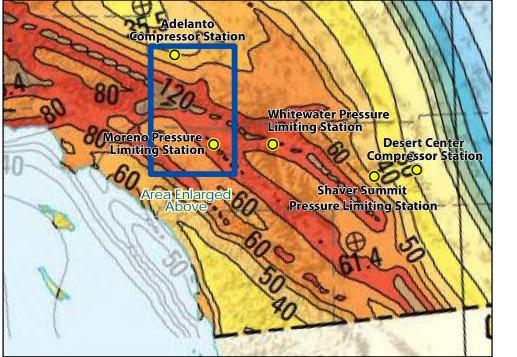


Image and Data Source: Seismic-Hazard Maps for the Conterminous United States, 2014: U.S. Geological Survey Scientific Investigations Map Issue Identification: 3325



Peak Ground Acceleration espressed as a percent of gravity (%g)

> > 160 120–160 80–120 60–80 50–60

> > 40-50

30-40

20-30

18-20

16-18

14–16 12–14

10–12 8–10 6–8

4-6

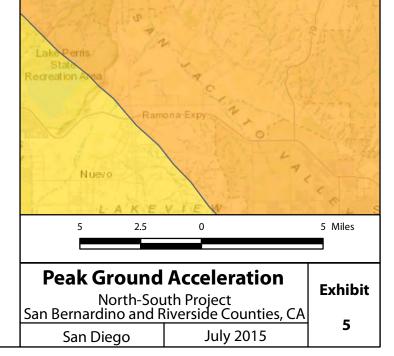
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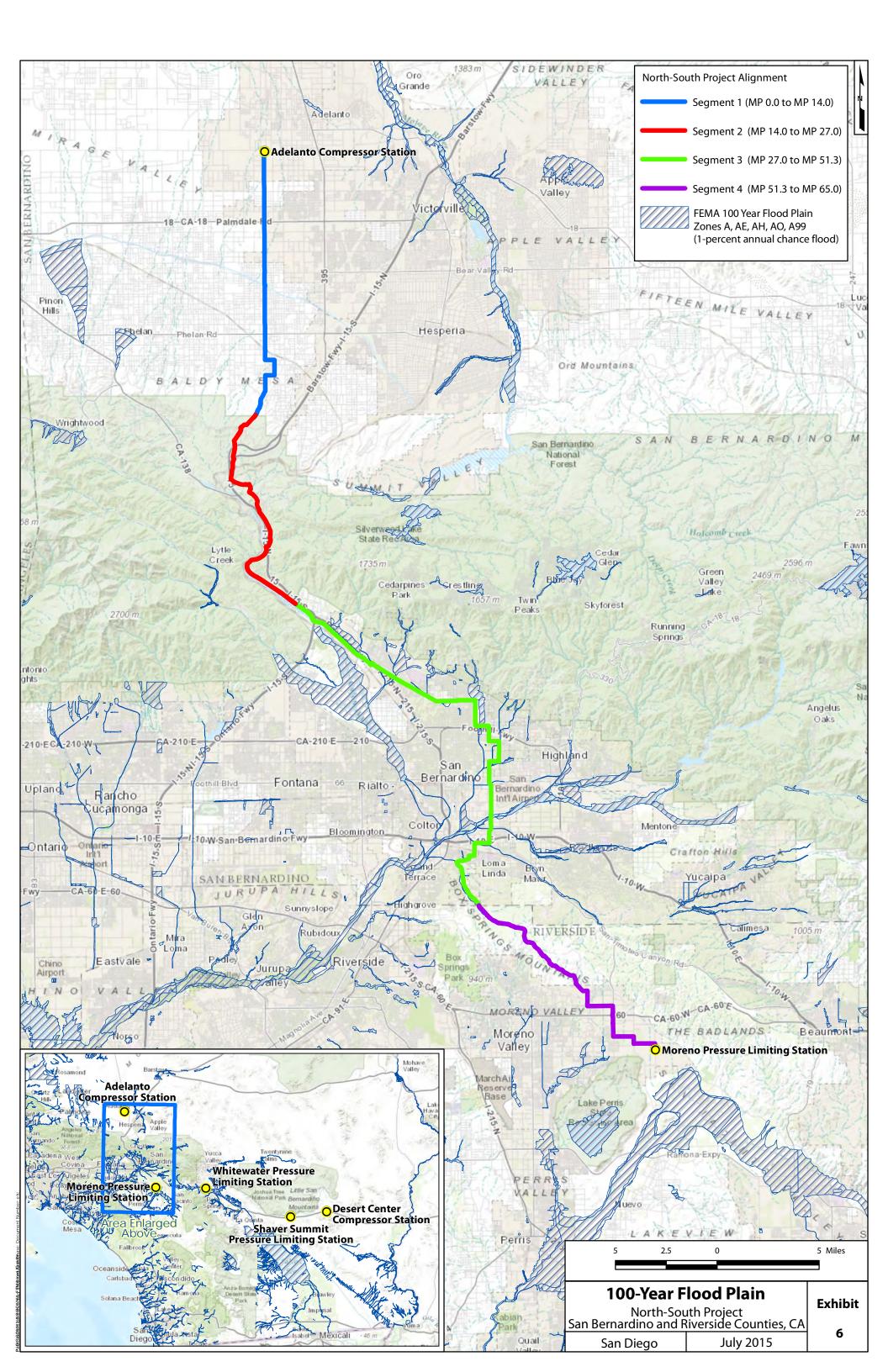
0-2

#### **O** Moreno Pressure Limiting Station

Beaumont

THE BADLANDS







## **APPENDIX B**

## LCI [2015] Geologic Hazards Evaluation



## Proposed North-South Pipeline Alignment Geologic Hazards Evaluation

Prepared for:

Southern California Gas Company 555 West Fifth Street Los Angeles, California 90051

Prepared by:

Lettis Consultants International, Inc. 27441 Tourney Road, Suite 220 Valencia, CA 91355

June 29, 2015



Lettis Consultants International, Inc. 27441 Tourney Road, Suite 220 Valencia, CA 91355 (661) 287-9900; fax (661) 287-9990

June 29, 2015

Mr. Ron Bott Southern California Gas Company 555 West Fifth Street Los Angeles, CA 90051

Subject: Proposed North-South Pipeline Alignment Geologic Hazards Evaluation

Dear Mr. Bott:

Lettis Consultants International, Inc. (LCI) is pleased to present to you our final draft report on the geologic and seismic hazards investigation for the proposed North-South Pipeline alignment. We identified and evaluated 60 alignment-fault crossings and provide within our report the results of our evaluation. Based on our evaluation and additional mapping of the identified fault crossings, LCI was able to refine and identify the principal fault crossings on the San Andreas and San Jacinto faults. The principal alignment-fault crossings will most likely experience the highest amount of ground rupture failure during the next major earthquake on these fault systems. We identified three principal fault crossings on the San Andreas Fault and two principal fault crossings on the San Jacinto fault.

We also compiled and evaluated landslide and liquefaction hazards along the proposed alignment. Liquefaction data was provided by the counties of San Bernardino and Riverside. The highest risk of permanent ground deformation due to liquefaction is located along the Santa Ana river corridor which has a large accumulation of loose, sandy alluvial materials and historically high groundwater levels. In addition to summarizing the liquefaction hazards within San Bernardino and Riverside counties, we have also provided an assessment of liquefaction induced settlement for San Bernardino County where risk of liquefaction is highest along the proposed alignment. Landslide hazards were compiled from various published maps and supplemented by LCI with localized mapping of potential landslides. The proposed alignment has been routed effectively around major landslide hazard areas, although potential landslide hazard areas exist in the Cajon Pass area that may generate new landslides due to strong ground shaking or oversaturation of hillslope deposits during years of above normal wet weather.



Please do not hesitate to contact us with any questions, comments, or concerns you may have regarding this report. You may contact us directly at 661-287-9900.

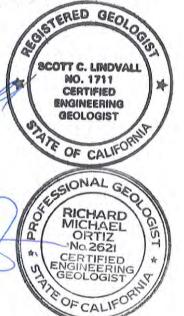
Sincerely,

Lettis Consultants International, Inc.

Scott C. Lindvall, CEG 1711 Senior Principal Geologist

Richard M. Ortiz, CEG 2621 Senior Project Geologist

Christopher D. Kemp Project Geologist





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## 1.0 INTRODUCTION

This report summarizes our findings regarding the geologic and seismic hazards identified along the proposed North-South Pipeline alignment. The principal issues addressed by this study are fault rupture, liquefaction and landslide hazards along the proposed alignment. We also performed an analysis of liquefaction induced settlement (Attachment 1).

The proposed pipeline alignment generally trends north-south between the Adelanto Compressor Station in the Mojave Desert and the Moreno Valley Large PLS Station in Moreno Valley. The proposed alignment is approximately 65-miles-long and traverses two major fault zones (Figures 1 and 2), the San Andreas fault zone (Figure 3) and the San Jacinto fault zone (Figure 4), along its planned route. Portions of both fault zones have been designated Alquist-Priolo (AP) Earthquake Fault Zones (Figure 2) and contain multiple Holocene (less than 11,000 years) active and Quaternary (less than 2.5 million years) active fault strands. Additionally, landslide hazards are present near the alignment in locations where the proposed alignment crosses steep terrain such as Cajon Pass and Reche Canyon, and liquefaction hazards are present in areas with high ground water and young, sandy alluvial materials, such as the Santa Ana River corridor in the San Bernardino Valley (Figure 1).

## 2.0 METHODOLOGY

## 2.1 Data Review

LCI compiled and reviewed a comprehensive set of geologic data which covers the proposed North-South Pipeline alignment. These data include:

- Quaternary faults published by the California Geological Survey (CGS) (Figure 1)
- Alquist-Priolo (AP) Earthquake Fault Zones and active faults (Figure 2)
- Quaternary faults published by the U.S. Geological Survey (USGS) (Figure 2)
- Modeled displacement estimates from ShakeOut (Figure 5B)
- Distribution of ShakeOut displacement estimates in the Cajon Pass crossing area (Figure 6)
- Geomorphic offset measurements compiled in UCERF3 (Figure 7)
- Available historic (1930s era) topographic maps (Figure 9)
- Available Dibblee geologic maps (1:24k scale) (Figure 10)
- Available USGS geologic maps (1:24k scale) (Figure 11)
- Available CGS geologic maps (1:100k scale) (Figure 12)
- Available historical (pre-1945) aerial photograph coverage for the pipeline alignment (Figure 13)
- Available lidar coverages for the pipeline alignment (Figure 14)
- Liquefaction hazards maps that cover the proposed pipeline alignment (Figure 15)



## 2.2 Identification of Fault Crossings

As noted above, the proposed pipeline alignment crosses the high slip rate San Andreas and San Jacinto fault zones, the main strands of which are high slip rate Holocene (less than 11,000 years) faults, as well as lower slip rate, Holocene and Quaternary (less than about 2.6 million years) faults such as the Cleghorn fault.

To identify fault crossings for the proposed pipeline alignment, fault traces were compiled from several map sources, including native digital sources and traces digitized from scanned paper maps. The data sources include:

- AP Earthquake Fault Zone Maps (CDMG 1974a, 1974b, 1974c, 1977a, 1977b),
- AP Fault Evaluation Reports (Burnett and Hart, 1976; Hart, 1977),
- USGS Quaternary Fault and Fold Database (USGS and CGS, 2006, accessed 2014),
- 2010 Fault Activity Map of California (Jennings and Bryant, 2010),
- Geologic maps from Bedrossian et al. (2012), Bedrossian and Roffers (2012), and Dibblee and Minch (2003a, b, c, d; 2004a, b).

A compilation of fault crossings from these sources (of various map scales) is listed in Table 1. Fault crossings are listed sequentially from north to south along the proposed pipeline alignment. The fault crossings also include faults mapped by LCI based on interpretation of lidar, aerial photography, and trench logs from existing consultant reports. Each fault crossing has been assigned a unique identifier which provides information on the pipeline alignment, fault crossed and source map of the fault trace. For example crossing ID "ADM-SAF-AP-01" corresponds with North-South Pipeline (ADM) - San Andreas Fault (SAF) – AP Map Source (AP) – Crossing #1 (01).

Table 1 provides the following information for each fault crossed by the proposed pipeline alignment:

- Unique fault crossing identification (Crossing ID)
- Location information is provided as State Plane (ft) and latitude and longitude coordinate pairs
- The fault name listed is the published fault name
- Dominant sense of slip, if known
- Age of the most recent fault movement (e.g. Holocene, Quaternary, etc.)
- Source of original mapping

The next step involved distilling this initial compilation of fault crossings from multiple sources (Table 1) into a final set of Holocene or probable Holocene fault crossings that are considered to represent a potential rupture hazard to the proposed pipeline alignment (Table 2). Table 1 includes duplicates of the same fault and in some cases, slightly different depictions of the same fault. In areas where the same fault has been depicted slightly differently by different mappers,



we typically chose to use the AP fault strand locations, as this represented the best available information. The exceptions to this rule were where trench logs or modern lidar was available to help refine and improve a fault's location. In these cases, the fault crossings are labeled as "LCI-preferred." Table 2 also includes fault crossing locations based on local geologic maps (Cleghorn and Loma Linda faults). Bedrock faults classified as pre-Quaternary in age are not considered as they do not represent a surface rupture hazard due to the lack of evidence for recent displacement and are not included in the final tabulation of fault crossings (Table 2).

Table 2 also includes the following information on the characteristics of each fault and the fault crossing:

- "Primary" or "secondary" designation for each fault crossing
- Published slip rates for primary fault strands
- Estimated fault displacement to consider for design
- Angle of incidence between alignment and fault
- Type of pipe deformation
- Figure showing location of fault crossing

Portions of the San Andreas and the San Jacinto fault zones were designated under the AP Act as Earthquake Fault Zones in the mid-1970s. As such, the State of California mandates the completion of a fault rupture hazard investigation for large developments and structures designed for human occupancy that are located within the designated fault zone. CGS has made fault investigation reports (FIRs) from these investigations performed between 1974 and 2000 available to the public, of which 26 studies have been performed along the San Andreas and San Jacinto faults in the vicinity of the proposed pipeline alignment. These studies were reviewed to determine locations of fault strands, and in some cases, which strands might be active.

In addition to FIRs, this study further refined the locations of the pipeline alignment-fault crossings of primary fault strands through interpretation of historic aerial photography and modern lidar based topographic data. All fault locations were assessed using these data, however, only the primary fault strands were well enough expressed that their previously mapped locations warranted revising. We have provided refined crossing locations for the primary and secondary strands of the San Andreas Fault in Cajon Pass (Figures 5a and 5b), and the principal strand of the San Jacinto fault near Loma Linda, CA (Figures 8a and 8b). LCI anticipates the majority of surface displacement will occur at these primary fault crossings during the next large magnitude earthquake on the San Andreas and San Jacinto faults.

## 2.3 Liquefaction Hazard Evaluation

We reviewed available USGS, CGS, and San Bernardino and Riverside County and City general plan safety elements to evaluate liquefaction hazards along the proposed pipeline alignment (Figure 15). Based on the review of available studies, Matti and Carson (1991)



provide a comprehensive assessment of liquefaction susceptibility for the San Bernardino Valley that was incorporated into the Safety Element of the San Bernardino County General Plan. We performed an analysis of liquefaction induced settlement (Attachment 1) based on data utilized by Matti and Carson (1991). For this approach, the liquefaction induced settlements are calculated numerically using the SPT data and the approach described by Tokimatsu and Seed (1987). The Riverside County General Plan (County of Riverside, 2003) assigned the liquefaction susceptibility as low for the pipeline alignment within Riverside County due to a lack of groundwater data along the alignment. Therefore, the settlement analysis was performed for San Bernardino County only.

For San Bernardino County, Matti and Carson (1991) used four data sources for the assessment of the liquefaction susceptibility: (1) soil and foundation investigations filed with city and county agencies in compliance with permitting procedures, (2) subsurface investigations for flood-control structures and building projects prepared by the U.S. Army Corps of Engineers, (3) logs of test borings for bridges on State and Federal highways prepared by the California Department of Transportation (CalTrans), and (4) SPT data obtained during a 27-site drilling project conducted in the San Bernardino Valley area by the U.S. Geological Survey.

GIS data provided by Riverside County (County of Riverside, 2014) was utilized for this study and provides the best liquefaction information for the Moreno Valley section of the alignment. The San Bernardino Valley and Moreno Valley represent the only areas along the proposed alignment where liquefaction hazards have been defined in previous studies. Liquefaction hazards may be present in areas along portions of the proposed pipeline alignment north of Cajon Pass, within Cajon Pass, or within Reche Canyon that were not classified by previous studies, but no additional known data is available, therefore, at a minimum, these areas should be considered to have a low probability of liquefaction hazard in areas dominated by alluvial materials.

## 2.4 Landslide Hazard Evaluation

The proposed pipeline alignment crosses two areas of landslide hazard potential due to steep topography and the presence of geologic formations susceptible to mass wasting: Cajon Pass and Loma Linda Hills. In addition to evaluation of published maps, this study also reviewed available aerial photographs (Table 3) and lidar topographic data to identify potential landslides that could potentially affect the proposed pipeline alignment. We limited our desktop assessment of landslide hazards to areas within 0.5 miles of the proposed pipeline alignment. We also performed a field assessment for observable landslides visible from CA-138, I-15/U.S. 66 through Cajon Pass, and along Reche Canyon Road.



# 3.0 RESULTS

Based on our review of the published studies and maps described above, the potential fault, liquefaction and landslide hazards along the planned pipeline alignment can be characterized as described below.

# 3.1 Fault Crossing Evaluation

The proposed North-South Pipeline alignment crosses 60 mapped fault strands, all of which are listed in Table 1. Many of the 60 individual crossings represent duplicate depictions of the same fault strand mapped by different authors and published at different scales. This study evaluated each of the crossings listed in Tables 1 and 2 and utilized historical aerial photography, lidar topographic data, and FIRs to further refine and characterize the locations of major fault crossings along the proposed alignment. Table 2 lists the results of the evaluation and refinement of fault crossing locations based on existing data and additional interpretation performed for this study. These final fault crossings (Table 2) represent the alignment fault crossings with the greatest exposure to surface fault rupture hazards. The greatest anticipated hazards are located where the alignment crosses the San Andreas and San Jacinto faults zones. The San Andreas and San Jacinto faults are both high slip rate faults that make up the primary components of the dextral boundary between the North American and Pacific Plates. As such, there is a potential that the proposed pipeline alignment could experience surface fault rupture on either one or both of these faults during the life of the project.

#### 3.1.1 San Andreas fault zone

The proposed pipeline alignment traverses the North San Bernardino section of the southern San Andreas Fault in Cajon Pass (Figure 1). This section of the fault has a UCERF3 preferred geologic slip rate of 20 to 30 mm/yr based on consideration of several of nearby studies compiled in Appendix B of UCERF3 (Dawson and Weldon, 2013). This section of the fault ruptured during the 1857 Fort Tejon earthquake, the most recent event on the southern San Andreas Fault. Based on previous studies, the surface rupture appears to have terminated about 8 miles south of the pass near Devil Canyon (Zielke et al., 2012; Madden et al., 2013).

Where it is crossed by the proposed pipeline alignment, the San Andreas fault zone is about 1.25 miles wide and comprises several discrete fault strands, as well as the potential for fault strands that cannot be identified in young material or steep bedrock slopes (Figure 3). Due to curves and bends, the alignment crosses multiple strands of the San Andreas fault zone over approximately 1.4 mi as measured along the alignment. The locations of strands comprising the primary fault zone were refined from previously published mapping through our interpretation of lidar and 1930s era aerial photography, which displayed strong geomorphic evidence for the locations of the primary fault strands. The 350-ft-wide zone between fault crossings "ADM-SAF-LCI\_Preferred-02" and "ADM-SAF-LCI\_Preferred-03" defines the primary zone of the fault and the section of the proposed pipeline alignment that could experience the greatest amount of



surface displacement during the next ground rupturing earthquake (Figures 5a and 5b). It is unclear how slip may be distributed within this primary fault zone, however, it appears slip will likely be concentrated along the faults bounding the zone, while interstitial strands may carry subordinate amounts of slip.

A secondary strand of the San Andreas fault zone, "ADM-SAF-LCI\_Preferred-01", is located to the northeast of the primary fault zone crossing (Figure 5b). This strand was included in Bedrossian's (2012) mapping and its location was refined on the basis of offset terrace risers on the east side of the valley, and geomorphology suggestive of faulting to the west observed in lidar data. We interpret that this secondary strand is likely to carry less than a meter, or about three feet of slip in the next large earthquake, consistent with Trieman et al. (2008) who interpret that all slip in the ShakeOut scenario is concentrated on the primary fault zone through Cajon Pass.

#### 3.1.2 San Jacinto fault zone

The entire width of the San Jacinto fault is crossed by the pipeline alignment in the cities of Colton, San Bernardino and Loma Linda (Figure 1). The alignment crosses the San Bernardino Valley section of the fault, which has a UCERF3 preferred slip rate of 2 to 10 mm/yr including uncertainty (Dawson and Weldon, 2013).

The San Jacinto fault zone is about 0.5 miles wide where it is crossed by the proposed alignment; however, the length of the alignment within the fault zone is about 0.6 miles (Figure 4). If the Loma Linda fault (early Holocene-late Pleistocene age) to the northeast and the Rialto- Colton fault (late Quaternary age) to the southwest, are considered secondary strands of the San Jacinto fault zone, the entire fault zone width increases to nearly 2 miles. The length of the proposed alignment between the Loma Linda fault and Rialto-Colton fault is about 3 miles (Figure 4).

Near the towns of San Bernardino, Colton, and Loma Linda the primary strand of the San Jacinto fault is located as a single point crossing identified as "ADM-SJF-LCI\_Preferred-01" (Figures 8a and 8b). This pipeline alignment crosses the fault within the intersection of Wier Road and East Washington Street, and replaces the "ADM-SJF-AP-03" as the location of the main fault strand. The location of the primary stand is well constrained locally because the fault was exposed in multiple trenches excavated by Sieh et al. (1973) and by Leighton (1980) prior to the construction of the Village Park subdivision. The nearest trench exposure was approximately 350 ft northwest of the Wier Road and East Washington Street intersection and the fault was expressed as a prominent west-facing fault scarp. These studies postdate the publishing of the official AP Earthquake Fault Zone Map for this area and therefore, have helped to improve the understanding of the fault zone that was originally shown on the AP map. Sieh et al. (1973) utilized C14 dating of unbroken layers above the last observed faulting event, which indicated an age of 670 years before present; however, the Leighton (1980) study found offset sediments containing manmade artifacts (square headed nails) which suggests a surface



rupturing earthquake occurred after the valley was settled in the early 1800s. This rupture may be associated with the 1899  $M_L6.5$  or the 1918  $M_L6.8$  San Jacinto Earthquakes, which both had epicenters south of Moreno Valley in Hemet, CA. The rupture may also be associated with the closer, but smaller magnitude 1923  $M_L6.3$  North San Jacinto Fault Earthquake, with an epicenter located near the alignment in Reche Canyon (Laughlin et al., 1923).

South of the primary San Jacinto fault crossing, fault crossing "ADM-SJF-LCI\_Preferred-02" is located in Reche Canyon based on interpretation of geomorphic features identified within lidar data. The fault crossing identified at this location is considered to be a secondary fault strand, which has been referred to the "Rialto-Colton fault" in previous fault compilations (e.g. USGS and CGS, 2006). At this location within the Loma Linda Hills, the fault is expressed as a series of well-defined linear ridges and scarps that project toward the proposed pipeline alignment. At the crossing location in the base of the canyon, deformed Holocene alluvial deposits (vertically separated) are observed in lidar data, as well as tonal lineations in aerial photography. Therefore, this strand is interpreted to be active and thus pose a hazard for potential ground rupture during a large earthquake on the San Jacinto fault zone.

#### 3.1.3 Minor faults

In addition to the main strands of the San Andreas and San Jacinto fault zones, the proposed pipeline alignment crosses several minor faults or secondary strands located well away from the major fault zones.

Several lower slip rate faults that do not comprise major elements of the plate boundary are crossed by the proposed alignment. These include the Cleghorn fault, Punchbowl fault, Peters fault, the Frontal Fault Zone of the San Bernardino Mountains, the Loma Linda fault, and the Colton-Rialto fault, which have been mapped by multiple authors (Bedrossian, 2012; Dibblee and Minch, 2003a; Jennings and Bryant, 2010; USGS and CGS, 2006) in the Cajon Pass area (Figures 1 and 2). Within Cajon Pass, none of these faults are well expressed in Quaternary deposits on the basis of lidar and aerial photo interpretation; their locations in previously published maps appear to be primarily based on bedrock relationships. With the exception of the Cleghorn fault, none of these faults are independently seismogenic (i.e., capable of generating earthquakes), therefore, we estimate earthquake surface rupture displacements below only for the Cleghorn fault.

The Cleghorn fault is a left-lateral, Quaternary active fault with a reported slip rate of 0.3 to 0.6 mm/yr based on studies compiled in UCERF3 Appendix B (Dawson and Weldon, 2013). The fault shares a similar orientation and sense of slip with other roughly east-west-striking faults in the Transverse Ranges and, therefore, may rupture independently of the San Andreas Fault. The fault is readily apparent in aerial photography along much of its trace, expressed prominently as erosional contrasts between different bedrock juxtaposed by the fault. The fault is considered pre-Holocene by Bryant (2003), however it is included as a seismic source in



UCERF3 (Field et al., 2013). As such, we conservatively assess the Cleghorn fault to be a probable Holocene fault and include it as a hazardous fault crossing in Table 2.

The Punchbowl fault is a secondary strand of the San Andreas fault zone. It is considered an AP fault, although no FER exists to evaluate the basis for its inclusion (Figure 3). In general, AP-mapped faults are included in the UGSS Quaternary fault and fold database exactly as they were drafted in official AP maps; however, the Punchbowl fault is not. As with other secondary faults crossed by the alignment, the Punchbowl fault is not well expressed in young alluvium; however, it is clearly expressed as linear valleys, aligned saddles, and ridge notches in steep bedrock terrain. Nearest the crossing area, expression fades, therefore, we adopt the traces depicted in AP maps. We conclude that the fault is likely an older strand of the San Andreas fault zone that does not independently generate earthquakes and has carried a small amount of slip (three feet or less) in Holocene earthquakes. This determination was made on the basis of the lack of expression in younger deposits.

Peters fault is also considered a secondary strand of the San Andreas fault and is zoned as an AP fault. It branches west off the San Andreas fault and crosses the pipeline alignment approximately 3.5 miles (along the pipeline alignment) southeast of the primary San Andreas fault crossing (labeled as "ADM-SAF-Bedrossian-10" crossing in Figure 2).

Trenching by Sieh et al. (1973) and Leighton (1980) identified the primary strand of the San Jacinto fault zone near the proposed North-South pipeline alignment (Figures 8a and 8b). The faults exposed within the trenches record the most recent rupture on the principal trace of the fault, but given the young, historic age of the deposits, not all secondary faults may have been recognized in these excavations.

Outboard of the primary San Jacinto fault zone, the Loma Linda fault to the northeast and the Rialto-Colton fault to the southwest are also considered secondary strands of the San Jacinto fault. The Loma Linda fault is approximately 1.7 miles (along the pipeline) northeast of the primary San Jacinto fault crossing, whereas the Rialto-Colton fault (labeled as "ADM-SJF-LCI\_Preferred-02" crossing in Figure 4) is approximately 1.3 miles (along the pipeline) southwest of the primary San Jacinto fault crossing.

## 3.1.4 Earthquake recurrence

Paleoseismic sites on the San Andreas Fault in the vicinity of the alignment include Wrightwood, Pitman Canyon and Plunge Creek paleoseismic sites, which are located about 2 to 20 miles from the Cajon Pass crossing area (Figure 7). Table 4 contains a summary of data (after Weldon et al., 2013) from each of these paleoseismic sites. Results of these paleoseismic studies suggest that large, surface-rupturing earthquakes on the San Andreas Fault occur at approximately 100 to 200 year intervals. The most recent event on the San Andreas Fault was the 1857 Fort Tejon earthquake.



On the San Jacinto fault, the Mystic Lake paleoseismic site lies about 10 miles southeast of the San Bernardino crossing area of the alignment (Figure 7). This site revealed evidence of 7 earthquakes since 579 A.D. with an average recurrence interval of 181 years (Onderdonk et al., 2013; Weldon et al., 2013). Accounting for uncertainty in age dating, a recurrence interval range of 150-212 years was obtained for past earthquake ruptures at the Mystic Lake site on the San Jacinto fault.

No recurrence information exists for the Cleghorn fault, however, a recurrence interval can be approximated based on a strain accumulation and existing data. Based on a slip rate of 0.3 to 0.6 mm/yr and the average displacement for a 25.9-km-long fault using Wells and Coppersmith (1994) surface rupture length-displacement relationships, the Cleghorn fault has a recurrence interval of approximately 1,400 to 1,900 years. However, based on the expression of the fault in aerial photography, the recurrence interval is likely much longer and on the order of a few to several thousand years.

## 3.2 Displacement Estimates

Deterministic fault displacement estimates were developed by incorporating multiple approaches and datasets to characterize potential future fault displacements on the San Andreas fault zone, San Jacinto fault zone, and Cleghorn fault. The single recommended displacement value to consider for design, listed for each crossing in Table 2, were developed by considering the following approaches:

- 1) Calculate average and maximum displacements for deterministic earthquake scenarios using Wells and Coppersmith (1994) empirical relations (Table 5).
- 2) Consider model-derived displacement values from the USGS ShakeOut Scenario for the San Andreas fault zone (Jones et al., 2008).
- 3) If available, use observed displacements from past earthquakes to inform measurements of offset landforms along the San Andreas and San Jacinto faults, compiled and reassessed in UCERF3 (Madden et al., 2013).
- 4) Assess strain accumulation based on geologic slip rates and elapsed time since the last rupture.

For the San Andreas fault, we describe displacement estimates from these four approaches. There is less information available, however, for the San Jacinto and Cleghorn faults, and only certain approaches for these faults are described. As noted above, there are also several secondary fault strands crossed by the proposed alignment (Table 2), that could produce small displacements. Given their short lengths and spatial association with the San Andreas and San Jacinto fault zones, these secondary faults are not considered independent seismogenic sources and will only accommodate secondary or sympathetic slip in larger, main fault ruptures. For the purposes of characterizing fault displacement, secondary fault strands are simply assigned a conservative, upper-bound displacement of 1 m (3 ft). Slip on secondary strands will typically be significantly less than this upper-bound estimate.



#### 3.2.1 San Andreas Fault

Fault displacements are commonly estimated using empirical relations relating earthquake magnitude to average or maximum displacement. The Wells and Coppersmith (1994) magnitude-displacement relations for all-slip-types were used to calculate average and maximum predicted displacements for a **M**7.8 San Andreas earthquake (Table 5). This deterministic scenario is the same magnitude as the 1857 earthquake and what was used for the ShakeOut Scenario (Jones et al, 2008). An average displacement of 3.8 m (12.5 ft) and a maximum displacement of 8.6 m (28.2 ft) is calculated for a **M**7.8 San Andreas earthquake (Table 5).

The ShakeOut Scenario was a CGS and USGS study that evaluated the impacts of a momentmagnitude 7.8 (**M**7.8) San Andreas scenario earthquake on southern California. This scenario assumed the San Andreas fault ruptured between the Salton Sea and Lake Hughes. Among other effects of the earthquake (e.g., damage, economic), the study estimated displacements of various lifelines (e.g., gas pipelines, power lines) that cross the fault. Displacement values were modeled at approximately 1,000 ft intervals along a single, simplified fault trace with modeled estimates taking into account observed paleoseismic slip measurements where available.

The two nearest ShakeOut estimates of net displacement values on either side of the proposed pipeline alignment crossing of San Andreas fault suggest a range from 3.5 to 4.0 m (11.5 to 13.1 ft) (Figure 5b). The San Andreas fault zone at Cajon Pass comprises several strands, each of which may carry a subordinate amount of net slip in an earthquake. For the ShakeOut study, Trieman et al. (2008) estimated the distribution of modeled net displacement on various fault strands in Cajon Pass. Trieman et al. (2008) concluded that all slip would be concentrated on strands within the primary fault zone as shown in Figure 6.

An alternative approach is to consider that future displacement at a point on a fault will be similar to observed displacements from past earthquakes at that location (e.g., Hecker et al., 2013). The UCERF3 study included an evaluation of offset geomorphic features (e.g., ridges or stream channels) that have been documented along the San Andreas fault. The observed displacements, which are based on measurements from the field, aerial photos, and lidar data, are listed below:

- Single-event displacement measurements nearest the proposed pipeline alignment crossing of the San Andreas fault are shown on Figure 7. All of these displacements were measured by Zielke and others (2012) and reassessed and compiled in UCERF3 Appendix R (Madden et al. (2013). These single-event displacements are interpreted by Madden et al. (2013) to represent displacement from the 1857 Fort Tejon earthquake, the most recent surface rupturing earthquake on the southern San Andreas fault.
- Best-estimate mean measurements near Cajon Pass and the proposed North-South pipeline alignment (Figure 7) range from 1.0 to 4.4 m (3.2 to 14.4 ft) with uncertainty ranges from 0.4 to 5.1 m (1.3 to 16.7 ft).



• Note that displacement values in the cluster to the southeast are relatively small and likely represent a steeply decreasing slip gradient related to the southeast end of the 1857 surface rupture (Figure 7).

The accumulated strain stored on a fault since the last earthquake can provide a rough estimate of the potential displacement if the next rupture were to occur today. For faults with well-constrained slip rates and well-defined most recent event ages, potential displacement can be approximated by multiplying the slip rate with the time elapsed since the most recent event. The most recent earthquake on this portion of the San Andreas fault was the 1857 Fort Tejon earthquake, which was 157 years ago. Multiplying the UCERF3 slip rate of 20 to 30 mm/yr for the North San Bernardino section of the fault (Dawson and Weldon, 2013) by 157 years of elapsed time, yields 3.1 to 4.7 m of accumulated strain. This is relatively consistent with the average offset per event of 3.28 m measured at the Wrightwood paleoseismic site (Figure 7).

In summary, the four different approaches yield potential displacements that approach about 4 to 5 m. The only exception to this is the estimate of maximum displacement (8.6 m) from the empirical relations. We suggest that a potential displacement of 5 m (16 ft) be considered for the San Andreas fault. Pipeline engineers should anticipate dominantly right-lateral slip on the order of 5 m (16 ft) at the fault crossing. Based on the topographic expression of the fault in lidar, the primary San Andreas fault crossing could experience a minor component of south-side-up displacement. We anticipate that the majority of this displacement would be concentrated on the primary fault (ADM-SAF-LCI\_Preferred-03).

## 3.2.2 San Jacinto fault

The proposed pipeline alignment crosses the northern San Jacinto fault zone (Claremont section), which extends from the San Andreas/San Jacinto fault juncture to the Mystic Lake step-over in San Jacinto Valley. The Mystic Lake step-over and paleoseismic site (Onderdonk et al., 2013) is located about 2 miles south of Moreno Compressor Station. Comparing the timing of earthquakes from long event histories developed at Mystic Lake and Hog Lake paleoseismic studies, Rockwell et al. (2014) have modeled the behavior of the northern and central San Jacinto fault zone, respectively. Their data suggest that the Claremont section (northern) of the fault commonly fails in earthquakes by itself, but may fail less frequently in larger events involving the Clark section to the south. The Hog Lake site and Clark section of the fault zone are located southeast of the Mystic Lake site and beyond the extent of Figure 7. We use this information on potential earthquake size to estimate displacements using empirical relations of Wells and Coppersmith (1994).

Rockwell et al. (2014) suggest that a rupture of the Claremont section of the San Jacinto fault would be a **M**7.1. For this magnitude, the Wells and Coppersmith (1994) relation provides an average displacement of 1.3 m (4.3 ft) and a maximum displacement of 2.3 m (7.5 ft) (Table 5). If the northern (Claremont) and central (Clark) sections of the fault ruptured together, their combined length (~170 km) and width (~15 km) would produce a **M**7.4 event using Wells and



Coppersmith (1994) rupture area-magnitude relation for all-slip-types. For a M7.4 earthquake on the San Jacinto fault, the empirical relations provide an average displacement of 2.0 m (6.6 ft) and a maximum displacement of 4.0 m (13.1 ft) (Table 5).

Unlike the San Andreas fault, there are no observations of past displacements near the proposed North-South Pipeline alignment crossing of the San Jacinto fault. The nearest offset measurements for the San Jacinto fault in the UCERF3 compilation are located on the Clark segment of the fault zone and are about 35 miles southeast of the proposed pipeline alignment crossing of the San Jacinto fault. Best-estimate mean measurements range from 1.3 to 2.8 m (4.3 to 9.2 ft) with a range of uncertainty from 1.0 to 3.3 m (3.3 to 10.8 ft) (Madden et al., 2013). These offset measurements provide insights into the past behavior of the Clark segment of the fault zone, but are not directly used to estimate displacement for the proposed pipeline alignment, given that these observations are located about 35 miles southeast of the proposed fault crossing.

The accumulated strain on the northern San Jacinto fault can be estimated using the time elapsed since the last event and slip rate. At Mystic Lake, Onderdonk et al. (2013) established the timing of the most recent event to have occurred between 1738 and 1850 A.D., yielding 164 to 276 years since the last event. Using a slip rate of 12.5 mm/yr (Blisniuk et al., 2013) produces accumulated strain of about 2.1 to 3.5 m (6.9 to 11.5 ft).

In summary, simple strain accumulation calculations suggest that the potential displacement stored on the fault may be about 2.1 to 3.5 m. Average displacements associated with M7.1 and M7.4 earthquakes are 1.3 and 2.0 m. Maximum displacements for these two scenarios are 2.3 and 4.0 m. The west-facing scarp described by Leighton and Associates (1980) indicates a minor component of east-side-up vertical displacement could occur at the primary San Jacinto fault crossing. We recommend that a dominant right-lateral slip in the order of 4 m (13 ft) be considered for engineering assessment and project design of the proposed pipeline alignment. This displacement recommendation is more conservative than other crossings, given the urban setting of this crossing location, as it incorporates a maximum displacement from a **M**7.4 event. Based on the expression of faults in lidar and aerial photographs at the San Jacinto fault zone crossing area, we anticipate that the majority of any displacement would be concentrated on the primary fault (ADM-SJF-LCI\_Preferred\_01), with no more than 1 m (3 ft) of slip on individual secondary strands.

## 3.2.3 Cleghorn fault

Historic displacement data are not available for the Cleghorn fault, therefore, we rely on the magnitude-average displacement relationship of Wells and Coppersmith (1994) for all slip types to estimate potential future displacements. Using a rupture area of 401 km<sup>2</sup> from the UCERF3 model, a moment magnitude (**M**) 6.6 earthquake is obtained from the Wells and Coppersmith (1994) rupture area-magnitude relation. A **M**6.6 event yields an average displacement of 0.6 m (2.0 ft). A maximum displacement of 0.9 m (3.0 ft) is obtained using the maximum displacement



relation of Wells and Coppersmith (1994). Dawson and Weldon (2013) postulated future displacement on the Cleghorn fault may be dominantly left-lateral. This fault is not expressed at the ground surface near the crossing; therefore, we rely on mapping by USGS and CGS (2006) for the crossing location tabulated in Table 2.

## 3.3 Liquefaction Hazards

This study utilized liquefaction susceptibility mapping published by San Bernardino and Riverside Counties to evaluate the liquefaction hazard along the proposed ADM pipeline alignment. Liquefaction hazard zones are typically located in areas underlain by poorly compacted late Quaternary alluvial deposits with groundwater located close to the ground surface (50 ft or less). The alignment is exposed to potential liquefaction hazards in the Santa Ana River basin near San Bernardino and Loma Linda and in the Moreno Valley. Figure 15 shows the identified liquefaction hazard zones along the proposed alignment.

The San Bernardino segment (I-15/215 Interchange to Loma Linda) of the proposed alignment traverses multiple zones of localized liquefaction hazard (Figure 15). The liquefaction hazard ranges from low to moderate to high (Matti and Carson, 1991). As shown on Figure 15, zones of high to moderate liquefaction hazard are limited to the mouths of canyons and along the Santa Ana River corridor. In San Bernardino County, the majority, or 39.5 miles of the proposed pipeline alignment, is located in unclassified areas or areas of no liquefaction hazard due primarily to the lack of information on shallow groundwater. The sediments in these areas are susceptible to liquefaction if groundwater levels rise to within 50 ft of the ground surface and these areas could locally liquefy during a large earthquake. Six (6.0) miles are located in moderate to moderately high liquifaction hazard areas, 4.3 miles of the proposed alignment are located in areas of high liquefaction hazard, and 1.5 miles are located in low liquefaction hazard areas.

Riverside County provides even more robust and extensive liquefaction hazard zonation maps. The Riverside segment (Loma Linda to Moreno Valley) of the proposed alignment traverses multiple zones of localized liquefaction hazard. The liquefaction hazard ranges from very low to moderate (County of Riverside, 2014). As shown on Figure 15, the Riverside segment of the proposed pipeline alignment is underlain primarily by zones of moderate to low liquefaction hazard. In Riverside County, the majority of liquefaction hazard, or 8.9 miles of the proposed alignment is located in zones of moderate liquefaction hazard, 2.0 miles are located in low liquefaction hazard areas. 0.1 miles is located in areas of very low liquefaction hazard areas of the planned alignment is located in unclassified areas or areas of no liquefaction hazard areas, which includes areas where the proposed alignment is located in granitic bedrock materials.

As shown on Figure 15, the Riverside segment of the proposed alignment is underlain primarily by zones of moderate to low liquefaction hazard. However, in San Bernardino County, 4.3 miles of the proposed alignment are located in areas of high liquefaction hazard, 6.0 miles are located



in moderate to moderately high liquefaction hazard areas, and 1.5 miles are located in low liquefaction hazard areas. In this report, the regional geotechnical investigation in the San Bernardino Valley (Carson et al., 1986) is used to evaluate the liquefaction-induced settlement for the proposed alignment in the absence of site-specific geotechnical studies along the proposed alignment. Using the regional information, a regional site class is assigned to the proposed alignment to estimate the ground motions using 2008 USGS hazard maps. The highest PGA from USGS hazard maps in San Bernardino valley region is used for the calculation of the liquefaction-induced settlement for the propped alignment. This approach is conservative since the ground motions might be lower than the regional maximum at different segments of the proposed alignment. Moreover, the use of site-specific geotechnical investigations along the proposed alignment would result in a more robust estimation of settlements compared to use of regional studies.

The liquefaction-induced settlement analysis indicates settlements ranging from 0 inches to 4.3 inches for different boreholes shown in Figure 1 of Attachment 1. Regardless of the relative position of the boreholes, a maximum differential settlement of 4.3 inches is considered for the region. Assuming that this settlement occurs across a distance of 100 feet, a maximum angular distortion of about of 0.004 inches per inch may result (Attachment 1).

## 3.4 Landslide Hazards

The proposed pipeline alignment traverses two areas with potential for landslide hazards the Cajon Pass in the San Gabriel Mountains and the Loma Linda Hills near Loma Linda, CA. (Figure 1). Through the Cajon Pass the alignment is predominantly located within to existing utility corridors, existing roads and valley bottoms which in general have low landslide hazard potential. In the vicinity of CA Highway 138, the alignment crosses a mountainous region with potential landslide hazards due to the area's high relief and localized dip-slope conditions on east and north-east facing slopes. In this area LCI mapped 25 potential landslides in the vicinity of the alignment. These are shown on the geologic base map of Dibblee in Figure 3. Many of these mapped landslides are adjacent to the proposed pipeline alignment, but the alignment does not cross any potential landslides mapped for this study. As shown in Figure 3, the landslides located near the proposed alignment and CA Highway 138, are concentrated in east to northeast dipping Tertiary coarse-grained sedimentary formations that are predominantly sandstone and conglomerate. In addition to the landslides mapped by LCI, we also evaluated potential landslides published by the CGS (Bedrossian et al. 2012). Bedrossian et al. (2012) mapped 27 landslides within <sup>1</sup>/<sub>2</sub> mile of the proposed alignment, no landslides were intersected by the proposed alignment (Figure 12).

# 3.5 Field Reconnaissance

LCI geologists Richard M. Ortiz and Christopher Kemp completed field visits to multiple key areas along the alignment and visited readily accessible fault crossings and areas with potential landslide hazards along the proposed alignment. In the Cajon Pass area, a broad zone of



faulted materials that corresponded to the multiple strands of the San Andreas Fault in Cajon Pass was observed, however, discrete fault strands were not identified in the immediate crossing area. Faulted alluvial materials along the mapped fault trace that correspond to the zone of faulting between alignment fault crossings "ADM-SAF-LCI\_Preferred-02" and "ADM-SAF-LCI\_Preferred-03" (Figures 5 and 16) were observed. The area surrounding the San Jacinto fault crossings near Loma Linda, CA has been completely developed. The building set back recommended by Leighton (1980) and associated with the primary trace of the San Jacinto fault zone and the crossing "ADM-SJF-LCI\_Preferred\_01" has been established as a green belt within the housing development north of E. Washington Street.

For this study, off highway access to the areas along the proposed alignment north of highway CA-138 in Cajon Pass was not available. Features suggestive of landslides were observed in the areas adjacent to CA-138, such as probable head scarps and accumulated landslide deposits at the base of slopes, but no landslide features were observed directly within the visible alignment corridor accessible to LCI. No landslide features within the granite cored ridges and hillslopes along the proposed alignment within Reche Canyon south of Loma Linda, CA were observed.

## 3.6 Geologic Materials

The proposed pipeline excavation will encounter a variety of geologic materials along the alignment. These materials may range from bare bedrock and soil-mantled bedrock in areas of greater relief, to unconsolidated to partially lithified alluvial deposits in canyon and valley floors. Figures 17 through 20 depict simplified geology after Bedrossian et al. (2012) along the proposed alignment; Figure 21 is a legend of units depicted on the maps. Table 6 summarizes the geologic materials expected to be encountered along the proposed alignment. Unconsolidated Quaternary materials make up about 82% of materials along the alignment, while more lithified Quaternary deposits make up about 10%. Solid bedrock units encountered include about 3% sandstones/conglomerates, 1% metamorphic rock, and 5% granitic rock. Most of the granitic materials encountered are located in Reche Canyon, which is characterized by shallow bedrock and boulders weathered out of bedrock.

## 4.0 CONCLUSIONS AND RECOMMENDATIONS

The final fault crossing locations identified on Table 2 represent our best estimates of the location of the main trace and major secondary faults associated with the San Andreas and San Jacinto fault zones. Previously published map traces were evaluated and compared against the modern lidar topographic data and historic aerial photographs. The preferred fault traces identified in Table 2 include those fault strands that were assessed to be the most accurately located (where multiple mapped interpretations existed) and also instances where mapping for this project had been improved using the lidar. Therefore, this study refined the fault crossing locations utilizing highly detailed topographic data (lidar) that was not available to many of the original sources.



Based on the short recurrence intervals and amount of elapsed time since the most recent events, there is a high likelihood that the pipeline alignment fault crossings identified at the San Andreas Fault (ADM-SAF-LCI\_Preferred-02 and -03) and at the San Jacinto fault zone (ADM-SJF-LCI\_Preferred-01) may experience large-magnitude, ground-rupturing earthquakes during the design lifetime of the proposed pipeline.

We recommend that dominantly right-lateral displacement for the San Andreas Fault crossing on the order of 5 m (16 ft) and dominantly right-lateral displacement for the San Jacinto fault crossing on the order of 4 m (13 ft) be considered in pipeline engineering and design. It appears the San Andreas fault zone will accommodate this displacement across a roughly 450-ft-wide zone with displacement concentrated on the primary fault (ADM-SAF-LCI\_Preferred03) and a secondary fault (ADM-SAF-LCI\_Preferred-02) bounding this zone. It appears that the San Jacinto fault zone will accommodate displacement along a much narrower zone, perhaps a single strand (ADM-SJF-LCI\_Preferred-01), on the basis of faulting exposed in trenches excavated by Leighton and Associates (1980). We recommend that dominantly left-lateral displacement at the Cleghorn fault crossing on the order of 1 m (3.3 ft) be considered in pipeline engineering and design.

It is anticipated that an unknown amount of surface displacement may occur on secondary faults within the San Andreas and San Jacinto fault zones. Future right-lateral displacements along these secondary strands would be significantly less than displacement on primary faults. We recommend that a conservative estimate of displacements of up to 1 m (3 ft) be considered in pipeline engineering and design. In most cases, secondary fault displacements will likely be much less than this 1 m (3 ft) upper-bound estimate.



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CROSSING ID	STATE PLANE X (FT)	STATE PLANE Y (FT)	LONGITUDE	LATITUDE	FAULT NAME	SENSE OF SLIP	AGE	SOURCE	COMMENTS		
SAN ANDREAS FAULT CROSSINGS											
ADM-SAF-AP-01	6728281.18	1923484.08	-117.4485	34.2766	San Andreas Fault	right-lateral strike-slip	Historic	CDMG, 1974 (Cajon Quad)			
ADM-SAF-AP-02	6727599.74	1922335.67	-117.4508	34.2734	San Andreas Fault	right-lateral strike-slip	Historic	CDMG, 1974 (Cajon Quad)			
ADM-SAF-AP-03	6725999.48	1920193.65	-117.4561	34.2676	San Andreas Fault	right-lateral strike-slip	Historic	CDMG, 1974 (Cajon Quad)			
ADM-SAF-AP-04	6724311.74	1919652.66	-117.4617	34.2661	San Andreas Fault	right-lateral strike-slip	Historic	CDMG, 1974 (Cajon Quad)			
ADM-SAF-AP-05	6723312.43	1919167.72	-117.4650	34.2648	Punchbowl (SAF)	right-lateral strike-slip	Historic	CDMG, 1974 (Cajon Quad)			
ADM-SAF- Bedrossian-01	6728393.67	1923768.11	-117.4481	34.2773	San Andreas Fault	right-lateral strike-slip	Historic	Bedrossian, et al., 2012			
ADM-SAF- Bedrossian-02	6728246.09	1923395.96	-117.4486	34.2763	San Andreas Fault	right-lateral strike-slip	Historic	Bedrossian, et al., 2012			
ADM-SAF- Bedrossian-04	6727084.30	1921052.01	-117.4525	34.2699	San Andreas Fault	right-lateral strike-slip	Historic	Bedrossian, et al., 2012	projected 65 feet from west		
ADM-SAF- Bedrossian-05	6726003.09	1920195.69	-117.4561	34.2676	San Andreas Fault	right-lateral strike-slip	Historic	Bedrossian, et al., 2012			
ADM-SAF- Bedrossian-06	6725894.92	1920135.88	-117.4564	34.2674	San Andreas Fault	right-lateral strike-slip	Historic	Bedrossian, et al., 2012			
ADM-SAF- Bedrossian-07	6724237.18	1919644.74	-117.4619	34.2661	San Andreas Fault	right-lateral strike-slip	Historic	Bedrossian, et al., 2012			
ADM-SAF- Bedrossian-08	6724078.56	1919629.16	-117.4625	34.2660	San Andreas Fault	right-lateral strike-slip	Historic	Bedrossian, et al., 2012			
ADM-SAF- Bedrossian-09	6723330.31	1919189.79	-117.4649	34.2648	Punchbowl (SAF)	right-lateral strike-slip	Historic	Bedrossian, et al., 2012			
ADM-SAF- Bedrossian-10	6733834.53	1909210.77	-117.4304	34.2373	San Andreas Fault	right-lateral strike-slip	Historic	Bedrossian, et al., 2012			
ADM-SAF-CGS-01	6725950.73	1920166.30	-117.4563	34.2675	San Andreas Fault	right-lateral strike-slip	Historic	Jennings and Bryant, 2010			
ADM-SAF-Dibblee-01	6727115.06	1921095.19	-117.4524	34.2700	San Andreas Fault	right-lateral strike-slip	Historic	Dibblee, 2003 (Cajon quad)			
ADM-SAF-Dibblee-02	6726170.39	1920292.04	-117.4555	34.2678	San Andreas Fault	right-lateral strike-slip	Historic	Dibblee, 2003 (Cajon quad)			
ADM-SAF-Dibblee-03	6723658.44	1919510.01	-117.4638	34.2657	Punchbowl (SAF)	right-lateral strike-slip	Historic	Dibblee, 2003 (Cajon Quad)			
ADM-SAF-Dibblee-04	6723370.94	1919240.46	-117.4648	34.2650	Punchbowl (SAF)	right-lateral strike-slip	Historic	Dibblee, 2003 (Cajon Quad)			
ADM-SAF- LCI_Preferred-01	6726981.86	1920933.61	-117.4528	34.2696	San Andreas Fault	right-lateral strike-slip	Historic	LCI, this study			
ADM-SAF- LCI_Preferred-02	6726491.18	1920512.06	-117.4544	34.2684	San Andreas Fault	right-lateral strike-slip	Historic	LCI, this study			
ADM-SAF- LCI_Preferred-03	6726098.68	1920249.35	-117.4558	34.2677	San Andreas Fault	right-lateral strike-slip	Historic	LCI, this study			

**Table 1.** Compilation of fault crossings along the proposed North-South Pipeline alignment.



CROSSING ID	STATE PLANE X (FT)	STATE PLANE Y (FT)	LONGITUDE	LATITUDE	FAULT NAME	SENSE OF SLIP	AGE	SOURCE	COMMENTS
ADM-SAF-LCI-01	6728276.99	1923473.37	-117.4485	34.2765	San Andreas Fault	right-lateral strike-slip	Historic	LCI, this study	
ADM-SAF-LCI-02	6754274.63	1892797.74	-117.3631	34.1918	San Andreas Fault	right-lateral strike-slip	Historic	LCI, this study	
ADM-SAF-LCI-03	6754830.19	1892525.27	-117.3612	34.1911	San Andreas Fault	right-lateral strike-slip	Historic	LCI, this study	
ADM-SAF-USGS-01	6733809.15	1909227.30	-117.4304	34.2373	San Andreas Fault	right-lateral strike-slip	Historic	USGS and CGS, 2006	
SAN JACINTO FAULT	CROSSINGS						L		I
ADM-SJF-AP-01	6780827.10	1842912.66	-117.2765	34.0542	San Jacinto fault	right-lateral strike-slip	Historic	CDMG, 1977 (San Bernardino So. Quad)	
ADM-SJF-AP-02	6779533.95	1841872.97	-117.2808	34.0514	San Jacinto fault	right-lateral strike-slip	Historic	CDMG, 1977 (San Bernardino So. Quad)	
ADM-SJF-AP-03	6778944.81	1841680.21	-117.2827	34.0509	San Jacinto fault	right-lateral strike-slip	Historic	CDMG, 1977 (San Bernardino So. Quad)	
ADM-SJF-AP-04	6778428.24	1841595.70	-117.2844	34.0507	San Jacinto fault	right-lateral strike-slip	Historic	CDMG, 1977 (San Bernardino So. Quad)	
ADM-SJF-Bedrossian- 01	6784107.57	1846140.49	-117.2656	34.0631	San Jacinto fault	right-lateral strike-slip	Historic	Bedrossian, et al., 2012	
ADM-SJF-Bedrossian- 02	6779342.37	1841805.66	-117.2814	34.0512	San Jacinto fault	right-lateral strike-slip	Historic	Bedrossian, et al., 2012	
ADM-SJF-Bedrossian- 03	6779137.39	1841730.22	-117.2821	34.0510	San Jacinto fault	right-lateral strike-slip	Historic	Bedrossian, et al., 2012	
ADM-SJF-Bedrossian- 04	6778565.85	1837003.70	-117.2841	34.0381	San Jacinto fault	right-lateral strike-slip	Historic	Bedrossian, et al., 2012	
ADM-SJF-CGS-01	6784951.40	1846145.29	-117.2628	34.0630	San Jacinto fault	right-lateral strike-slip	Historic	Jennings and Bryant, 2010	
ADM-SJF-CGS-02	6778483.35	1837498.40	-117.2843	34.0394	San Jacinto fault	right-lateral strike-slip	Historic	Jennings and Bryant, 2010	
ADM-SJF-Dibblee-01	6778827.52	1841654.00	-117.2831	34.0508	San Jacinto fault	right-lateral strike-slip	Historic	Dibblee, 2004 (San Bernardino So. quad)	
ADM-SJF- LCI_Preferred-01	6779117.22	1841723.05	-117.2821	34.0510	San Jacinto fault	right-lateral strike-slip	Historic	LCI, this study	
ADM-SJF- LCI_Preferred-02	6778480.88	1837530.88	-117.2843	34.0395	San Jacinto fault	right-lateral strike-slip	Historic	LCI, this study	
ADM-SJF-LCI-01	6783455.43	1846136.79	-117.2677	34.0631	San Jacinto fault	right-lateral strike-slip	Historic	LCI, this study	
ADM-SJF-LCI-02	6779010.83	1841694.97	-117.2825	34.0509	San Jacinto fault	right-lateral strike-slip	Historic	LCI, this study	
ADM-SJF-LCI-03	6778455.00	1837855.19	-117.2844	34.0404	San Jacinto fault	right-lateral strike-slip	Historic	LCI, this study	
ADM-SJF-USGS-01	6784174.42	1846140.87	-117.2653	34.0631	San Jacinto fault	right-lateral strike-slip	Historic	USGS and CGS, 2006	
ADM-SJF-USGS-02	6778527.49	1837190.00	-117.2842	34.0386	San Jacinto fault	right-lateral strike-slip	Historic	USGS and CGS, 2006	



CROSSING ID	STATE PLANE X (FT)	STATE PLANE Y (FT)	LONGITUDE	LATITUDE	FAULT NAME	SENSE OF SLIP	AGE	SOURCE	COMMENTS
OTHER FAULT CROS	SINGS								
ADM-Banning- Dibblee-01	6785819.01	1858719.21	-117.2596	34.0976	Banning fault	right-lateral strike-slip/oblique right-reverse	Holocene	Dibblee, 2004 (San Bernardino South Quad)	inferred buried trace based gravity data of Williangham, 1981
ADM-BrA-Bedrossian- 01	6719088.25	1945447.97	-117.4785	34.3370	unnamed bedrock fault A	thrust	unknown	Bedrossian, et al., 2012	
ADM-BrA-Bedrossian- 02	6721023.86	1939233.79	-117.4722	34.3199	unnamed bedrock fault A	thrust	unknown	Bedrossian, et al., 2012	
ADM-BrB-Bedrossian- 01	6728651.90	1926172.16	-117.4472	34.2839	unnamed bedrock fault B	unknown	unknown	Bedrossian, et al., 2012	
ADM-BrC-Bedrossian- 01	6794520.10	1823581.26	-117.2317	34.0009	unnamed bedrock fault C	unknown	unknown	Bedrossian, et al., 2012	field observations suggest this is not a fault
ADM-BrD-Bedrossian- 01	6805882.93	1812166.66	-117.1946	33.9692	unnamed bedrock fault D	unknown	unknown	Bedrossian and Roffers, 2012	inferred bedrock fault
ADM-BrD-Bedrossian- 02	6807787.89	1810592.78	-117.1883	33.9649	unnamed bedrock fault D	unknown	unknown	Bedrossian and Roffers, 2012	inferred bedrock fault
ADM-BrD-Bedrossian- 03	6810762.20	1807845.49	-117.1786	33.9573	unnamed bedrock fault D	unknown	unknown	Bedrossian and Roffers, 2012	inferred bedrock fault
ADM-Cleghorn- Bedrossian-01	6727041.10	1930387.87	-117.4525	34.2955	Cleghorn fault	left-lateral strike-slip	Holocene(?)	Bedrossian, et al., 2012	
ADM-Cleghorn-CGS- 01	6725901.51	1931883.58	-117.4562	34.2997	Cleghorn fault	left-lateral strike-slip	Holocene(?)	Jennings and Bryant, 2010	
ADM-Cleghorn-LCI-01	6725913.39	1931869.30	-117.4562	34.2996	Cleghorn fault	left-lateral strike-slip	Holocene(?)	LCI, this study	
ADM-Cleghorn- USGS-01	6725795.06	1932013.82	-117.4566	34.3000	Cleghorn fault	left-lateral strike-slip	Holocene(?)	USGS and CGS, 2006	
ADM-FFZ-Dibblee-01	6726341.64	1931345.73	-117.4548	34.2982	Frontal Fault Zone	thrust	Holocene to Quaternary	Dibblee, 2003 (Cajon Quad)	
ADM-FFZ-Dibblee-02	6728564.61	1925108.51	-117.4475	34.2810	Frontal Fault Zone	thrust	Holocene to Quaternary	Dibblee, 2003 (Cajon Quad)	
ADM-FFZ-Dibblee-03	6728244.90	1923392.98	-117.4486	34.2763	Frontal Fault Zone	thrust	Holocene to Quaternary	Dibblee, 2003 (Cajon Quad)	
ADM-FFZ-Dibblee-04	6727768.39	1923087.75	-117.4502	34.2755	Frontal Fault Zone	thrust	Holocene to Quaternary	Dibblee, 2003 (Cajon Quad)	



**Table 2.** Hazardous fault crossings along the proposed North-South Pipeline alignment.

CROSSING ID	STATE PLANE X (FT)	STATE PLANE Y (FT)	LONGITUDE	LATITUDE	FAULT NAME	SENSE OF SLIP	AGE	PRIMARY (SLIP RATE) OR SECONDARY STRAND?	EST. DISPLACEMENT	ANGLE OF	TYPE OF ANTICIPATED PIPE DEFORMATION	SHOWN IN FIGURE	SOURCE FOR LOCATION
ADM- Cleghorn- USGS-01	6725795.06	1932013.82	-117.4566	34.3000	Cleghorn	LL	Holocene(?)	Primary (0.3-0.6 mm/yr)	1m	20	tension	3	USGS and CGS, 2006
ADM-SAF-AP- 01	6728281.18	1923484.08	-117.4485	34.2766	SAF	RL	Historic	Secondary	<1m	85	tension	3	CDMG, 1974
ADM-SAF-AP- 02	6727599.74	1922335.67	-117.4508	34.2734	SAF	RL	Historic	Secondary	<1m	75	tension	3	CDMG, 1974
ADM-SAF- LCI_Preferred- 01	6726981.86	1920933.61	-117.4528	34.2696	SAF	RL	Historic	Secondary	<1m	90	tension	5B	LCI, this study
ADM-SAF- LCI_Preferred- 02	6726491.18	1920512.06	-117.4544	34.2684	SAF	RL	Historic	Secondary	<1m	72	tension	5B	LCI, this study
ADM-SAF- LCI_Preferred- 03	6726098.68	1920249.35	-117.4558	34.2677	SAF	RL	Historic	Primary (20-30 mm/yr)	5 m	65	tension	5B	LCI, this study
ADM-SAF-AP- 04	6724311.74	1919652.66	-117.4617	34.2661	SAF	RL	Historic	Secondary	<1m	45	tension	5B	CDMG, 1974
ADM-SAF-AP- 05	6723312.43	1919167.72	-117.4650	34.2648	SAF - Punchbowl	RL	Historic	Secondary	<1m	90	tension	5B	CDMG, 1974
ADM-SAF- Bedrossian-10	6733834.53	1909210.77	-117.4304	34.2373	SAF – Peters	RL	Historic	Secondary	<1m	33	compression	2	Bedrossian, et al., 2012
ADM-SJF- Bedrossian-01	6784107.57	1846140.49	-117.2656	34.0631	SJF – Loma Linda	RL	Historic	Secondary	<1m	60	tension	4	Bedrossian, et al., 2012
ADM-SJF-AP- 01	6780827.10	1842912.66	-117.2765	34.0542	SJF	RL	Historic	Secondary	<1m	35	tension	8B	CDMG, 1977
ADM-SJF-AP- 02	6779533.95	1841872.97	-117.2808	34.0514	SJF	RL	Historic	Secondary	<1m	75	tension	8B	CDMG, 1977
ADM-SJF- LCI_Preferred- 01	6779060.17	1841788.88	-117.2823	34.0512	SJF	RL	Historic	Primary (2-10 mm/yr)	4m	65	tension	8B	LCI, this study
ADM-SJF-AP- 04	6778428.24	1841595.70	-117.2844	34.0507	SJF	RL	Historic	Secondary	<1m	50	tension	8B	CDMG, 1977
ADM-SJF- LCI_Preferred- 02	6778480.88	1837530.88	-117.2843	34.0395	SJF - Colton	RL	Historic	Secondary	<1m	53	compression	4	LCI, this study





Table 3. Summa	ry of aerial photo	graphs reviewec	I for this study.	
FLIGHT	DATE	SCALE	FRAME NUMBERS	FORMAT
C-910	3/25/1930	1:24,000	78; 92; 88; 101	black & white 7"x9" scanned print
C-1940D	2/22/1932	1:14,400	14; 15	black & white 7"x9" scanned print
AXL-1938	1938	1:20,000	63-81; 79-25; 79-27; 61-23 61-25; 65-85; 67-47	black & white 7"x9" scanned print
AXM-1938A	1938	1:20,000	36-40; 53-98; 36-73; 36-75	black & white 7"x9" scanned print
C-8305	3/12/1943	1:14,400	1; 26; 30	black & white 9"x9" scanned print



**Table 4.** Summary of San Andreas and San Jacinto fault earthquake recurrence information from nearby trench sites.

SITE NAME NO. OF EVENTS		OLDEST EVENT	MOST RECENT EVENT	RECURRENCE RANGE (YEARS)	AVERAGE RECURRENCE (YEARS)	
SAN ANDREAS FAULT						
Wrightwood (young sedimentary section)	15	533 A.D.	1857 A.D.	90 to 100	95	
Wrightwood (old sedimentary section)	14	1503 B.C.	2915 B.C.	96 to 124	110	
Pitman Canyon	7	931 A.D.	1812 A.D.	132 to 162	147	
Plunge Creek	3	1499 A.D.	1812 A.D.	100 to 214	157	
SAN JACINTO FAULT						
Mystic Lake	7	712 A.D.	1799 A.D.	150 to 212	181	

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FAULT	MOMENT	AVERAGE DIS		MAXIMUM DISPLACEMENT <sup>a</sup>		
FAULI	MAGNITUDE	meters	feet	meters	feet	
San Andreas	7.8	3.8	12.5	8.6	28.2	
San Jacinto	7.1	1.3	4.3	2.3	7.5	
San Jacinto	7.4	2.0	6.6	4.0	13.1	
Cleghorn	6.6	0.6	2.0	0.9	3.0	

Table 5. Fault displacements for deterministic earthquake scenarios.

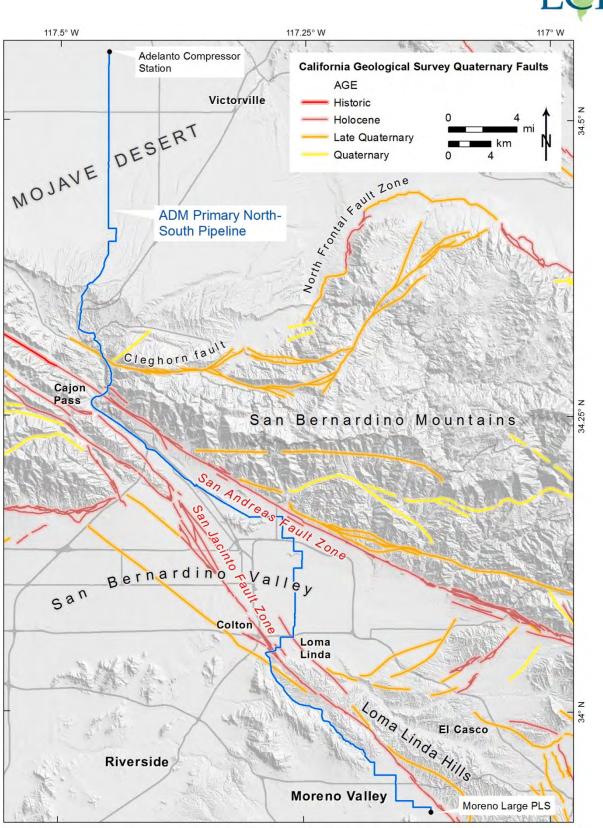
Notes:

a. Calculated using the empirical Wells and Coppersmith (1994) magnitude-average displacement and magnitude-maximum displacement relations for all-slip-types. Displacements rounded to the nearest tenth in meters and feet.

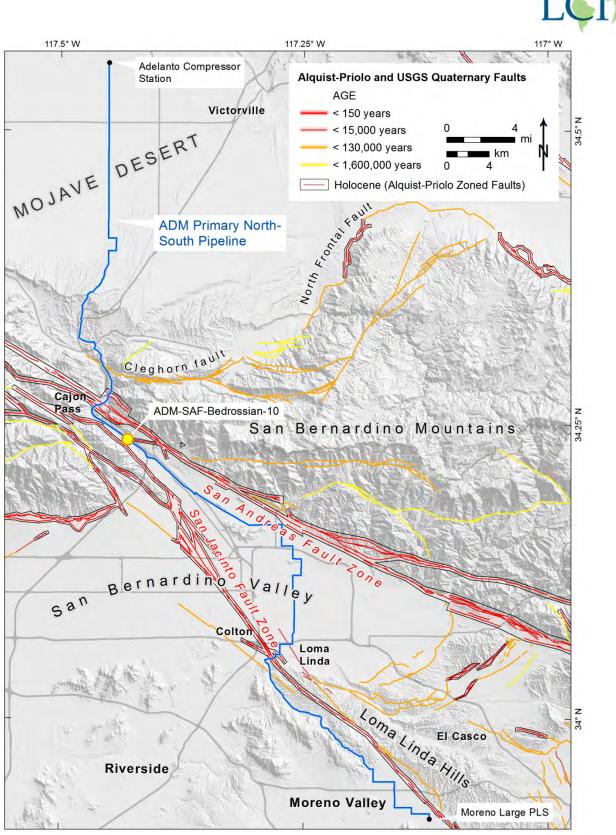


**Table 6.** Geologic materials encountered along the proposed North-South Pipeline alignment.

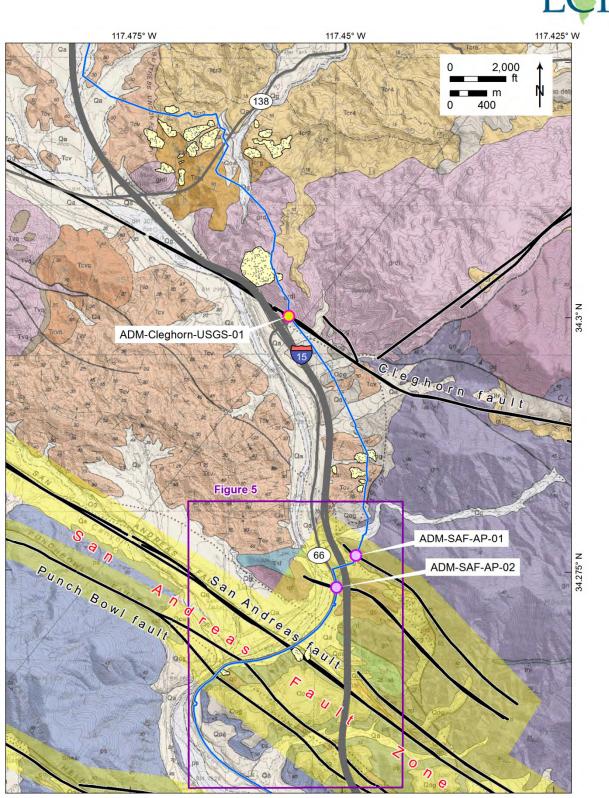
MAP SYMBOL	UNIT NAME	AGE	PERCENT OF ALIGNMENT LENGTH	COMMENTS
af	Artificial Fill	Late Holocene	0.3%	
Qa	Alluvial Valley Deposits	Late Holocene	14.0%	includes 3.85 miles at N end of alignment not mapped, but considered Qa
Qf	Alluvial Fan Deposits	Late Holocene	13.5%	
Qw	Alluvial Wash Deposits	Late Holocene	14.7%	
Qy	Young Alluvium - undifferentiated	Holocene to Late Pleistocene	39.8%	
Qo	Old Alluvium - undifferentiated	Late to Middle Pleistocene	6.7%	
Qoa	Old Alluvial Valley Deposits	Late to Middle Pleistocene	0.2%	
Qof	Old Alluvial Fan Deposits	Late to Middle Pleistocene	2.3%	
Qvo	Very Old Alluvium - undifferentiated	Middle to Early Pleistocene	0.3%	
Tss	Coarse-grained sedimentary formations	Tertiary	2.5%	primarily sandstone and conglomerate
pKm	Metamorphic formations	Cretaceous and older	0.7%	sedimentary and volcanic origin
gr	Granitic and other intrusive crystalline rocks	Cretaceous and older	4.8%	



**Figure 1.** Proposed North-South Pipeline alignment and Quaternary faults published by California Geological Survey (Jennings and Bryant, 2010).

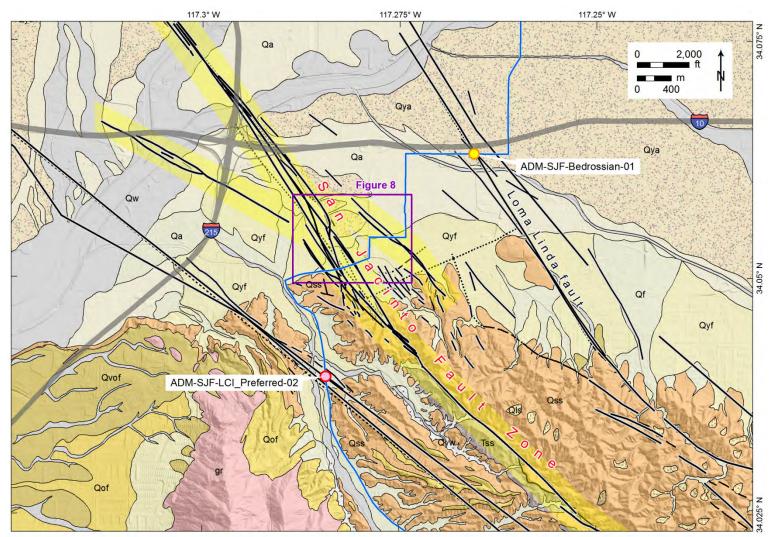


**Figure 2.** Location Map of proposed North-South Pipeline alignment with AP Earthquake Fault Zones and Quaternary faults published by United States Geological Survey (USGS and CGS, 2006).



**Figure 3.** Proposed North-South Pipeline alignment fault crossings of the San Andreas fault zone at Cajon Pass. Base geologic map from Dibblee and Minch (2003). Yellow shading depict AP Earthquake Fault Zones; yellow stippled polygons are landslides mapped by LCI for this study; faults from CGS (1974a) and USGS and CGS (2006).





**Figure 4.** Proposed North-South Pipeline alignment crossings of the San Jacinto fault zone in the southern San Bernardino Valley. Base geologic map from Bedrossian et al. (2012). Yellow shading depicts AP Earthquake Fault Zones, faults from AP and USGS and CGS (2006).



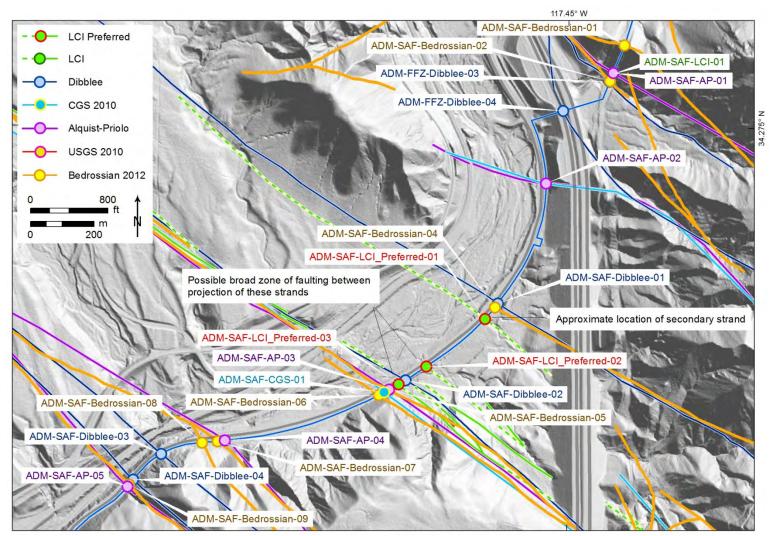
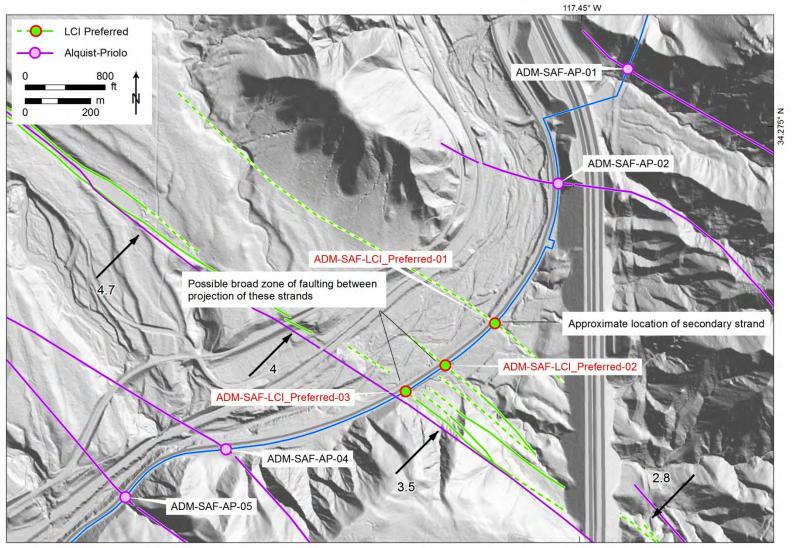


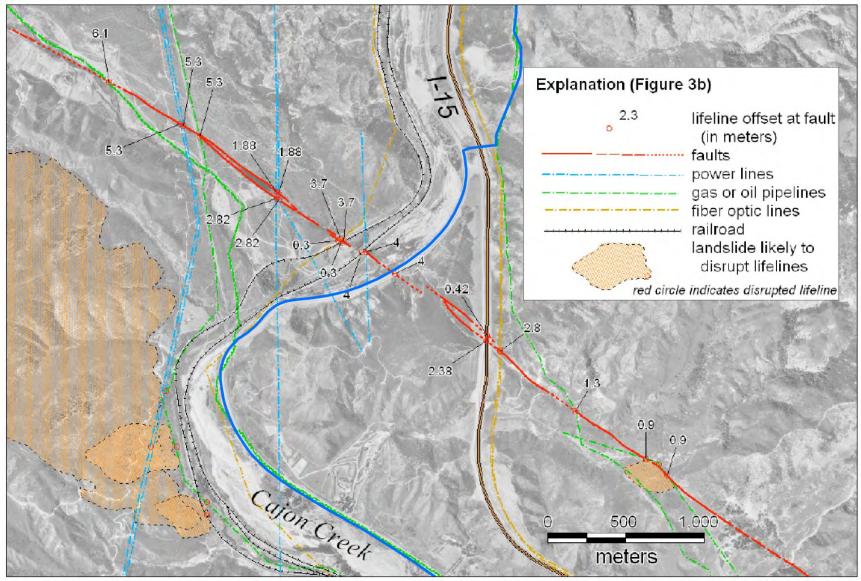
Figure 5a. All fault crossings of the proposed North-South Pipeline alignment through Cajon Pass.





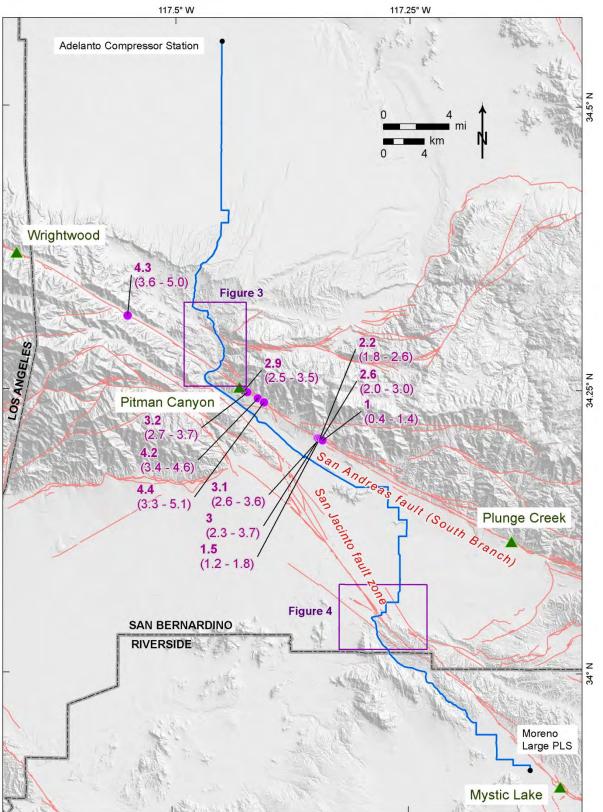
**Figure 5b.** Hazardous fault crossing locations from Table 2 along the proposed North-South Pipeline alignment through Cajon Pass. Also shown with arrows are modeled displacement values from the ShakeOut Scenario (values in meters).





**Figure 6.** Distribution of modeled ShakeOut displacement estimates in the Cajon Pass crossing area, modified from Figure 3b of Trieman et al. (2008). Proposed North-South Pipeline alignment shown with thick blue line.





**Figure 7.** Displacement measurements from offset geomorphic features in lidar from (purple dots from Madden et al., 2013) and paleoseismic sites (green triangles). Displacement values in meters; bold values represent best estimates and values in parentheses represent uncertainty range.



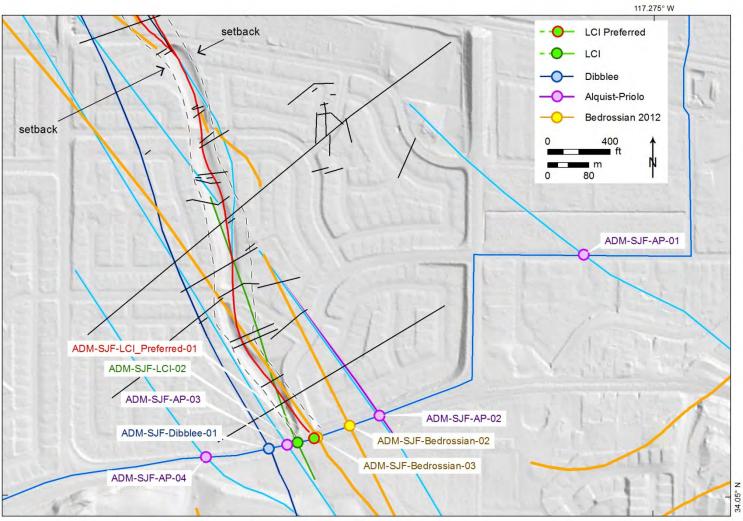


Figure 8a. All San Jacinto fault crossings of the proposed North-South Pipeline alignment near Loma Linda. Black lines are trenches; red line is fault depicted by Leighton and Associates (1980).



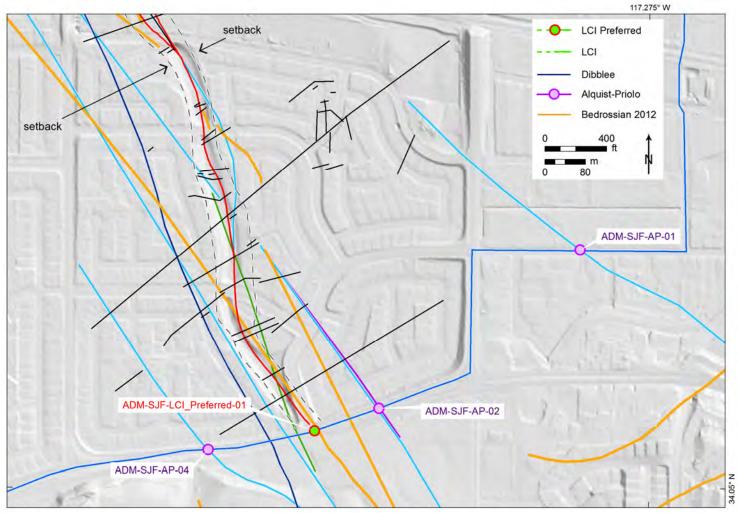


Figure 8b. Hazardous San Jacinto fault crossings from Table 2 of the proposed North-South pipeline alignment.



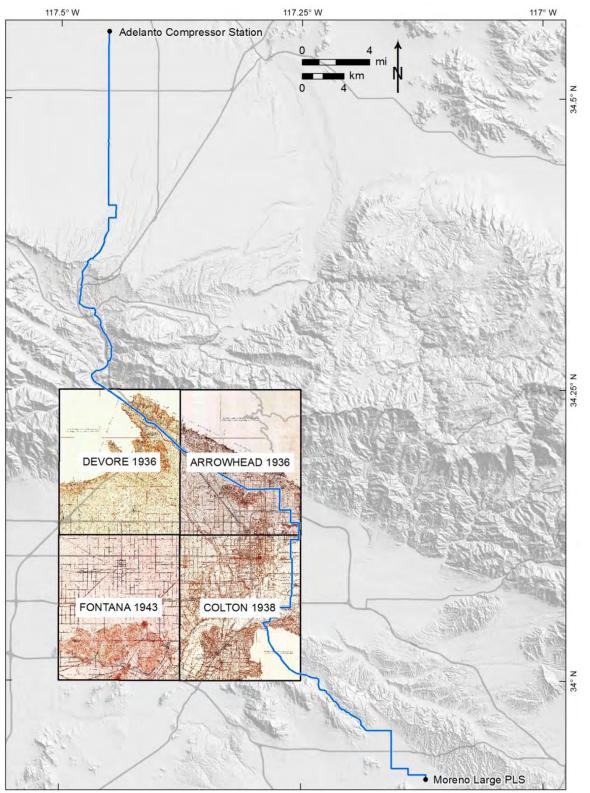
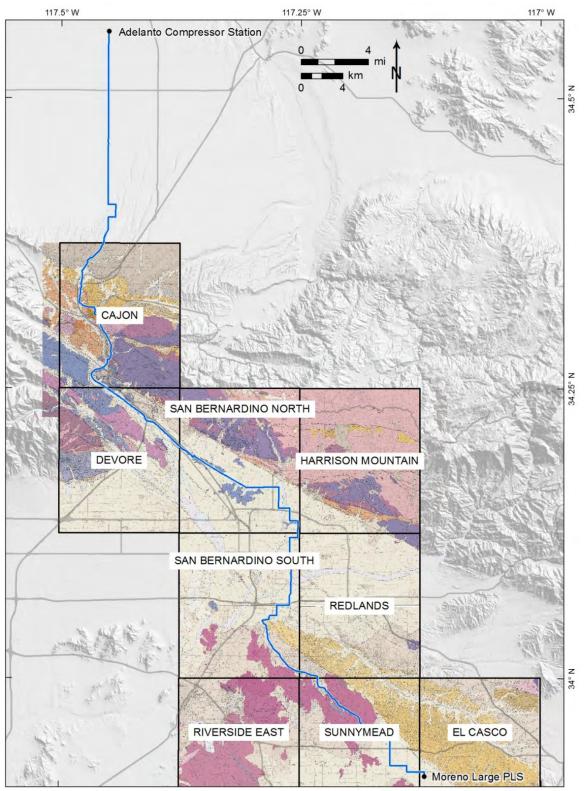


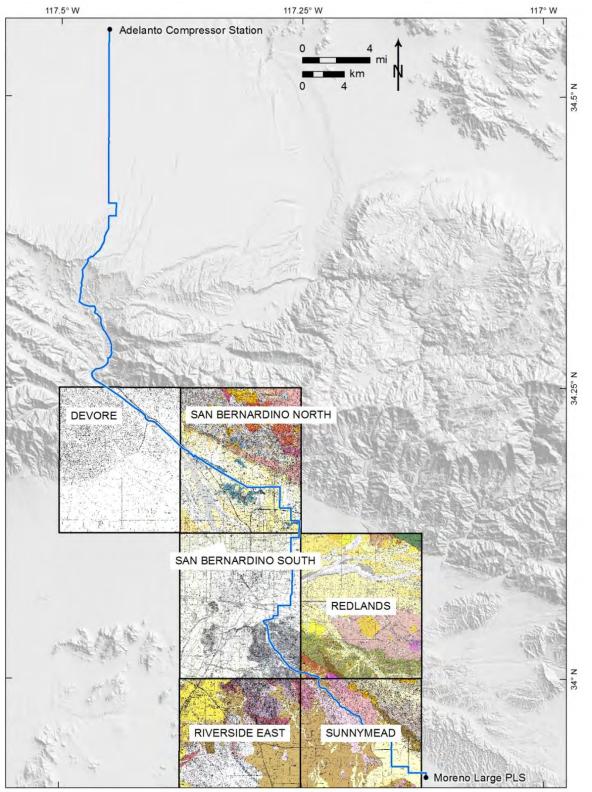
Figure 9. Coverage of available historic (1930's) topographic maps in the vicinity of the proposed North-South Pipeline alignment.





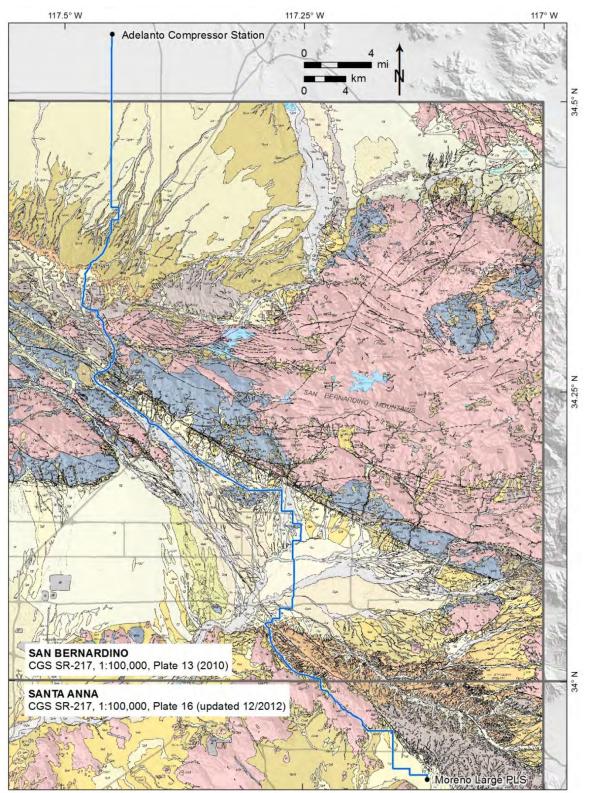
**Figure 10.** Available coverage of Dibblee Foundation geologic maps (1:24,000 scale) along the proposed North-South Pipeline alignment.





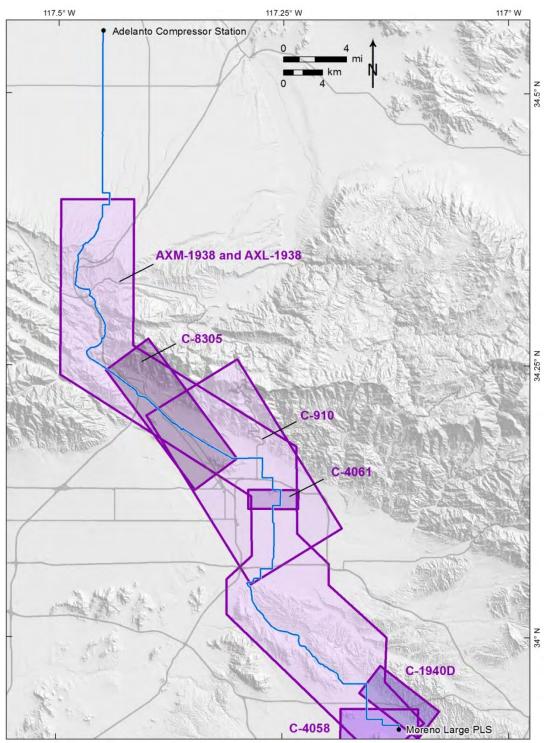
**Figure 11.** Available coverage of USGS geologic maps (1:24,000 scale) along the proposed North-South Pipeline alignment.





**Figure 12.** Available coverage of USGS surficial geologic maps (1:100,000 scale) along the proposed North-South Pipeline alignment.





**Figure 13.** Available coverages of historic (pre-1945) aerial photograph flights along the proposed North-South Pipeline alignment.



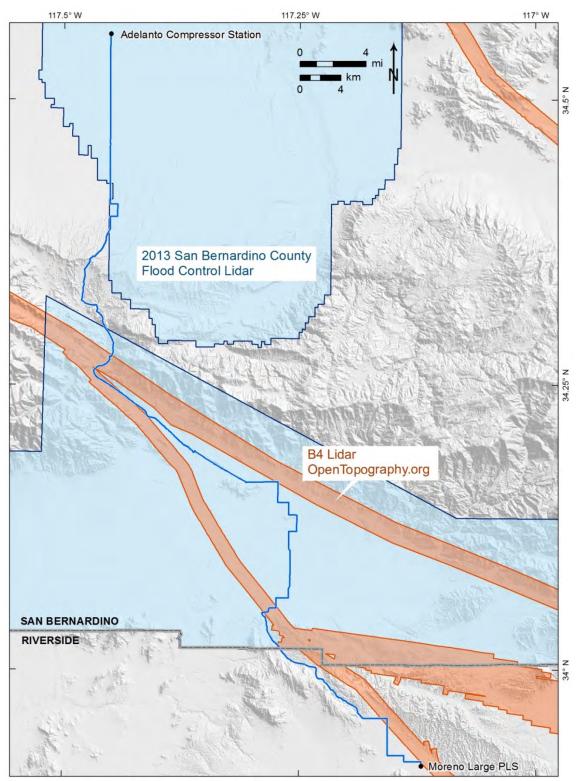
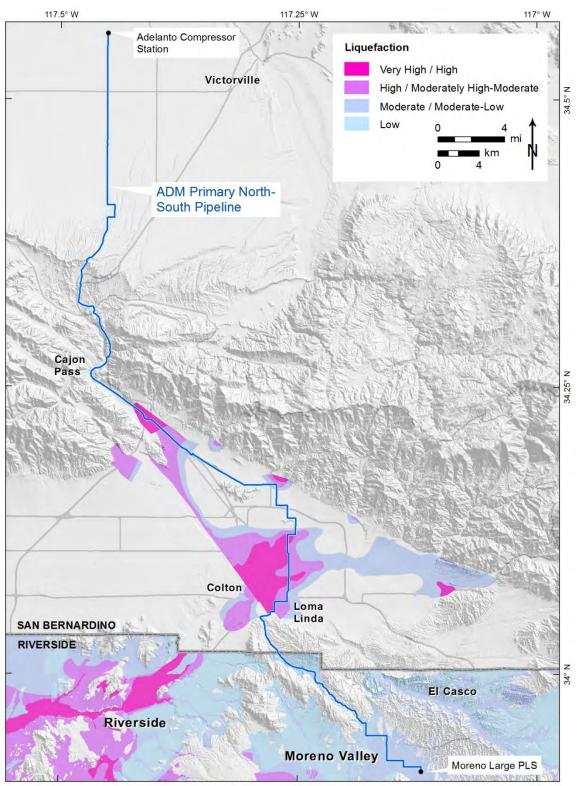


Figure 14. LiDAR coverage available in the vicinity of the proposed North-South Pipeline alignment.





**Figure 15.** Compilation of liquefaction hazards from Matti and Carson (1991) in San Bernardino County and the County of Riverside (2014) in Riverside County.



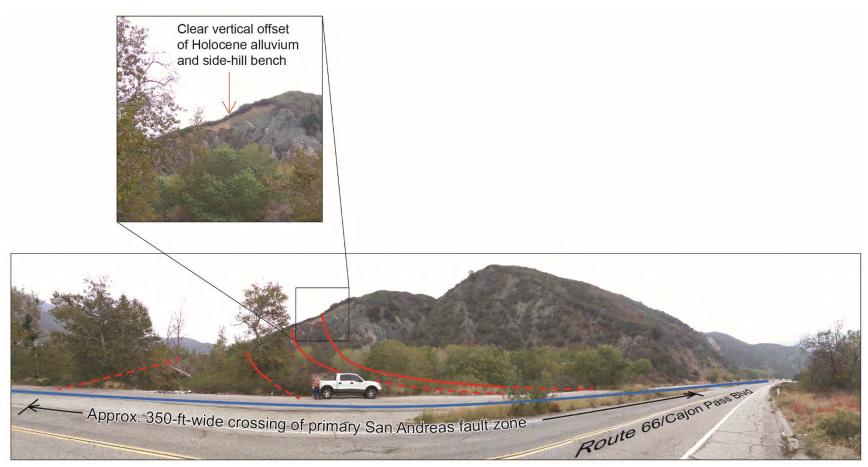


Figure 16. Annotated photo of San Andreas Fault observed along Route 66/Cajon Pass Boulevard

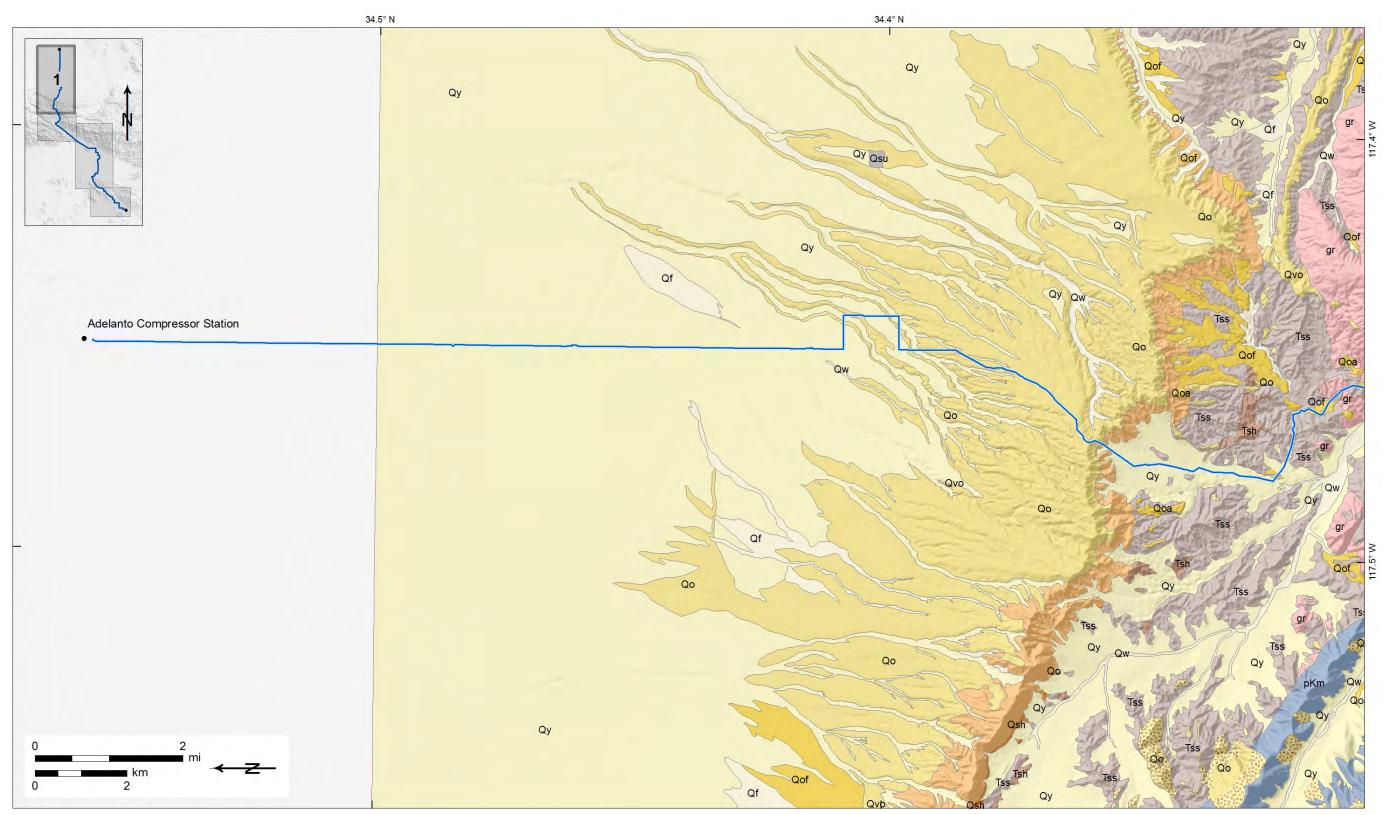
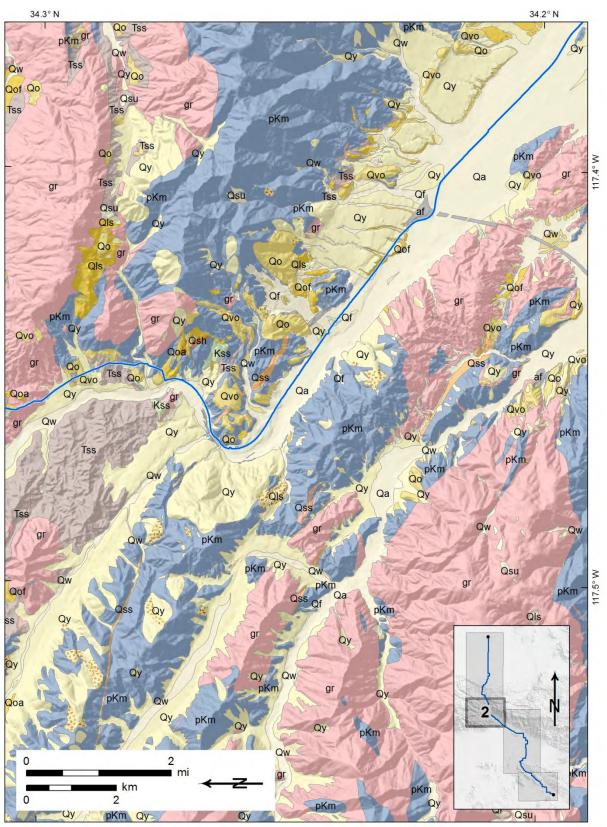


Figure 17. Geologic units along the proposed North-South Pipeline alignment (Panel 1 of 4; legend on Figure 21). Geologic basemap modified from Bedrossian et al. (2012).







**Figure 18.** Geologic units along the proposed North-South Pipeline alignment (Panel 2 of 4; legend on Figure 21). Geologic basemap modified from Bedrossian et al. (2012).

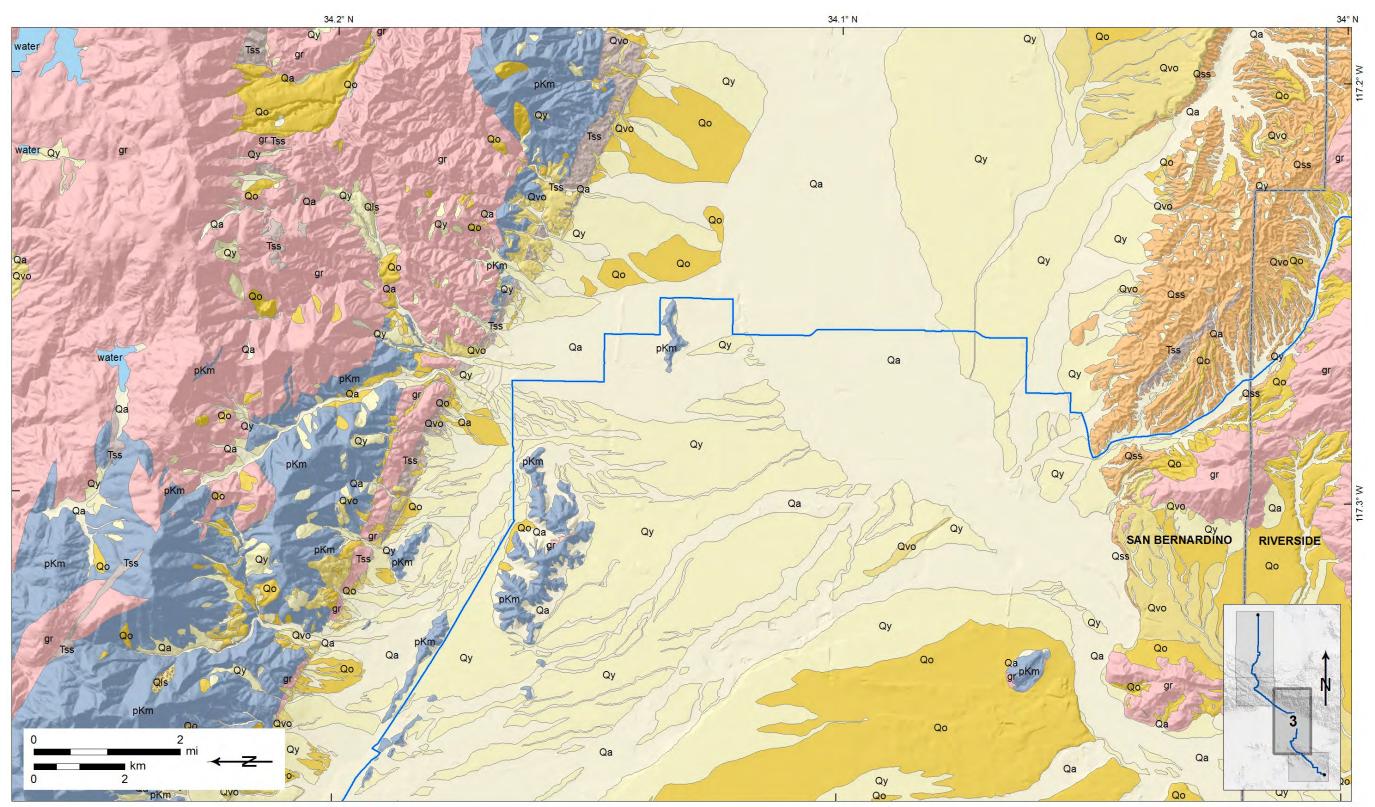
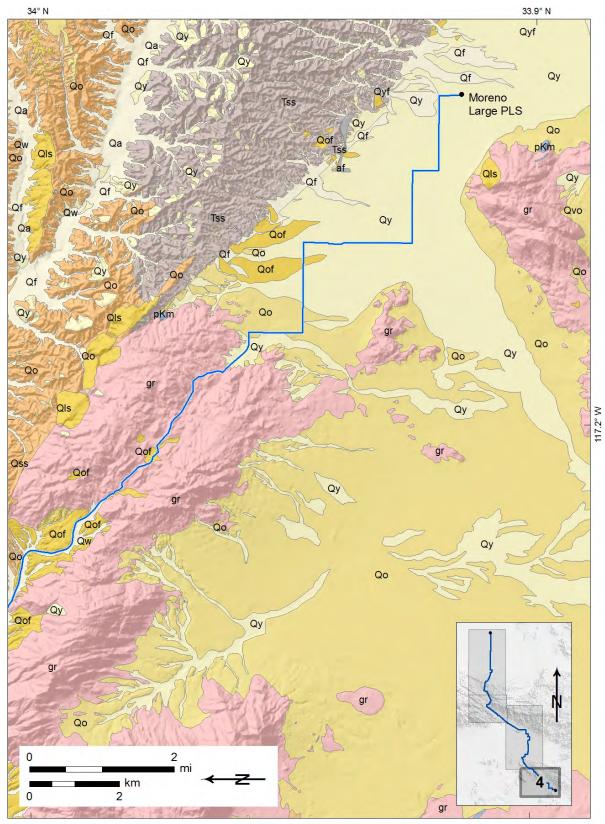


Figure 19. Geologic units along the proposed North-South Pipeline alignment (Panel 3 of 4; legend on Figure 21). Geologic basemap modified from Bedrossian et al. (2012).







**Figure 20.** Geologic units along the proposed North-South Pipeline alignment (Panel 4 of 4; legend on Figure 21). Geologic basemap modified from Bedrossian et al. (2012).



#### **Surficial Deposits**

af	Artificial fill
Qy	Young alluvial deposits
Qsu	Undifferentiated deposits
QIS	Landslide deposits
Qw	Alluvial wash deposits
Qf	Alluvial fan deposits
Q	Alluvial valley deposits Lacustrine, playa, and estuarine deposits
Qe	Eolian and dune deposits
Qyf	Young alluvial fan deposits
Qo	Old alluvial deposits
Qof	Old alluvial fan deposits
Qoa	Old alluvial valley deposits
Qoe	Old eolian and dune deposits
Qvo	Very old alluvial deposits
Qss	Quaternary bedrock; coarse-grained formations of Pleistocene age and younger
Qsh	Quaternary bedrock; fine-grained formations of Pleistocene age and younger
Tss	Tertiary bedrock; coarse-grained Tertiary age formations
Tsh	Tertiary bedrock; fine-grained Tertiary age formations
, "Tv" -	Tertiary bedrock; Tertiary age formations of volcanic origin
Kss	Cretaceous coarse-grained age formations of sedimentary origin
Kv	Cretaceous age formations of volcanic origin
pKm	Cretaceous and pre-Cretaceous metamorphic formations of sedimentary and volcanic origin.
gr	Mesozoic and older bedrock; Granitic and other intrusive crystalline rocks of all ages

**Figure 21.** Legend for geologic units depicted in Figures 17 through 20. Modified from Bedrossian et al. (2012)



# Attachment 1 Liquefaction Induced Settlement Analysis



#### **Earthquake and Seismic Design Considerations**

The project site is located within a municipality that employs the 2013 California Building Code (CBC). As a part of this code, the design of structures must consider dynamic forces resulting from seismic events. These forces are dependent upon the magnitude of the earthquake event as well as the properties of the soils that underlie the site. Within the procedure to evaluate seismic forces, the code requires an evaluation of the Seismic Site Class, which categorizes the site by Vs30, the average shear-wave velocity (Vs) in the upper 30 m (100 ft) below the ground surface.

#### Site Class

In this study, data obtained from the Standard Penetration Test (SPT) presented in Table 8 of Matti and Carson (1991) are used to evaluate Vs30. To estimate Vs30 from SPT, the recommended Vs-SPT correlation for Quaternary Sands in Equation 4.77 of Wair et al. (2012) is used:

$$Vs = 30 \times (N_1)_{60}^{0.23} \times \sigma'_{vo}^{0.25}$$

where Vs is for Quaternary Sand in m/s,  $(N_1)_{60}$ , is the corrected SPT blow count number, and  $\sigma'_{vo}$  is the effective vertical overburden stresses. The effect of the geologic age is incorporated by the use of age scaling factors. Wair et al. (2012) recommended age scaling factors of 0.9 for Holocene soils and 1.17 for Pleistocene soil deposits.

Matti and Carson (1991) used the stratigraphic and geotechnical data from a drilling investigation in the San Bernardino Valley (Carson et al., 1986) to evaluate liquefaction susceptibility in the region. They evaluated the liquefaction susceptibility for SPT samples collected by Carson et al. (1986). The parameters used by Matti and Carson (1991) to evaluate liquefaction susceptibility for SPT data of Carson et al. (1986) are shown in Table 8 of Matti and Carson (1991). The location of drill sites in Carson et al. (1986) investigation is shown in Figure 1.

Matti and Carson (1991) corrected the raw SPT numbers of Carson et al. (1986) to compensate for effective overburden pressure, for silty or partially silty materials, and for differences in the length and type of the rod used to drive the sampler during the penetration test. However, they did not correct the SPT numbers to an energy ratio of 60% (the average ratio of the actual energy delivered by safety hammers to the theoretical free-fall energy). The correction factor for energy ratio is defined in Kramer et al. (1996) as:

$$(N_1)_{60} = N_1 \frac{E_m}{0.6E_{ff}}$$

where  $(N_1)_{60}$  is the corrected SPT number including the energy ratio correction,  $N_1$  is the corrected SPT number excluding the energy ratio correction,  $E_m$  is the actual hammer energy, and  $E_{ff}$  is the theoretical free-fall hammer energy.

Matti and Carson (1991) provided the  $N_1$ . They applied no energy ratio correction to the data. However, they stated that the USGS system used for SPT actually has a driving efficiency of 68 percent. Therefore,  $E_m = 0.68E_{ff}$  and  $(N_1)_{60} = 1.13N_1$ . In this study, a correction factor of 1.13 is applied to  $N_1$  values reported in Matti and Carson (1991) to obtain  $(N_1)_{60}$ .



In this study, values of  $\sigma'_{vo}$  calculated in Matti and Carson (1991) are used for SPT data. Using  $(N_1)_{60}$  and  $\sigma'_{vo}$ , Vs for each of the SPT sample data in Matti and Carson (1991) are calculated using Equation 4.77 of Wair et al. (2012). The calculation of Vs for each SPT sample is tabulated in Appendix A of this report and shown as function of depth below the ground surface in Figure 2. A linear regression using the power law functional form is performed to estimate Vs as a function of depth and also shown in Figure 2.

Vs data in this study do not extend to a depth of 30 m (Figure 2). Therefore, an extrapolation of shallow velocity data is required to estimate Vs30 (Wair et al., 2012). Boore (2004) proposed an extrapolation method based on statistical analysis of borehole data in California. The Boore (2004) model involves a correlation between Vs30 and the time-averaged Vs to the terminal depth of measurement (Vsd). Boore (2004) proposed the following equation:

 $\log Vs30 = a + b \cdot \log Vsd$ 

The SPT data used in this study extend to the depth of about 10 m. We used the Vs-depth relation shown in Figure 2 to calculate Vs10. Then we used Boore (2004) model to calculate Vs30 from Vs10.

Using the above procedure, a Vs30 of 197 m/s is calculated for the region. Section 1613.3.2 of CBC 2013 defines the site classes in accordance with Chapter 20 of ASCE 7-10. Based on the calculated Vs30 the site is classified as Site Class D.

#### **Ground Motion Parameters**

The USGS *Custom Hazard Maps* tool (<u>http://geohazards.usgs.gov/hazards/apps/cmaps/</u>) is used to create a peak ground acceleration (PGA) map for a hazard level corresponding to 2% probability of exceedance in 50 years for the San Bernardino region (Figure 3).

The USGS 2008 Interactive Deaggregations tool (<u>http://geohazards.usgs.gov/deaggint/2008/</u>) is used for a site within the region with highest PGA values in Figure 3 to obtain mapped PGA (PGA<sub>mapped</sub>), mapped 5 percent damped response spectral acceleration (Sa) at short periods (S<sub>s</sub>), and mapped 5 percent damped Sa at a period of 1 second (S<sub>1</sub>) at the site corresponding to the same hazard level as follows:

- PGA=1.2052 g
- S<sub>s</sub>=3.1895 g
- S<sub>1</sub>=1.2330 g

The USGS deaggregation tool indicates that the mean event contributing to this level of PGA is an earthquake with moment magnitude ( $M_w$ ) of 7.5.

The short-period site coefficient at 0.2 second,  $F_a$ , and the long-period site coefficient at 1.0 second,  $F_v$ , are calculated as 1.0 and 1.5, respectively from Tables 1613.3.3(1) and 1613.3(2) of CBC 2013. Moreover, from Section 1613.3.3 of CBC 2013 the design 5 percent damped Sa at short periods ( $S_{DS}$ ) and the design 5 percent damped Sa at a period of 1 second ( $S_{D1}$ ) are calculated as 2.13 g and 1.23 g, respectively. Therefore, according to Tables 1613.3.5(1) and 1613.5(2) of CBC 2013, the seismic design category is D.



According to Section 1803.5.12 of the CBC 2013 for seismic design category D, when evaluating the potential for liquefaction and soil strength loss, PGA shall be determined in accordance with Section 11.8.3 of ASCE 7-10 as  $F_{PGA}$ xPGA<sub>mapped</sub>; where  $F_{PGA}$  is a site coefficient from Table 11.8.1 of the ASCE 7-10. For Site Class D and PGA<sub>mapped</sub> $\geq$ 0.5 g,  $F_{PGA}$  is equal to 1. Therefore, the mapped PGA of 1.2052 g can be used for the liquefaction analysis.

#### Liquefaction-Induced Settlement

The methodology introduced by Tokimatsu and Seed (1987) for saturated sands is used to evaluate the potential for liquefaction-induced settlement. This procedure utilizes SPT values for sand layers to obtain an estimate of how much the settlement due to liquefaction may be expected.

In this study, the SPT data from Table 8 of Matti and Carson (1991) is used to estimate liquefactioninduced settlement, which Matti and Carson (1991) obtained from the Carson et al. (1986) drilling investigation in the San Bernardino Valley. Matti and Carson (1991) used data from 22 boreholes drilled in Carson et al. (1986) investigation. The location of drill sites in Carson et al. (1986) investigation is shown in Figure 1. This study utilizes the same SPT data for each borehole as shown in Table 8 of Matti and Carson (1991), and includes the correction from  $N_1$  to  $(N_1)_{60}$  as discussed earlier.

The lithology of each stratigraphic unit in which the SPT sample is collected is provided in Carson et al. (1986). Each SPT number is assigned to entire stratigraphic unit in which the SPT sample was collected. The depths associated with each stratigraphic unit are provided in Carson et al. (1986).

Tokimatsu and Seed (1987) provide the chart in Figure 4 for the case of saturated sands. Figure 4 is used in this study to estimate the corresponding volumetric strain of each soil layer. Settlement is calculated by multiplying the volumetric strain by the layer thickness. The total settlement then is the sum of the settlements for each of the layers. In Figure 4, the volumetric strain is calculated using  $(N_1)_{60}$  and cyclic stress ratio (CSR) defined as:

$$CSR = 0.65 \frac{a_{max}}{g} \frac{\sigma_{vo}}{\sigma'_{vo}} r_d$$

where  $a_{max}$  is the PGA at ground surface generated by the earthquake, g is the acceleration of gravity, and  $\sigma_{vo}$  is the total vertical overburden stresses, respectively, and  $r_d$  is a nonlinear stress reduction coefficient that varies with depth and can be obtained from Brandes (2002).

Figure 4 corresponds to an earthquake of magnitude 7.5. For earthquakes with magnitudes other than 7.5, the volumetric strain can be obtained from the volumetric strain scaling factors provided in Tokimatsu and Seed (1987). For this study, since the deaggregated earthquake for PGA has an Mw of 7.5 the scaling factor of 1.0 is used for volumetric strains.

Matti and Carson (1991) calculated  $\sigma_{vo}$  and  $\sigma'_{vo}$  for each SPT sampling depth by using hypothetical values for ground-water depth. The hypothetical ground-water depth corresponds to the nearest ground-water multiple of 10 ft overlying the SPT depth. We used the same definition of the ground-water table and calculated  $\sigma'_{vo}$  and  $\sigma_{vo}$  for the midpoint of each sand layer in this study.

The liquefaction induced settlement analysis is tabulated in Appendix B of this report. The settlement is calculated for each stratigraphic layer in which the SPT sample is collected. For each borehole, the total



settlement is obtained as the summation of the settlements for each of the layers. Note that the volumetric strain from the Tokimatsu and Seed (1987) curves (Figure 4) reaches a maximum when CSR is greater than 0.5. In this study, CSR is greater than 0.5 for all the boreholes. When  $(N_1)_{60}$  is greater than about 32 and CSR is greater than 0.5, Figure 4 does not estimate any volumetric strain. Therefore, we did not calculate any settlement when  $(N_1)_{60}$  is greater than 32.

The liquefaction-induced settlement analysis shown in Appendix B indicates settlements ranging from 0 inches to 4.3 inches for different boreholes. Regardless of the relative position of the boreholes, a maximum differential settlement of 4.3 inches is considered for the region. Assuming that this settlement occurs across a distance of 100 feet, a maximum angular distortion of about of 0.004 inches per inch would result.

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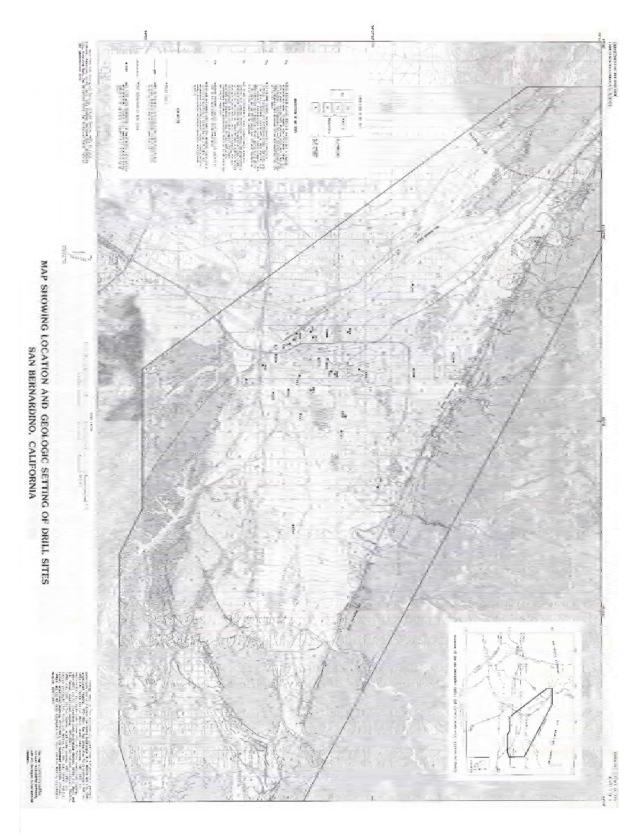
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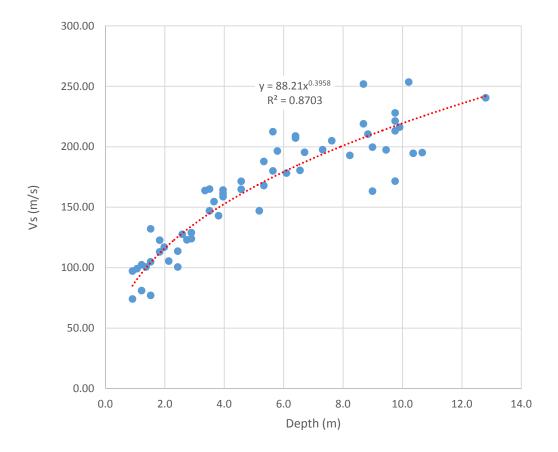
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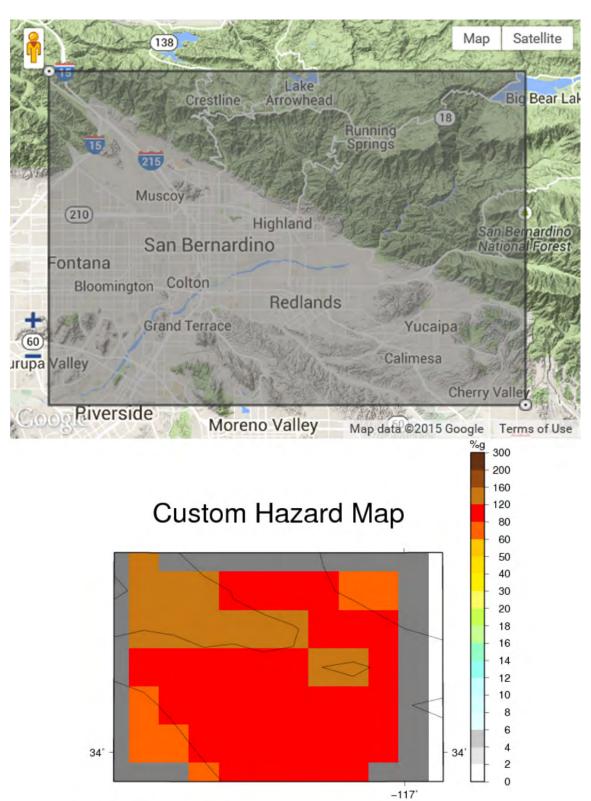


**Figure 1.** Map showing location and geologic setting of drill sites, San Bernardino, California (Adopted from Carson et al. 1986)



**Figure 2.** Vs versus depth calculated form SPT data in Matti and Carson (1991) [blue circles]. A power law linear regression is shown for Vs-depth relationship [red dotted line].

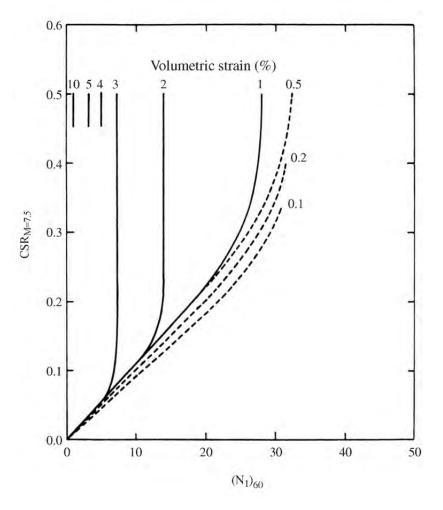




Peak Ground Acceleration



**Figure 3.** Selected region (top) and the contour map for PGA for 2% probability of exceedance in 50 years from USGS *Custom Hazard Maps* tool (bottom).



**FIGURE 4.** Relationship between cyclic stress ratio and volumetric strain for saturated clean sands. (Adopted from Tokimatsu, and Seed, 1987)



### Appendix A

#### **Calculation of Vs**

Borehole	Geologic unit <sup>1</sup>	SPT sampling depth (ft)	SPT sampling depth (m)	Raw SPT N	Corrected SPT N <sub>I</sub>	(N <sub>1</sub> ) <sub>60</sub>	Ground water depth (ft)	σ' <sub>vo</sub> (lb/ft <sup>2</sup> )	σ' <sub>vo</sub> (kPa)	Age Scaling Factors	Vs (m/s)
DRQ02	Qh1	8	2.4	7	7.8	8.8	0	541.0	25.9	0.9	111.72
DRQ02	Qh1	13	4.0	19	22.8	25.8	10	1253.0	60.0	0.9	176.39
DRQ02	Qh1	27	8.2	27	24.3	27.5	20	2573.0	123.2	0.9	214.27
DRQ02	Qh1	32	9.8	36	28.8	32.6	30	3285.0	157.3	0.9	236.84
DSN02	Qh2	4.5	1.4	5	14.6	16.5	0	304.0	14.6	0.9	111.73
DSN03	Qh2	5	1.5	3	4.1	4.6	0	338.0	16.2	0.9	85.67
DSN03	Qh2	9.5	2.9	11	19.1	21.6	0	642.0	30.7	0.9	143.28
DSS01	Qh1	6	1.8	14	25.4	28.8	0	405.0	19.4	0.9	136.34
DSS01	Qh1	17.5	5.3	23	23	26.1	10	1557.0	74.5	0.9	186.61
DSS01	Qh1	28.5	8.7	76	74.4	84.3	20	2674.0	128.0	0.9	279.84
DSS01	Qh1	33.5	10.2	74	59.2	67.1	30	3387.0	162.2	0.9	281.68
DSS02	Qh2	25	7.6	29	33.6	38.1	20	2438.0	116.7	0.9	227.76
DSS02	Qh2	28.5	8.7	40	40.5	45.9	20	2674.0	128.0	0.9	243.31
DSS02	Qh2	32	9.8	33	33.9	38.4	30	3285.0	157.3	0.9	245.89
DSS02	Qh2	42	12.8	45	36	40.8	40	4335.0	207.6	0.9	267.21
DSS03	Qh2	17.5	5.3	34	37.4	42.4	10	1557.0	74.5	0.9	208.69
DSS03	Qh2	34	10.4	30	27	30.6	30	2420.0	115.9	0.9	216.19
DSS04	Qh1	11	3.4	17	29.6	33.5	10	1118.0	53.5	0.9	182.04
DSS04	Qh1	21	6.4	34	41.5	47.0	20	2168.0	103.8	0.9	232.18
DSS04	Qh1	31	9.4	17	21.1	23.9	30	3218.0	154.1	0.9	219.35
DSS05	Qh1	12.5	3.8	12	14.9	16.9	10	1219.0	58.4	0.9	158.85
DSS05	Qh1	21.5	6.6	24	21.6	24.5	20	2201.0	105.4	0.9	200.56
DSS05	Qh1	29.5	9.0	33	26.4	29.9	20	2742.0	131.3	0.9	221.90
DSS05	Qh1	32	9.8	42	38.6	43.7	30	3285.0	157.3	0.9	253.34
DSS06	Qh2	4	1.2	4	6.5	7.4	0	270.0	12.9	0.9	90.05
DSS07	Qh2	9	2.7	12	16.5	18.7	0	608.0	29.1	0.9	136.67
DSS07	Qh2	29.5	9.0	11	11	12.5	20	2742.0	131.3	0.9	181.43
DSS08	Qh2	18.5	5.6	27	29.7	33.7	10	1624.0	77.8	0.9	200.01
DSS09	Qh2	3.5	1.1	12	18	20.4	0	237.0	11.3	0.9	110.17
DSS09	Qh2	8.5	2.6	13	20.6	23.3	0	574.0	27.5	0.9	141.77
DSS-10	Qh1	5	1.5	26	42.6	48.3	0	338.0	16.2	0.9	146.78
DSS-10	Qh1	15	4.6	15	24	27.2	10	1388.0	66.5	0.9	183.11
DSS-10	Qh1	35	10.7	23	18.4	20.9	30	3488.0	167.0	0.9	216.88
DSS-11	Qh2	6	1.8	10	17.8	20.2	0	405.0	19.4	0.9	125.64
DSS-11	Qh2	15	4.6	20	28.5	32.3	10	1388.0	66.5	0.9	190.49
DSS-11	Qh2	32	9.8	14	11.2	12.7	30	3285.0	157.3	0.9	190.60
DSS-12	Qh2	3	0.9	4	6	6.8	0	203.0	9.7	0.9	82.32
DSS-12	Qh2	13	4.0	22	26.4	29.9	10	1253.0	60.0	0.9	182.44



Borehole	Geologic unit <sup>1</sup>	SPT sampling depth (ft)	SPT sampling depth (m)	Raw SPT N	Corrected SPT N <sub>I</sub>	(N <sub>1</sub> ) <sub>60</sub>	Ground water depth (ft)	σ' <sub>vo</sub> (lb/ft <sup>2</sup> )	σ' <sub>vo</sub> (kPa)	Age Scaling Factors	Vs (m/s)
DSS-12	Qh2	19	5.8	39	42.5	48.2	10	1658.0	79.4	0.9	218.32
DSS-12	Qh2	22	6.7	28	30	34.0	20	2235.0	107.0	0.9	217.13
DSS-13	Qh2	7	2.1	3	11.1	12.6	0	473.0	22.6	0.9	117.17
DSS-13	Qh2	12	3.7	18	21.6	24.5	10	1185.0	56.7	0.9	171.80
DSS-14	Qh2	5	1.5	6	15.6	17.7	0	338.0	16.2	0.9	116.50
DSS-14	Qh2	21	6.4	34	40	45.3	20	2168.0	103.8	0.9	230.22
DSS-15	Qh1	3	0.9	8	19.5	22.1	0	203.0	9.7	0.9	107.96
DSS-15	Qh1	9.5	2.9	11	16.1	18.2	0	642.0	30.7	0.9	137.76
DSS-15	Qh1	13	4.0	13	24.4	27.7	10	1253.0	60.0	0.9	179.16
DSS-15	Qh1	24	7.3	22	29.5	33.4	20	2370.0	113.5	0.9	219.49
DSS-15	Qh1	29	8.8	29	33.6	38.1	20	2708.0	129.7	0.9	233.82
DSS-15	Qh1	32.5	9.9	27	30.3	34.3	30	3319.0	158.9	0.9	240.24
DSS-16	Qp	18.5	5.6	12	19.5	22.1	10	1624.0	77.8	1.17	181.56
DSS-17	Qh2	6.5	2.0	9	18.9	21.4	0	439.0	21.0	0.9	129.98
DSS-17	Qh2	11.5	3.5	17	29.6	33.5	10	1151.0	55.1	0.9	183.37
DSS-20	Qh1	17	5.2	12	13.2	15.0	10	1523.0	72.9	0.9	163.33
DSS-20	Qh1	20	6.1	14	21.5	24.4	20	2100.0	100.5	0.9	198.01
DSS-21	Qh2	4	1.2	9	18	20.4	0	270.0	12.9	0.9	113.82
DSS-21	Qh2	8	2.4	6	13.3	15.1	0	541.0	25.9	0.9	126.31
DSS-21	Qh2	11.5	3.5	8	17.9	20.3	10	1151.0	55.1	0.9	163.34

<sup>1</sup>Qh1: Younger Holocene deposits include sedimentary materials that Matti and Carson (1991) interpreted to have accumulated within the last 500 to 1,000 yr.

Qh2: Older Holocene deposits include sedimentary materials that Matti and Carson (1991) interpreted to have accumulated between 500 or 1,000 yr ago and 10,000 to perhaps 15,000 yr ago.

Qp: Pleistocene deposits include sedimentary materials that Matti and Carson (1991) interpreted to have accumulated between 10,000 or 15,000 yr ago and about 750,000 yr ago.



### Appendix B

#### Liquefaction Induced Settlements

Borehole	Geologic unit <sup>1</sup>	(N <sub>1</sub> ) <sub>60</sub>	Ground water depth (ft)	σ' <sub>v0</sub> (mid height) (lb/ft <sup>2</sup> )	σ <sub>vo</sub> (mid height) (lb/ft <sup>2</sup> )	Depth to the top of layer (ft)	Depth to the bottom of layer (ft)	Layer thickness (ft)	Depth to midpoint (ft)	Depth to midpoint (m)	r <sub>d</sub>	CSR	Volumetric strain	Settlement of the layer (inch)	Total settlement (inch)
DRQ02	Qh1	8.8	0	557.5	1072.5	6.50	10.00	3.50	8.25	2.51	0.98	1.472	2.8%	1.16	
DRQ02	Qh1	25.8	10	1556.8	2025.0	10.00	25.00	15.00	17.50	5.33	0.96	0.973	1.2%	2.08	3.87
DRQ02	Qh1	27.5	20	2606.8	3075.0	25.00	30.00	5.00	27.50	8.38	0.94	0.861	1.0%	0.62	3.07
DRQ02	Qh1	32.6	30	3487.9	3800.0	30.00	40.00	10.00	35.00	10.67	0.89	0.756	-	-	
DSN02	Qh2	16.5	0	388.5	747.5	3.00	8.50	5.50	5.75	1.75	0.99	1.481	1.8%	1.20	1.20
DSN03	Qh2	4.6	0	270.3	520.0	0.00	8.00	8.00	4.00	1.22	0.99	1.487	4.2%	4.02	- 4.37
DSN03	Qh2	21.6	0	608.1	1170.0	8.00	10.00	2.00	9.00	2.74	0.98	1.469	1.5%	0.35	
DSS01	Qh1	28.8	0	337.9	650.0	0.00	10.00	10.00	5.00	1.52	0.99	1.483	0.9%	1.10	2.33
DSS01	Qh1	26	10	1354.1	1635.0	10.00	19.00	9.00	14.50	4.42	0.97	0.910	1.1%	1.23	
DSS01	Qh1	84.3	20	2488.5	2847.5	21.50	30.00	8.50	25.75	7.85	0.94	0.839	-	-	
DSS01	Qh1	67.1	30	3386.5	3605.0	30.00	37.00	7.00	33.50	10.21	0.90	0.748	-	-	
DSS02	Qh2	38.1	20	2336.5	2555.0	22.00	25.00	3.00	23.50	7.16	0.95	0.806	-	-	
DSS02	Qh2	45.9	20	2674.3	3205.0	27.00	30.00	3.00	28.50	8.69	0.93	0.873	-	-	0.00
DSS02	Qh2	38.4	30	3217.6	3280.0	30.00	32.00	2.00	31.00	9.45	0.92	0.733	-	-	0.00
DSS02	Qh2	41	40	4301.4	4395.0	40.00	43.00	3.00	41.50	12.65	0.84	0.666	-	-	
DSS03	Qh2	42.4	10	1556.8	2025.0	16.50	18.50	2.00	17.50	5.33	0.96	0.973	-	-	0.38
DSS03	Qh2	31	30	3302.0	3442.5	30.00	34.50	4.50	32.25	9.83	0.91	0.741	0.7%	0.38	0.38
DSS04	Qh1	33.5	10	1235.8	1407.5	10.00	15.50	5.50	12.75	3.89	0.97	0.862	-	-	
DSS04	Qh1	47.0	20	2336.5	2555.0	20.00	27.00	7.00	23.50	7.16	0.95	0.806	-	-	1.16
DSS04	Qh1	23.9	30	3403.4	3637.5	30.00	37.50	7.50	33.75	10.29	0.90	0.750	1.3%	1.16	
DSS05	Qh1	16.9	10	1185.1	1310.0	10.00	14.00	4.00	12.00	3.66	0.97	0.838	1.8%	0.86	
DSS05	Qh1	24.5	20	2319.6	2522.5	20.00	26.50	6.50	23.25	7.09	0.95	0.802	1.3%	0.98	2.02
DSS05	Qh1	29.9	20	2708.1	3270.0	28.00	30.00	2.00	29.00	8.84	0.93	0.878	0.8%	0.19	2.03
DSS05	Qh1	43.7	30	3285.1	3410.0	30.00	34.00	4.00	32.00	9.75	0.91	0.740	-	-	



Borehole	Geologic unit <sup>1</sup>	(N <sub>1</sub> ) <sub>60</sub>	Ground water depth (ft)	σ' <sub>vo</sub> (mid height) (lb/ft <sup>2</sup> )	σ <sub>vo</sub> (mid height) (lb/ft <sup>2</sup> )	Depth to the top of layer (ft)	Depth to the bottom of layer (ft)	Layer thickness (ft)	Depth to midpoint (ft)	Depth to midpoint (m)	r <sub>d</sub>	CSR	Volumetric strain	Settlement of the layer (inch)	Total settlement (inch)
DSS06	Qh2	7.4	0	337.9	650.0	3.00	7.00	4.00	5.00	1.52	0.99	1.483	3.0%	1.44	1.44
DSS07	Qh2	18.7	0	692.6	1332.5	6.00	14.50	8.50	10.25	3.12	0.98	1.465	1.7%	1.70	2.10
DSS07	Qh2	12	20	2725.0	3302.5	28.50	30.00	1.50	29.25	8.92	0.93	0.881	2.2%	0.40	2.10
DSS08	Qh2	33.7	10	1641.2	2187.5	15.00	22.50	7.50	18.75	5.72	0.96	0.994	-	-	0.00
DSS09	Qh2	20	0	168.9	325.0	0.00	5.00	5.00	2.50	0.76	0.99	1.492	1.5%	0.93	1.49
DSS09	Qh2	23.3	0	557.5	1072.5	6.50	10.00	3.50	8.25	2.51	0.98	1.472	1.3%	0.56	1.49
DSS-10	Qh1	48.3	0	337.9	650.0	0.00	10.00	10.00	5.00	1.52	0.99	1.483	-	-	
DSS-10	Qh1	27	10	1371.0	1667.5	11.50	18.00	6.50	14.75	4.50	0.97	0.916	1.1%	0.83	1.92
DSS-10	Qh1	20.9	30	3352.7	3540.0	30.00	36.00	6.00	33.00	10.06	0.91	0.746	1.5%	1.09	
DSS-11	Qh2	20.2	0	388.5	747.5	0.00	11.50	11.50	5.75	1.75	0.99	1.481	1.6%	2.15	
DSS-11	Qh2	32.3	10	1371.0	1667.5	11.50	18.00	6.50	14.75	4.50	0.97	0.916	-	-	3.59
DSS-11	Qh2	12.7	30	3335.8	3507.5	30.00	35.50	5.50	32.75	9.98	0.91	0.744	2.2%	1.44	
DSS-12	Qh2	7	0	202.7	390.0	0.00	6.00	6.00	3.00	0.91	0.99	1.490	3.2%	2.32	
DSS-12	Qh2	29.9	10	1320.3	1570.0	10.00	18.00	8.00	14.00	4.27	0.97	0.897	0.8%	0.75	3.07
DSS-12	Qh2	48.2	10	1658.1	2220.0	18.00	20.00	2.00	19.00	5.79	0.96	0.998	-	-	5.07
DSS-12	Qh2	34	20	2235.1	2360.0	20.00	24.00	4.00	22.00	6.71	0.95	0.781	-	-	
DSS-13	Qh2	12.6	0	506.8	975.0	5.00	10.00	5.00	7.50	2.29	0.98	1.474	2.2%	1.32	2.68
DSS-13	Qh2	24.5	10	1354.1	1635.0	10.00	19.00	9.00	14.50	4.42	0.97	0.910	1.3%	1.35	2.08
DSS-14	Qh2	17.7	0	337.9	650.0	0.00	10.00	10.00	5.00	1.52	0.99	1.483	1.7%	2.08	2.08
DSS-14	Qh2	45	20	2184.5	2262.5	18.50	24.00	5.50	21.25	6.48	0.95	0.768	-	-	2.08
DSS-15	Qh1	22.1	0	287.2	552.5	0.00	8.50	8.50	4.25	1.30	0.99	1.486	1.4%	1.45	
DSS-15	Qh1	18.2	0	709.5	1365.0	8.50	12.50	4.00	10.50	3.20	0.98	1.464	1.7%	0.81	
DSS-15	Qh1	27.7	10	1539.9	1992.5	16.00	18.50	2.50	17.25	5.26	0.96	0.969	1.0%	0.31	2.57
DSS-15	Qh1	33.4	20	2252.0	2392.5	18.50	26.00	7.50	22.25	6.78	0.95	0.786	-	-	2.57
DSS-15	Qh1	38.1	20	2674.3	3205.0	26.00	31.00	5.00	28.50	8.69	0.93	0.873	-	-	
DSS-15	Qh1	34.3	30	3369.6	3572.5	31.00	35.50	4.50	33.25	10.13	0.90	0.747	-	-	
DSS-16	Qp	22.1	10	1624.3	2155.0	17.00	20.00	3.00	18.50	5.64	0.96	0.990	1.4%	0.51	0.51
DSS-17	Qh2	21.4	0	439.2	845.0	4.00	9.00	5.00	6.50	1.98	0.98	1.478	1.5%	0.88	0.88



Borehole	Geologic unit <sup>1</sup>	(N <sub>1</sub> ) <sub>60</sub>	Ground water depth (ft)	σ' <sub>vo</sub> (mid height) (lb/ft <sup>2</sup> )	σ <sub>vo</sub> (mid height) (lb/ft <sup>2</sup> )	Depth to the top of layer (ft)	Depth to the bottom of layer (ft)	Layer thickness (ft)	Depth to midpoint (ft)	Depth to midpoint (m)	r <sub>d</sub>	CSR	Volumetric strain	Settlement of the layer (inch)	Total settlement (inch)
DSS-17	Qh2	33.5	10	1134.5	1212.5	10.00	12.50	2.50	11.25	3.43	0.97	0.812	-	-	
DSS-20	Qh1	15.0	10	1556.8	2025.0	16.50	18.50	2.00	17.50	5.33	0.96	0.973	1.9%	0.46	0.77
DSS-20	Qh1	24.4	20	2471.6	2815.0	24.50	26.50	2.00	25.50	7.77	0.94	0.836	1.3%	0.30	0.77
DSS-21	Qh2	20.4	0	236.5	455.0	2.00	5.00	3.00	3.50	1.07	0.99	1.488	1.5%	0.56	
DSS-21	Qh2	15.1	0	473.0	910.0	5.00	9.00	4.00	7.00	2.13	0.98	1.476	1.9%	0.92	1.94
DSS-21	Qh2	20.3	10	1134.5	1212.5	10.00	12.50	2.50	11.25	3.43	0.97	0.812	1.6%	0.47	

<sup>1</sup>Qh1: Younger Holocene deposits include sedimentary materials that Matti and Carson (1991) interpreted to have accumulated within the last 500 to 1,000 yr. Qh2: Older Holocene deposits include sedimentary materials that Matti and Carson (1991) interpreted to have accumulated between 500 or 1,000 yr ago and 10,000 to perhaps 15,000 yr ago.

Qp: Pleistocene deposits include sedimentary materials that Matti and Carson (1991) interpreted to have accumulated between 10,000 or 15,000 yr ago and about 750,000 yr ago.



# **APPENDIX C**

**Public Records Search** 



### **APPENDIX C1**

**Municipal Public Records Requests** 



### **APPENDIX C1.1**

## **Adelanto Industrial Park III**



American Engineering Laboratories, Inc.

13641 John Glenn Rd., Suite C Apple Valley, CA 92307 Telephone (619) 247-8445 FAX (619) 247-8029



San Diego • Modesto • Corona • Yucca Valley • Apple Valley

December 25, 1989

A.E.L. Project # 45024 Report No. 1

City of Adelanto P. O. Box 10 Adelanto, CA 92301

Attn: Mr. Roland "Dee" Dorval, City Engineer

Subject: REPORT OF GEOTECHNICAL INVESTIGATION ADELANTO INDUSTRIAL PARK III INDUSTRIAL WAY AT KOALA ROAD ADELANTO, CALIFORNIA

Gentlemen:

In accordance with your request, we have completed a geotechnical investigation for the proposed project. We are presenting herein our findings and recommendations.

The findings of this study indicate that the site is suitable for the proposed development provided the recommendations presented in the attached report are complied with.

If you questions after reviewing the findings have any and recommendations contained in the attached report, please do not contact this office. This opportunity to hesitate to be of professional service is sincerely appreciated.

Respectfully submitted, AMERICAN ENGINEERING LABORATORIES, INC.

Staff Civil Endin NO. C 034288 RCE No. C034288

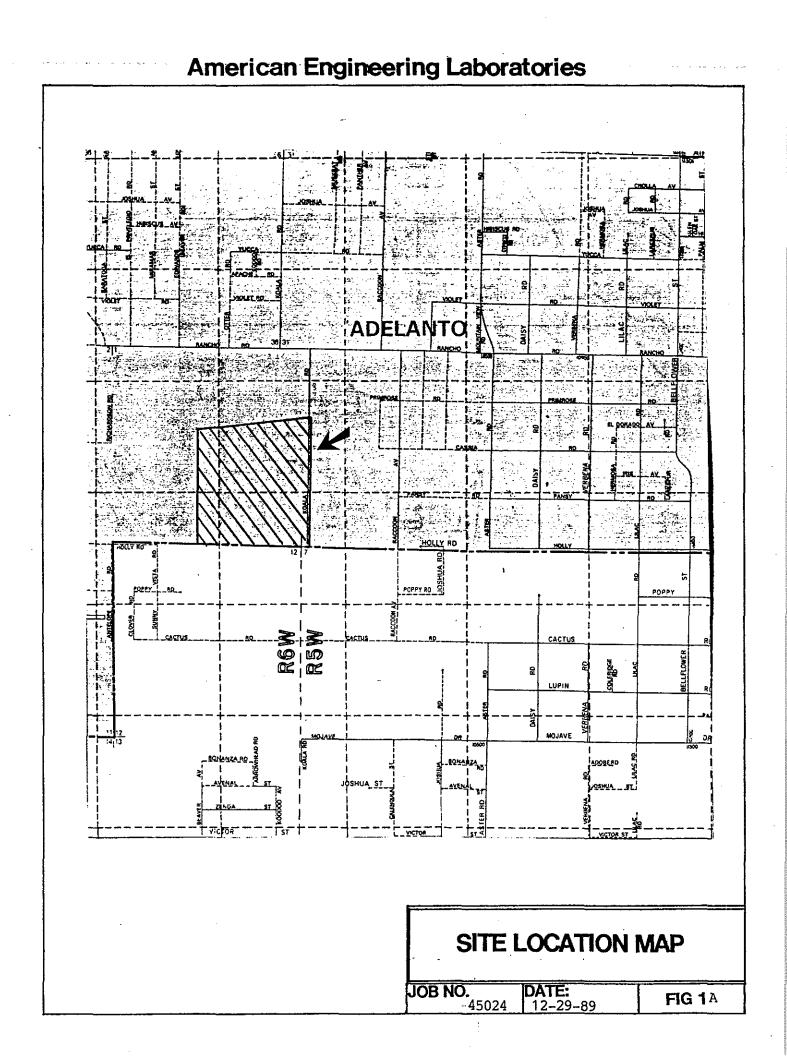
Dan D. Goodwin Project Manager

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	14			
		(15)		
5	13			
(2) (3) (12)	(16	)		•
	U)	(17.)		_
8	10	18		
9	- -			
APPROXIMATE TEST PIT LOCA	TIONS			
		PLOT PL		
	JOB NO. 45	024 DATE: 12-29	-89 PLATE	<u> </u>

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#### AMERICAN ENGINEERING LABORATORIES

		AMENI		· • · ·	OF TERMS	· · · · · · · · · · · · · · · · · · ·		<u> </u>				
P	RIMARY DIVI	SIONS	SY	MBOLS	SEC	CONDARY DI	/ISIONS					
	GRAVELS	CLEAU	i• .	GW	Well graded grave fines.	sia, gravel-sand	mixtures, little o	or no				
SOILS MATERIAL	MORE THAN HALF OF	LESS T	HAN 💽	GP	Poorly graded gra	vela or gravel-	sand mixtures, lit	tle or				
	COARSE FRACTION IS	GRAVE	•	GM	Silty gravels, grav fines.	rei-sand-silt mix	tures, non-plast	c				
$\Gamma U = h$	ILARGER THAN	WITH FI	NES	GC	Clayey gravels, gr fines.	avel-sand-clay	mixtures, plasti	•				
ARSE GRAINE E THAN HALF C LARGER THAN SIEVE SU	SANDS MORE THAN	CLEA SAND	Well graded sand	s, gravelly sand	s, little or no fine	8.						
RSE THAN ARG	HALF OF COARSE	(LESS T 5% Fin		• SP	Poorly graded san	ids or gravelly :	sands, little or no	fines.				
COAI MORE IS L	FRACTION IS SMALLER THAN	SAND WITH FI		III SM								
	NO. 4 SIEVE			<u> </u>	Clayey sands, sai		· · · · · · · · · · · · · · · · · · ·					
	O       W         I       W         I       W         I       W         I       W         I       W         I       W         I       W         I       W         I       Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.         I       I											
ED S HALF SMAI		LIMIT IS Han 50%	H		clays, sandy cla	ys, lean clays.	- •					
ANA	A A Inorganic silts, micaceous or diatomaceous fine san											
l O w m m		ID CLAY	's	-,,,,	or silty solis, ela inorganic clays c		, fat clays.	·				
FINE MOR	GREATER	THAN 50	*	/// он				anic				
HIG	HLY ORGAN			Pt	Peat and other h	ighly organic so	)ils.					
		• •		GRAIN	N SIZES							
			SAND		GRAY	VEL						
SILTS A	ND CLAYS		MEDIUM		h	COARSE		DERS				
	200	40 U.S. STANI	DARD SE	10 RIES SIE	4 3/4 VE CLE	-	EVE OPENINGS					
	RELATIVE	· · · · · ·	• •	<b>-</b>	-	NSISTENC	Y 	:				
S	ANDS, GRAVELS Non-plastic Si	AND BLOW	S/FOOT*		CLAYS AND PLASTIC SILTS		BLOWS/FOOT*					
	VERY LOOSE LOOSE		- 4 - 10		VERY SOFT	0 - 1/4 1/4 - 1/2 1/2 - 1	0 - 2 2 - 4					
	MEDIUM DENSE 10 - 30 STIFF 1 - 2 8 - 16											
DENSE30 - 50VERY STIFF2 - 416 - 32VERY DENSEOVER 50HARDOVER 4OVER 32												
*NUMBER OF BLOWS OF 140 POUND HAMMER FALLING 30-INCHES TO DRIVE A 2-INCH O.D.												
(1-3/8-INCH I.D.) SPLIT SPOON (ASTM D-1586). **UNCONFINED COMPRESSIVE STRENGTH IN TONS/SQ. FT. AS DETERMINED BY LABORATORY												
TESTING OR APPROXIMATED BY THE STANDARD PENETRATION TEST (ASTM D-1586), POCKET PENETROMETER, TORVANE, OR VISUAL OBSERVATION												
KEY TO LOGS - UNIFIED SOIL CLASSIFICATION SYSTEM (ASTM D-2487)												
JOB NO	JOB NO.: 45024 DATE: 12-29-89 FIGURE: 2A											

DAT	DATE OBSERVED: NOV. 14, 1989 METHOD OF DRILLING: CASE 580 C EXTENDAHOE EXISTING LOGGED BY: P.H. GROUND ELEVATION: GRADE LOCATION: SEE LOCATION PLAN											
DEPTH (FEET)	-	_				IN PLACE DRY DENSITY (PCF)	TEST PIT NO1	Soil test				
-0		m N/A		± x		=0	VERY LOOSE LT. BROWN SILTY MEDIUM TO COARSE SAND (RECENT ALLUVIUM)	SEE TEST RESULTS				
	SM		x	х			GRAYISH BROWN FINE SILTY SAND, MODERATELY CEMENTED/TRACE OF CALICHE					
5	SM SC		x	x			LT. BROWN SILTY FINE TO MEDIUM SAND MODERATE TO STRONGLY CEMENTED. CALCAREOUS WEBBING THROUGHOUT (CALICHE)	· · · · · · · · · · · · · · · · · · ·				
	SM SC		x	x			GRAY SILTY MEDIUM TO LOOSE SILTY SAND W/COBBLES TO 2" TO 5"/ WITH MORE CALICHE					
15-							BOTTOM OF EXCAVATION NO WATER ENCOUNTERED					
-							NOTE: TRENCH LOCATED IN MINOR "WASH"					

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LOGGED BY: P.H. GROUND EL	EVATION: E.G. LOCATION: SEE LOCATION	PLAN
	TEST PIT NO. 2	
SM X	DARK BROWN SILTY FINE SAND W/ TRACE OF ORGANIC/ROOTS (TOPSOIL)	SEE TEST RESULTS
SM X X	LT. BROWN SILTY FINE-MEDIUM SAND, VERY STRONGLY CEMENTED	
5-SM SC X X	REDDISH BROWN MEDIUM TO COARSE SLIGHTLY SILTY SAND-MILD TO STRONGLY CEMENTED	
-SM 10-ML - X X	GRAYISH BROWN SILTY VERY FINE TO FINE SAND, STRONGLY CEMENTED WITH INTERBEDDED LAYERS IF WHITISH CALCAREOUS MATERIAL	· · · · · · · · · · · · · · · · · · ·
	BOTTOM OF EXCAVATION NO WATER ENCOUNTERED	· · ·
JOB NO.: 45024	LOG OF TEST PIT	FIGURE: 1

DAT	DATE OBSERVED: NOV. 14, 1989 METHOD OF DRILLING: CASE 580 C EXTENDAHOE								
LOG	GED	BY:	<u>P.</u>	Н	GROU	ND EL	EVATION: E.G. LOCATION: SEE LOCATION	PLAN	
DEPTH (FEET)	CLASSIFICATION	BLOWS/FOOT	UNDISTURBED SAMPLE	BULK SAMPLE	MOISTURE CONTENT (%)	IN PLACE DRY DENSITY (PCF)	TEST PIT NO DESCRIPTION	SOIL TEST	
-	G₽			x			VERY LOOSE MEDIUM CLEAN COARSE GRAVELY SAND	SEE TEST RESULTS	
	SM		x	x			LT. BROWN WELL GRADED SILTY SAND, MEDIUM CEMENTED W/TRACE OF CALCAREOUS WEBBING		
6	SM		x	х			BROWN SILTY FINE TO MEDIUM SAND/MEDIUM DENSE		
							BOTTOM OF EXCAVATION NO WATER ENCOUNTERED NOTE: TRENCH LOCATED IN MEDIUM WASH		

LOC	GED	BY: P.1	н	GROUNE	D EL	EVATION: E.G. LOCATION: SEE LOCATION	PLAN		
						TEST PIT NO4	-		
	SM SM	x	X X			SILT (TOPSOIL) W/TRACE OF ROOTS	SEE TES	ST RESULTS	
- 5						LT. REDDISH BROWN FINE TO COARSER WITH DEPTH SILTY SAND. MEDIUM CEMENTATION/NO EVIDENT WEBBING			
-	SM SC	X	x			WHITISH GRAY SILTY FINE TO MEDIUM SAND-STRONG CEMENTATION WITH WEBBING AND CALCAREOUS CHUNKS			
10-	SM	x	x			GRAY SILTY VERY FINE SAND, MEDIUM DENSE			
-						BOTTOM OF EXCAVATION NO WATER ENCOUNTERED			
- 15-									
101	NO.							GURE: 2	
		<u>4502</u>	4			LOG OF TEST PIT			أسعيها

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AMERICAN ENGINEERING LABORATORIES

[	DAT	EOB	SER	VED:	N	OV. 1	5, 19	89 METHOD OF DRILLING: CASE 580 C EX	TENDAHOE
	LOG	GED	BY:	P.1	ł	GROU	ND EL	EVATION: E.G. LOCATION: SEE LOCATION	PLAN
•	DEPTH (FEET)	<b>CLASSIFICATION</b>	BLOW8/FOOT	UNDISTURBED SAMPLE	BULK SAMPLE	MOISTURE CONTENT (%)	IN PLACE DRY DENSITY (PCF)	TEST PIT NO5	SOIL TEST
		SM	-		X			LT. GRAYISH BROWN SILTY FINE TO MEDIUM SAND W/GRAVEL (LOOSE TO MED. DENSE)	SEE TEST RESULTS
		SM SC		x	х			LT. BROWN SILTY FINE TO MEDIUM SAND/ CALCAREOUS/STRONGLY CEMENTED	
	5-	sc		x	х			WHITISH GRAY VERY SILTY SAND-HIGHLY CEMENTED-VERY DIFFICULT DIGGING	· · · · · ·
		SM SC		x	x			LT. BROWN SILTY MED. SAND W/CALCAREOUS WEBBING.	
	10  	SM		x	x			GRAY VERY FINE SILTY SAND, MEDIUM DENSE WITH INTERBEDDED LAYERS OF CEMENTED SAND	
	15- - -							BOITOM OF EXCAVATION NO WATER ENCOUNTERED	

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		· · · •				TEST PIT NO6		
	SM		x			VERY THIN TOPSOIL, THEN LT. GRAYISH WHITE SILTY MEDUIM TO COARSE SAND VERY DENSE	SEE TEST	RESULTS
-	SM SC SM				   	GRAYISH BROWN SILTY FINE TO MEDIUM COARSE SAND-EXTREMELY DENSE, CEMENTED		<del></del> , · ·
-	SC GM SM					LT. BROWN SILTY FINE TO MEDIUM SAND WITH CALCAREOUS WEBBING THROUGHOUT. VERY DENSE		<i>.</i>
						MOTTLED BROWNISH ORANGE GRAVELLY MED. TO COARSE SAND, MEDIUM TO STRONGLY CEMENTED. COBBLES TO 6" AT $9-9\frac{1}{2}$ FT.		
- - - -						BOTTOM OF EXCAVATION NO WATER ENCOUNTERED		
В	NO.:	45024		<u> </u>		LOG OF TEST PIT	FIGU	RE: 3

DAT	EOE	ISER	VED:	NO	<b>v.</b> 15	, 198	9 METHOD OF DRILLING: CASE 580 C EX	TENDAHOE
LOG	GED	BY:	P.H		GROU		EVATION: E.G. LOCATION: SEE LOCATION	PLAN
DEPTH (FEET)	<b>CLASSIFICATION</b>	BLOWS/FOOT	UNDISTURBED SAMPLE	BULK SAMPLE	MOISTURE CONTENT (%)	IN PLACE DRY DENSITY (PCF)	TEST PIT NO7	SOIL TEST
-	SM	N/A		х			VERY DRY FIRM LIGHT BROWN FINE TO MEDIUM SILTY SAND W/THIN TOPSOIL	SEE TEST RESULTS
5-	SM SC SM SC	-	x	x		-	BROWN SILTY SAND W/TRACE OF CLAY- MEDIUM DENSE TO DENSE LT. BROWN SILTY FINE/MEDIUM SAND WITH EXRENSIVE CALCAREOUS WEBBING AND STRONGLY CEMENTED	·
	SM SC		x	x			WHITISH GRAY SLIGHTLY SILTY HIGHLY CEMENTED FINE TO COARSE SAND DIFFICULT TO DIG BOTTOM OF EXCAVATION NO WATER ENCOUNTERED	

LOG	OGGED BY: GROUND ELEVATION: E.G. LOCATION: SEE LOCATION PLAN									
	_						TEST PIT NO8	·		
-0	SM	N/A		x			LT. BROWN LOOSE TO MEDIUM DENSE GRAVELLY SILTY SAND W/THIN TOPSOIL	SEE TEST RESULTS		
-	SM SC		x	x			LT. BROWN HIGHLY CEMENTED FINE TO MEDIUM SILTY SAND			
5	SC ML		х	x			AS ABOVE BUT "EXTREMELY DENSE MARGINAL REJECTION OF HOE			
-  10 - - -							BOTTOM OF EXCAVATION NO WATER ENCOUNTERED			
- 15- - -										
JOE	JOB NO.: 45024						LOG OF TEST PIT	FIGURE: 4		

AMERICAN ENGINEERING LABORATORIES

DAT	DATE OBSERVED: NOV. 15, 1989 METHOD OF DRILLING: CASE 580 C. EXTENDAHOE									
LOG	IGED	BY:	P	.н.	GROU	ND EL	EVATION: E.G. LOCATION: SEE LOCATION	PLAN		
DEPTH (FEET)	CLASSIFICATION	BLOWS/FOOT	UNDISTURBED SAMPLE	BULK SAMPLE	CONTENT (%)	IN. PLACE DRY DENSITY (PCF)	TEST PIT NO. 9	SOIL TEST		
ļ .	SM						BROWN SILTY MEDIUM SAND W/TRACE GRAVEL-LOOSE TO MEDIUM DENSE, THIN	SEE TEST RESULTS		
	SM SC	ĺ	x	x			TOPSOIL			
5-	SM		x	x			GRAYISH BROWN HIGHLY CALCAREOUS SILTY SAND, FIRMNON CEMENTED			
-0							BROWN GRAVELLY SILTY FINE TO COARSE SAND/HIGHLY CEMENTED			
-	SM GM		x	х			AS ABOVE WITH CALCAREOUS WEBBING			
-	SM SC		x							
10	SM SC						LT. GRAYISH BROWN GRAVELLY SLITY WELL GRADED SAND-MODERATELY CEMENTED			
							BROWN SILTY WELL GRADED SAND, STRONGLY CEMENTED-VERY DIFFICULT DIGGING			
15							BOTTOM OF EXCAVATION NO WATER ENCOUNTERED			

LOG	GED	BY:	P.	н.	GROU	ND EL	EVATION: E.G. LOCATION: SEE LOCATI	ON PLAN
•							TEST PIT NO. 10	
	ŚМ			x			BROWN SILTY MEDIUM SAND W/TRACE GRAVEL-LOOSE TO MEDIUM DENSE, THIN TOPSOIL	SEE TEST RESULTS
-	SM SC		х	x			BROWN SILTY MEDIUM TO COARSE CEMENTED SAND	
5-	SM SC		x				SAME AS ABOVE WITH CALCAREOUS WEBBING THROUGHOUT	
-	SM GM		x	x	·		WHITISH GRAY GRAVELLY SILTY SAND, STRONGLY CEMENTED WITH WEBBING AND COBBLES TO 4"	
- 10- -	SC ML		x				MEDIUM GRAY VERY FINE SANDY SILT MEDIUM DENSE, SLIGHTLY PLASTIC, NO CEMENTATION	
-								<u>     .                               </u>
15— - -							BOTTOM OF EXCAVATION NO WATER ENCOUNTERED	
JOB	NO.	<b>:</b> 45(	024	. <u></u>	<u>`</u>	L	LOG OF TEST PIT	FIGURE: 5

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AMERICAN ENGINEERING LABORATORIES

DA	DATE OBSERVED: NOV. 16, 1989 METHOD OF DRILLING: CASE 580 C EXTENDAHOE									
LO	GGEI	BY:	P.	H	GROU		EVATION: E.G. LOCATION: SEE LOCATION	PLAN		
DEPTH (FEET)	CLASSIFICATION	BLOW8/FOOT	UNDISTURBED SAMPLE	BULK SAMPLE	MOISTURE CONTENT (%)	IN. PLACE DRY DENSITY (PCF)	TEST PIT NO. 11	SOIL TEST		
	- SM	N/A		х			LT. BROWN SILTY MEDIUM/COARSE SAND, LOOSE (RECONT ALLUVIUM)	SEE TEST RESULTS		
	- SM		х	х			GRAY SILTY WELL GRADED HIGHLY CEMENTED SAND, TRACE OF WEBBING			
5	- S™		x	x			BROWN GRAVELLY SILTY SAND /SANDY SILT , MODERATELY DENSE	· · · · · · · · · · · · · · · · · · ·		
	SM	·	x				LT. BROWN SILTY FINE TO MEDIUM SAND W/CALCAREOUS WEBBING STRONGLY CEMENTED			
10			X X		-		LT. GRAY FINE SANDY SILT-STRONGLY CEMENTED			
			х				ORANGISH BROWN MED. TO COARSE SLIGHTY CEMENTED SAND W/TRACE GRAVEL			
15							MOTTLED BROWN/WHITE/ORANGE CLAYEY SILT/SANDY SILT-SOFT TO FIRM			
							BOTTOM OF EXCAVATION NO WATER ENCOUNTERED NOTE: TRENCH IN DRAINAGE "WASH"AREA			

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							TEST PIT NO. 12			
SM N/A X			LT. GRAY FINE SILTY SAND W/TRACE OF GRAVEL & THIN TOPSOIL	SEE TEST RESULT						
	SM		Х				LT. GRAY FINE TO MEDIUM SAND SILT/ SILTY SAND.NUMEROUS VOIDS(ROOTS) & HIG	LY CEMENTED		
-	SM SP		х				GRAY SLIGHTLY CEMENTED MEDIUM-COARSE SAND, W/COBBLES 5"-7" AT BOTTOM			
       -	SM		X	 			LT. GRAY VERY FINE SANDY SILT/			
							SILTY SAND-SLIGHTLY CEMENTED			
							BOITOM OF EXCAVATION NO WATER ENCOUNTERED			
<u> </u> Эв	NO.	45	024		[	[	LOG OF TEST PIT	FIGURE: 6		

DAT	E.OE	SER	VED:	NC	<u>v. 1</u> 6	5 <u>, 19</u> 8	39 METHOD OF DRILLING: CASE 580 C EX	TENDAHOE
LOG	IGED	BY:	<u> </u>	I	GROU	ND EL	EVATION: E.G. LOCATION: SEE LOCATION	PILAN
DEPTH (FEET)	<b>CLASSIFICATION</b>	BLOWS/FOOT	JNDISTURBED SAMPLE	JULK SAMPLE	MOISTURE CONTENT (%)	IN PLACE DRY DENSITY (PCF)	TEST PIT NO. <u>13</u> DESCRIPTION	SOIL TEST
-0   -	SM		_	x			DARK REDDISH BROWN FINE TO MEDIUM SANDY SILT W/TRACE OF GRAVEL-ROOTS- THIN TOPSOIL	SEE TEST RESULTS
	SM SC		x		<u> </u>		MEDIUM BROWN HIGHLY CEMENTED SILTY FINE TO MEDIUM SAND W/CALCAREOUS WEBBING	
-	SM		x				GRAY VERY FINESANDY SILT/SILTY SAND	
-	SM SC		x				GRAY STRONGLY CEMENTED WELL GRADED	
10	SM		x				SAND, VERY CALCAREOUS. LENSES OF SLIGHTLY PLASTIC SILT W/TRACES OF SOFT WHITE CALCAREOUS MATEIAL	
							GRAY VERY FINE SANDY SILT/SILTY SAND- MEDIUM DENSE	· ·
							BOTTOM OF EXCAVATION NO WATER ENCOUNTERED	

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					TEST PIT NO. 14	· 1		
	SM GP	x	x		GRAY FINE SANY DILT/SILTY SAND W/ ANGULAR AND SUBANGULAR SMALL ROCKS OF VARIOUS TYPES	SE	E TEST RE	SULT
	SM SM SC	x		$\vdash$	LT. GRAY VERY FINE SILTY LENSE	V	······································	
-	SM	x	x		WHITE/GRAY HIGHLY CEMENTED CALCAREOUS LENSE GRAY VERY FINE SANDY SILT/SILTY SAND	V		
	SM	x			GRAY LOOSE TO MEDIUM DENSE FINE TO MEDIUM SANDY SILT/SILTY SAND		· .	· .
-	SM SC	x			GRAY FINE TO MEDIUM SANDY SILT, FIRM TO STIFF, SLIGHTLY MOIST, APPEARS SLIGHTLY PLASTIC			
-					BOTTOM OF EXCAVATION NO WATER ENCOUNTERED			
)B	NO.:	45024			LOG OF TEST PIT	_	FIGURE:	7

1								39 METHOD OF DRILLING: CASE 580 C EX	
	LOG	GED	BY:	P.	н.	GROU		EVATION: E.G. LOCATION: SEE LOCATION	PLAN
-	DEPTH (FEET)	<b>CLA88IFICATION</b>	BLOWS/FOOT	UNDISTURBED SAMPLE	BULK SAMPLE	CONTENT (%)	IN. PLACE DRY Density (PCF)	TEST PIT NO. 15	SOIL TEST
		SM	N/A		х			LT. BROWN VERY FINE SANDY SILT/SILTY SAND W/COARSE SAND & GRAVEL	SEE TEST RESULTS
	- - 5_	SM GM		x	x		-	(RECENT ALLIVIUM)THIN TOPSOIL BROWN SILTY WELL GRADED SAND W/GRAVEL (COARSER)	
	1	SM SC		X				BROWN SILTY COARSE SAND, SLIGHTLY	
	- - 10 -	SM		x				CEMENTED HIGHLY CEMENTED VERY CALCAREOUS LENSE BROWN SILTY FINE TO MEDIUM SAND. CEMENTED, CALCAREOUS WEBBING	
	- 15- -							BOTTOM OF EXCAVATION NO WATER ENCOUNTERED	

LOG	DGGED BY: P.H. GROUND ELEVATION: E.G. LOCATION: SEE LOCATION PLAN							
			•				TEST PIT NO. 16	
•	SM	N/A		x			MED. BROWN SILTY SAND, TRACE OF GRAVEL-LOOSE	SEE TEST RESULTS
-	SM		x	x			BROWN CEMENTED SILTY FINE TO MEDIUM SAND-DIFFICULT DIGGING	
δ	SM SC		x				WHITISH GRAY SILTY FINE TO MEDIUM SAND, VERY STRONGLY CEMENTED	
10-							MODERATE REJECTION	
-							BOTTOM OF EXCAVATION NO WATER ENCOUNTERED	
- 15- -								
JOB	NO	· 4	1 5024	ļ		1	LOG OF TEST PIT	FIGURE: 8

AMERICAN ENGINEERING LABORATORIES

DAT	EOB	SER	VED:	NOV	7.16	, 198	METHOD OF DRILLING: CASE 580 C E	KTENDAHOE	
LOG	LOGGED BY: P.H. GROUND ELEVATION: E.G. LOCATION: SEE LOCATION PLAN								
DEPTH (FEET)	<b>CLASSIFICATION</b>	BLOWS/FOOT	UNDISTURBED SAMPLE	BULK SAMPLE	MOISTURE CONTENT (%)	IN. PLACE DRY DENSITY (PCF)	TEST PIT NO. 17	SOIL TEST	
	5	8	5	9(	.0	≦ā			
-	SM			x			LT. TO MEDIUM BROWN SILTY VERY FINE TO MEDIUM SAND W/TRACE OF GRAVEL VERY LOOSE	SEE TEST RESULTS	
5	SM		x				BROWN CEMENTED SILTY FINE TO MEDIUM SAND	· ··· · · · · · · · · · · · · · · · ·	
-	SM SC		x:				WHITISH GRAY VERY STRONGLY CEMENTED SILTY FINE SAND (CALCAREOUS) STRONG REJECTION		
10							BOTIOM OF EXCAVATION NO WATER ENCOUNTERED		
15-									

\_ . . .

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OGGED BY: P.H. GROUND ELEVATION: E.G. LOCATION: SEE LOCATION PLAN							
<b>.</b>	TEST PIT NO. 18						
SM N/A X	BROWN SILTY SAND W/GRAVEL LOOSE TO MEDIUM DENSE WITH NUMEROUS ROOTS	SEE TEST RESULT					
5-SM X X	GRAYISH BROWN VERY DENSE CALCAREOUS STRONGLY CEMENTED WELL GRADED SAND.						
- X ·	6" LENSE OF BROWN FINE SAND W/SLIGHT CEMENTATION AT 7' 6"						
	REJECTION						
- - δ	BOTTOM OF EXCAVATION NO WATER ENCOUNTERED						
	· ·						
<b>јов NO.:</b> 45024	LOG OF TEST PIT	FIGURE: 9					



# **APPENDIX C1.2**

# Track Nos. 31286 and 91269, City of Moreno Valley



OFFICES IN THE COUNTIES OF ORANGE • SAN DIEGO • RIVERSIDE • LOS ANGELES • SAN BERNARDINO

> May 3, 2004 J.N. 289-03

Mr. Mike McGovern RICHMOND AMERICAN HOMES OF CALIFORNIA, INC. 16845 Von Karman Avenue, Suite 100 Irvine, CA 92606

# Subject: Geotechnical Review of Rough Grading Plans, Tract Nos. 31268 and 31269, City of Moreno Valley, Riverside County, California

References: See Attached List

Dear Mr. McGovern:

In accordance with your request, we have reviewed the 40-scale rough grading plans for the site prepared by Adams Streeter Civil Engineers, Inc., dated February, 2004. This report presents a summary of our review of the plans, as well as our geotechnical recommendations for rough grading of the site and for design and construction of foundations for the proposed structures.

Petra Geotechnical, Inc., appreciates this opportunity to be of continued service. Please call if you have any questions pertaining to the information presented in this report.

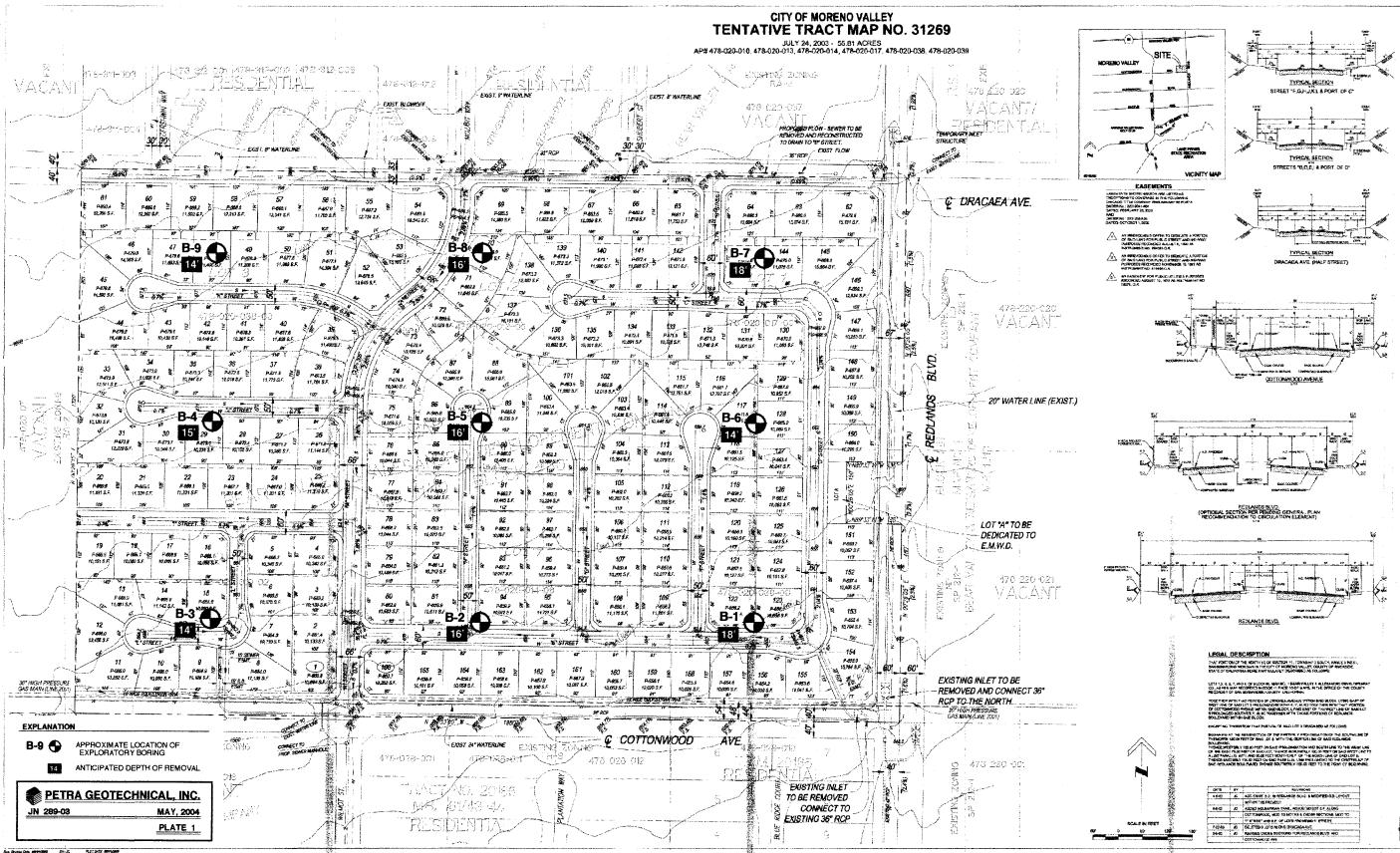
Respectfully submitted,

PETRA GEOTECHNICAL, INC.

Robert W. Ruff Vice President

#### PETRA GEOTECHNICAL, INC.

3185 Airway Avenue • Suite A • Costa Mesa • CA 92626 • Tel: (714) 549-8921 • Fax: (714) 549-1438



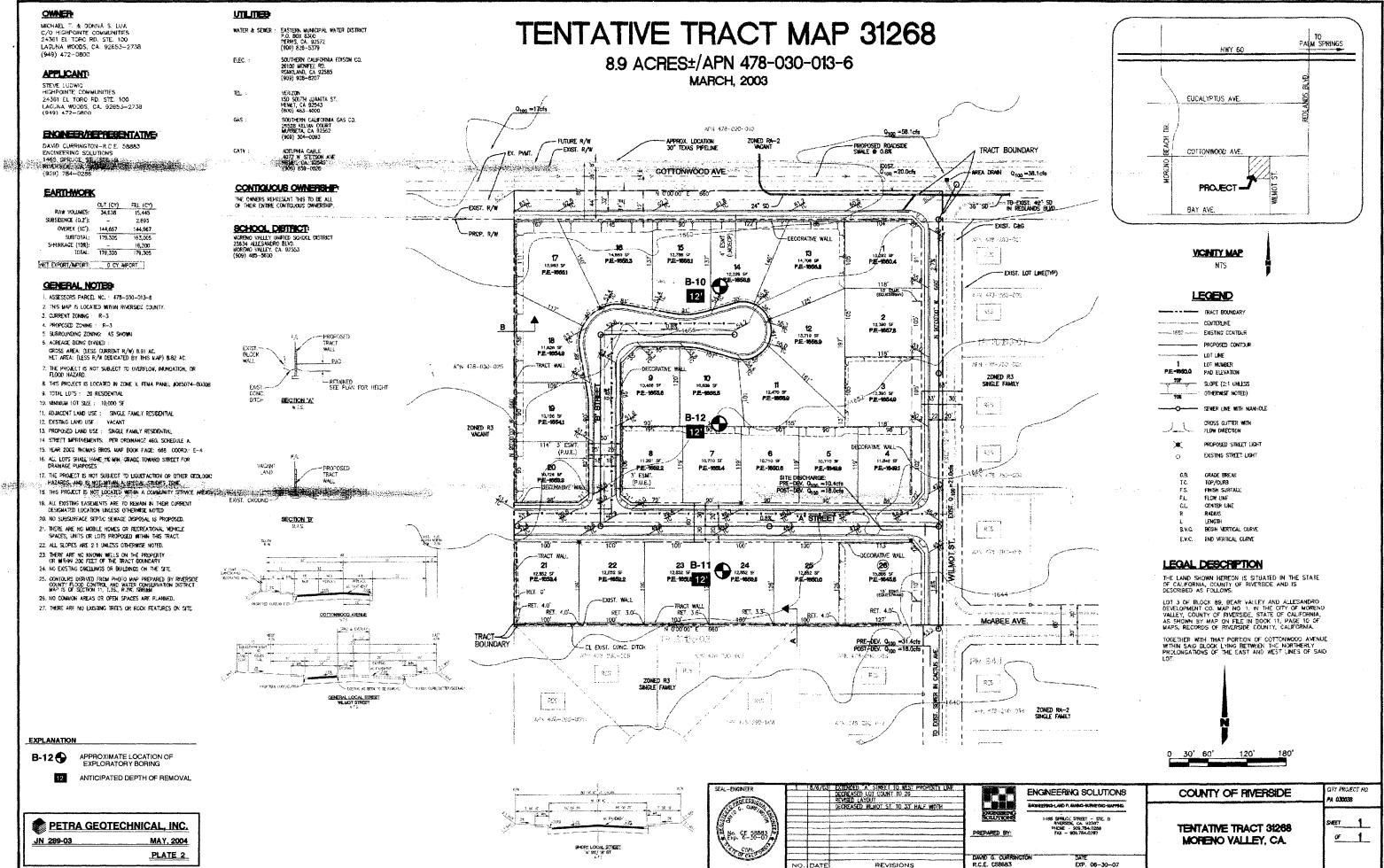
#### GENERAL NOTES

- 2 PROFESSIO CAMBRAL PLANDERGARISAN R-S CLATHO ZONING A4 - 2
- A PROPOSITO DOMINIC R 1
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- NAMES ARE NO KNOWN KNOTHO WELS, WE DIR COLUMN, CENTRESCO BOAGE LEADS FRADE, REVEAU, CLUSCHILL STORE ON CANDER OR UNDER DIV STORED WITHIN THE PREJECT SCHWARES.
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#### LEGEND

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# Key to Soil and Bedrock Symbols and Terms



Unif	fie	d Se	oil C	lassification Syste	em		
		_	the f	GRAVELS	Clean Gravels	GW	Well-graded gravels, gravel-sand mixtures, little or no fines
1	oarse-grained Soils /2 of materials urger than #200 sieve is about		more than half of coarse	(less than 5% fines)	GP	Poorly-graded gravels, gravel-sand mixtures, little or no fines	
ii i		d e	fraction is larger than #4	Gravels	GM	Silty Gravels, poorly-graded gravel-sand-silt mixtures	
E SI			sieve	with fines	GC	Clayey Gravels, poorly-graded gravel-sand-clay mixtures	
Soi S			SANDS	Clean Sands	SW	Well-graded sands, gravelly sands, little or no fines	
Lug I		lie/	more than half of coarse	(less than 5% fines)	SP	Poorly-graded sands, gravelly sands, little or no fines	
US à			fraction is smaller than #4	Sands	SM	Silty Sands, poorly-graded sand-gravel-silt mixtures	
· · · · · · · · · · · · · · · · · · ·	Λ		ible	sieve	with fines	SC	Clayey Sands, poorly-graded sand-gravel-clay mixtures
			tandard visible t			ML	Inorganic silts & very fine sands, silty or clayey fine sands,
oils si si	S		S S	SILTS &			clayey silts with slight plasticity
l s ia a	007#		U.S. article	Liquid		CL	Inorganic clays of low to medium plasticity, gravelly clays,
	_	υ υ	<u> </u>		an 50	CL	sandy clays, silty clays, lean clays
ia ii		sieve	່ບ			OL	Organic silts & clays of low plasticity
- <u>1</u>	ine-gr: 1/2 of smaller s		No. Mall	SILTS &	CLAYS	MH	Inorganic silts, micaceous or diatomaceous fine sand or silt
ine 1/2	nıa		The T sm	Liquid	Limit	СН	Inorganic clays of high plasticity, fat clays
<u>"</u> ^ '	s		F	Greater T	'han 50	OH	Organic silts and clays of medium-to-high plasticity
				Highly Organic Soils		PT	Peat, humus swamp soils with high organic content

_		a: a:		· · ·
Desci	ription	Sieve Size	Grain Size	Approximate Size
Boulders		>12"	>12"	Larger than basketball-sized
Cobbles		3 - 12"	3 - 12"	Fist-sized to basketball-sized
<u> </u>	coarse	3/4 - 3"	3/4 - 3"	Thumb-sized to fist-sized
Gravel	fine	#4 - 3/4"	0.19 - 0.75"	Pea-sized to thumb-sized
	coarse	#10 - #4	0.079 - 0.19"	Rock salt-sized to pea-sized
Sand	medium	#40 - #10	0.017 - 0.079"	Sugar-sized to rock salt-sized
	fine	#200 - #40	0.0029 - 0.017"	Flour-sized to sugar-sized to
Fines		Passing #200	<0.0029"	Flour-sized and smaller

Labora	tory Test Abbreviations		
MAX	Maximum Dry Density	MA	Mechanical (Particle Size) Analysis
EXP	Expansion Potential	AT	Atterberg Limits
SO4	Soluble Sulfate Content	#200	#200 Screen Wash
RES	Resistivity	DSU	Direct Shear (Undisturbed Sample)
pН	Acidity	DSR	Direct Shear (Remolded Sample)
CON	Consolidation	HYD	Hydrometer Analysis
SW	Swell	SE	Sand Equivalent
CL	Chloride Content	OC	Organic Content
RV	R-Value	COMP	Mortar Cylinder Compression

Modifiers						
Trace	< 1 %					
Few	1 - 5%					
Some	5 - 12 %					
Numerous	12 - 20 %					

Sam	pler and Symbol Descriptions
Ā	Approximate Depth of Seepage
¥	Approximate Depth of Standing Groundwater
	Modified California Split Spoon Sample
	Standard Penetration Test
	Bulk Sample Shelby Tube
Ø	No Recovery in Sampler

Bedrock H	Iardness
Soft	Can be crushed and granulated by hand; "soil like" and structureless
Moderately Hard	Can be grooved with fingernails; gouged easily with butter knife; crumbles under light hammer blows
Hard	Cannot break by hand; can be grooved with a sharp knife; breaks with a moderate hammer blow
Very Hard	Sharp knife leaves scratch; chips with repeated hammer blows

#### Notes:

Blows Per Foot: Number of blows required to advance sampler 1 foot (unless a lesser distance is specified). Samplers in general were driven into the soil or bedrock at the bottom of the hole with a standard (140 lb.) hammer dropping a standard 30 inches unless noted otherwise in Log Notes. Drive samples collected in bucket auger borings may be obtained by dropping non-standard weight from variable heights. When a SPT sampler is used the blow count conforms to ASTM D-1586

Project: M	loreno 192			E	Boring	No.:	<b>B-</b> 1		
Location: T	entative Tracts 31268 &	31269, Moreno Val	ley, Calif.	E	Elevatio	on:	1656		
Job No.: 2	89-03	Client: Richmon	d American	I	Date:		8/18/03		
Drill Method:	Hollow-Stem Auger	Driving Weight:	140 lbs / 30 in	I	Logged	By:	DPO/EP		
			w	Sam	oles	Lat	ooratory Test	\$	
Depth Lith- (Feet) ology	М	aterial Description		a t e r	Blows Per 6-inch	ou rl	Moisture Content (%)	Dry Density (pcf)	Othe Lab Tests
	ALLUVIUM (Qal) <u>Silty Sand (SM</u> ): Olive gra dense; fine-grained sand; p	y; dry to slightly moist; borous.	loose to medium		3 3 6		4.8	99.2	MA EXI SO pH RE CL RV
	<u>Silty Sand (SM)</u> : Olive gra fine-grained sand; some m fine-grained gravel; porous	edium- and coarse-grain	dense; very fine- to ed sand, trace of	-	3 5 8		4.9	99.5	
- 10	@9 Feet: Less porosity.				4 5 9		3.4	103.7	
	@12 Feet: Occasional roo	ot filaments.			4 5 10		4.0	98.9	
- 15	<u>Silty Sand (SM)</u> : Olive gra fine-grained sand.	ıy; dry; medium dense; v	ery fine- to		5 7 7		2.0	101.0	
- 20	Sand (SP): Gray; dry to sli sand; some light brown sil friable than material above	t inclusions, less porous,	ise; fine-grained slightly more		5 6 9		2.0	98.6	со
	Silty Sand (SM): Gray; dry pods of course grained san	; medium dense; fine-gr d, slightly porous.	ained sand; some		5 7 11		4.0	100.5	
	@24 Feet: Becomes dry to white silt, some		clusions of tan and		4		6.9	97.0	со
								PLA	ATE A

Pr	oject	: M	loreno 192			I	Boring	No.:	<b>B-</b> 1		
Lo	ocatic	on: <b>T</b>	entative Tracts 31268 &	31269, Moreno Val	ley, Calif.	I	Elevatio	on:	1656		
Jo	b No	.: 28	89-03	Client: Richmon	d American	I	Date:		8/18/03		
Dr	rill M	lethod:	Hollow-Stem Auger	Driving Weight:	140 lbs / 30 in	I	Jogged	By:	DPO/EP		
						w	Sam	++		oratory Tests	
1	pth eet)	Lith- ology	М	aterial Description		a t e r	Blows Per 6-inch	CB ou r1 ek	Moisture Content (%)	Dry Density (pcf)	Other Lab Tests
							57				
- 3	0 —		<u>Silty Sand (SM)</u> : Reddish-ł fine-grained sand; some mo	prown; moist; medium d edium- to coarse-grained	ense to dense; I sand, porous.		7 11 13		3.7	115.7	
3			Silty Sand (SM): Reddish-I fine-grained sand; moderat	prown; moist; medium c ely to very porous.	ense to dense;		6 9 10		÷		
4 	10 —		<u>Silty Sand (SM)</u> : Reddish-l dense; fine-grained sand; s	orown to light grayish bi light pinhole porosity, a	own; slightly moist; bundant caliche.		11 25 43				
EXPLORATION LOG - V3 289-03.GPJ PETRA GDT 4/29/04	15 —		Sandy Silt to Silty Sand (M fine-grained sand; pods of porosity.	( <u>L/SM</u> ): Gray; dry to slin medium- and coarse-gra	ghtly moist; dense; ined sand, slight		15 24 26				
ON LOG - V3			Sandy Silt to Silty Sand (M fine-grained sand; some ler	(L/SM): Reddish-brown uses of medium- and coa	; moist; dense; arse-grained sand,		14				
ORATIC										PLA	TE A-2
EXPL			-	Petra Geote	echnical, Inc.	•					

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- Annual Contraction

Depth Lith- $t = Per \begin{bmatrix} 0 \\ r \end{bmatrix} $ Content Density La	Job No	o.: 28	89-03	Client: Richmon	d American	[	Date:		8/18/03			
Depth (Feet)Lith- ologyMaterial Description $W_{a}$ t e rBlows Per r t e rC B u t t e rMoisture Density La (%)Dry Density (pcf)Oth La (%)	Drill N	/lethod:	Hollow-Stem Auger	Driving Weight:	140 lbs / 30 in	I	Logged	By:	DPO/EP	<u> </u>		
$\begin{array}{c c c c c c c c c c c c c c c c c c c $				- <u></u>		w						
Total Depth = 50.5 Feet	Depth (Feet)		М	aterial Description		t	Per	0 U	Content	Density	Othe Lab Test	
Total Depth = 50.5 Feet			slight porosity.			_						
										, Alexandre de la companya de la com		
			Total Depth = 50.5 Feet									
				red.					-			
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Project		oreno 192	212(0) Manage Mal			Boring Elevation		в-2 1660		
Locatio		entative Tracts 31268 &			+		501:			
Job No	D.: 28	39-03	Client: Richmon			Date:		8/18/03		
Drill M	1ethod:	Hollow-Stem Auger	Driving Weight:	140 lbs / 30 in	I	Logged	-	DPO/EP		
-		N	aterial Description		W a	Samp Blows	CB	Lat Moisture	Doratory Test	Othe
Depth (Feet)	Lith- ology	141			t e r	Per 6-inch	ou rl ek	Content (%)	Density (pcf)	Lab Test
		<u>ALLUVIUM (Qal)</u> <u>Silty Sand (SM)</u> : Light gra some medium- and coarse-	yish-brown; dry; loose; grained sand, porous, ro	fine-grained sand; otlets.		2				
- - - 5 —		Silty Sand (SM): Light gra	yish-brown; dry to sligh	tly moist; loose to		43		5.7	94.4	
-		medium dense; fine-graine sand, moderately porous, s	d sand; some medium- a ome caliche.	nd coarse-grained		6 7				
- - 10 —		Silty Sand (SM): Light gra dense; fine- to coarse-grain sub-angular, trace rootlets.	ned sand; rare fine-grain	st; loose to medium ed gravel,		4 6 6		1.6	108.5	
- - -		<u>Silty Sand (SM)</u> : Pale olive dense; fine-grained sand; s some gravel, slightly porou	ome medium- and coars	moist; medium e-grained sand,		3 4 4		3.7	103.4	
 15						3 4 6		4.6	103.8	
						2 3 4		4.4	104.7	CO
- 20 - -		Sand to Silty Sand (SP/SM dense; fine-grained sand; s	(): Pale olive-gray; mois ome medium- and coars	; loose to medium e-grained sand.		2 3 3		3.2	102.3	
-		Silty Sand (SM): Pale olive fine-grained sand; some m porous.	e-gray; slightly moist; m edium- and coarse-grain	edium dense; ed sand, moderately		3 4 6		9.7	98.7	CO
	<u>r 67 (* 1. 1</u>								PL	ATE A

	90.02	Client: Richmon	ley, Calif.	+-	ate:		8/18/03		
	89-03								
Drill Method	: Hollow-Stem Auger	Driving Weight:	140 lbs / 30 in		ogged		DPO/EP		
Depth Lith- Feet) ology	Ма	terial Description		t e	Sam Blows Per 6-inch	C B o u	Lat Moisture Content (%)	Dry Dry Density (pcf)	Oth La Tes
	<u>Silty Sand (SM)</u> : Pale olive- dense; fine-grained sand; so micaceous, no visible porosi	gray; slightly moist to me medium- and coars ity,.	moist; medium e-grained sand,	-	4 5 6				
30 -	@29 Feet: Some black iron	-oxide mineral grains.	· · · · · · · · · · · · · · · · · · ·		6 6 7				
	Total Depth = 30.5 Feet No Groundwater Encounter	ed.						<b>.</b> .	
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Project	t: M	loreno 192			E	Boring	No.:	B- 3		
Locatio	on: T	entative Tracts 31268 & 3	31269, Moreno Val	ley, Calif.	E	Elevatio	on:	1664		
Job No	o.: 28	89-03	Client: Richmon	d American	I	Date:		8/18/03		
Drill N	1ethod:	Hollow-Stem Auger	Driving Weight:	140 lbs / 30 in	L	ogged	By:	DPO/EP		
					w	Sam	ples	Lat	oratory Test	s
Depth (Feet)	Lith- ology		terial Description	· · ·	a t e r	Blows Per 6-inch	CB url ek	Moisture Content (%)	Dry Density (pcf)	Other Lab Tests
		ALLUVIUM (Qal) Silty Sand (SM): Pale olive- sand; some pods of medium moderately porous, trace roc	- and coarse-grained sa	se; fine-grained and, slightly to		4		4.8	106.8	
_ 5 _ _						4 7 11				
		@7 Feet: Becomes moderat	ely porous, some gray	silt inclusions.		3 5 5		5.7	92.9	
- 10 		@10 Feet: Becomes modera	ately to very porous.			4 4 8		6.9	95.8	CON
 15		@13 Feet: Decrease in porc	sity.			4 5 7		6.4	97.6	
		Silty Sand (SM): Pale olive- sand; slight to moderate pore	gray; dry; medium den osity.	se; fine-grained	-	4 5 8		6.5	99.8	CON
20		Sand (SP): Gray; dry; mediu sand; some gravel up to 0.75		- to coarse-grained	-	6 7 8		1.3	104.3	
20   		Silty Sand (SM): Pale olive- to dense; fine-grained sand; sand, slightly to moderately	some pods of medium	<ul> <li>to coarse-grained</li> </ul>	-	9 11 15		7.4	115.4	
,	لسلاحاتهما	·		· · · · · · · · · · · · · · · · · · ·			<u> </u>		PLA	ATE A-6
			Petra Geote	echnical, Inc.						

Location:     Tertative Tracts 31268 & 31269, Moreno Valley, Calif.     Elevition:     1664       Job No.:     289-03     Client:     Richmond American     Date:     \$V18/03       Drill Method:     Hollow-Stem Auger     Driving Weight:     140 lbs / 30 in     Logged By:     DPO/JEP       Uppth     Lift:     Material Description     \$V18/03     Distance Tests     Blows (16 Mosture Doing)     Distance Tests       10     User     Sample     Luborator Tests     Blows (16 Mosture Doing)     Distance Tests       10     Sim Sand (Mi) Pate elive-gray:     dry to slightly moist medium dense in educately porous, pores filled with calicle, no burrows.     7     9.9     104.8       30     Classer Sill (MI):     Medium brown; moist; stiff to very stiff; some     7     9.9     104.8       31     Classer Sill (MI):     Medium brown; moist; stiff to very stiff; some     7     9     10       35     Classer Sill (MI):     Medium brown; moist; stiff to very stiff; some     7     9     15       15     Total Depth = 38.5 Feet     No Groundwater Encoantered.     7     9     15       16     Sin Mi : Medium brown; moist; stiff to very stiff; some     7     9     15	Project:	Moreno 192		· · · · · · · · · · · · · · · · · · ·	E	Boring	No.:	B- 3		
Drill Method: Hollow-Stem Auger     Driving Weight:     140 lbs / 30 in     Logged By:     DPO/EP       Depth     Lith- (Feet)     Material Description     W *     Samples Blows C     Laboratory Tests       W     Site State     Dry     Other     Destity     Destity       Site State     Site State     Dry     Other     Content     Dry       0     Site State     Site State     Dry     Other       0     Site State     Site State     Site State     Dry     Other       0     Site State     Site State     Site State     Dry     Other       0     Site State     Site State     Site State     Dry     Other       0     Site State     Site State     Site State     Dry     Other       0     Site State     Site State     Site State     Provide State     Provide State       0     Site State     Site State     Provide State     Provide State     Provide State       0     Site State     Site State     Site State     Provide State     Provide State       0     Site State     Site State     Provide State     Provide State     Provide State       0     Site State     Site State     Provide State     Provide State     Provide State </th <th>Location</th> <th>n: Tentative Tracts 31268 &amp;</th> <th>31269, Moreno Val</th> <th>ley, Calif.</th> <th>F</th> <th>Elevatio</th> <th>on:</th> <th>1664</th> <th></th> <th></th>	Location	n: Tentative Tracts 31268 &	31269, Moreno Val	ley, Calif.	F	Elevatio	on:	1664		
Depth       Lith- (reet)       Material Description       Description       Other Lab	Job No.	: 289-03	Client: Richmon	d American	1	Date:		8/18/03		
Depth       Luh- (Feet)       Material Description       Waterial Description       Blows C is in Moisture Content is in Content Description       Doy Desists is in Content Description         1       Silty Sand (SM): Pale olive-gray; dry to slightly moist; medium dense to dense; fine-grained sand; rare fine-grained gravel, sub-rounded, moderately porous, pores filled with caliche, no burrows.       7       9.9       104.8         30       -	Drill Me	ethod: Hollow-Stem Auger	Driving Weight:	140 lbs / 30 in	I	logged	By:	DPO/EP	)	
Depth       Lin- (Feet)       Material Description       a       Per e       b       Content Content       Density (%)       Density (					w	Samp	T 1		T	
30       13         30		Lith-	aterial Description		a t e	Per	o u r l	Content	Density	Lab
PLATE A-		Silty Sand (SM): Pale olive         to dense; fine-grained sand         moderately porous, pores f         Clayey Silt (ML): Medium         fine-grained sand, parting         Clayey Silt (ML): Medium         fine-grained sand, moderate         Clayey Silt (ML): Medium         fine-grained sand, moderate         Total Depth = 38.5 Feet	l; rare fine-grained grave illed with caliche, no bu brown; moist; stiff to v surfaces. brown; moist; stiff to v e porosity.	el, sub-rounded, irrows. ery stiff; some		13 20 10 20 29 7 9				
PLATE A-	06 - V3 289-03 GPJ PETRA GDT									
Detre Costeshnicel Inc						. <u></u>	<u>I</u>	L	ـــــــــــــــــــــــــــــــــــــ	TE A-7
	LORAT		Detre Cest							

### Petra Geotechnical, Inc.

Project: Mo	reno 192		5.00.00	F	Boring	No.:	B- 4	(	
Location: Ter	ntative Tracts 31268 &	31269, Moreno Vall	ey, Calif.	I	Elevatio	on:	1674		
Job No.: 289	9-03	Client: Richmon	l American	I	Date:		8/18/03		
Drill Method: H	Iollow-Stem Auger	Driving Weight:	140 lbs / 30 in	1	Logged	By:	DPO/EP	)	
			<u></u>	w	Sam	oles	Lal	boratory Test	5
Depth Lith- (Feet) ology	Ma	terial Description		a t e r	Blows Per 6-inch	CB ou rl ek	Moisture Content (%)	Dry Density (pcf)	Oth Lat Test
- 5	ALLUVIUM (Qal) Silty Sand (SM): Pale olive- fine-grained sand; some me porosity, roothairs, some ca @6 Feet: Becomes fine- to caliche.	dium- to coarse-grained liche filled pores.	sand, moderate		4 4 6 4 4 7		4.1 2.8	104.0	
10	<u>Silty Sand (SM)</u> : Pale olive- fine-grained sand; some coa moderate porosity.	gray; dry; loose to med rse-grained sand, rare f	ium dense; ine-grained gravel,		4 7 8		3.6	101.8	
	@12 Feet: Becomes slightly	y porous.			5 7 8		4.3	96.7	
	<u>Silty Sand (SM)</u> : Pale olive- fine-grained sand; some coa moderately porous.	gray; dry; loose to med rse-grained sand, rare g	ium dense; gravel up to 0.5",		6 9 12		2.6	106.9	
	<u>Sand (SP)</u> : Pale olive-gray; coarse-grained sand.	dry; medium dense to d	ense; fine- to	-	4 7 11		1.2	109.9	cc
	<u>Clayey Silt (ML)</u> : Pale olive micaceous, caliche infilling.	-gray; slightly moist; st	iff to very stiff;		10 11 12		5.9	112.5	
	<u>Silty Sand (SM)</u> : Pale reddis dense; fine-grained sand; so	sh-brown; slightly mois me medium- and coarse	t; medium dense to e-grained sand.		7				
								PLA	ATE 2
		Petra Geote	chnical, Inc.					4 	

Locati	ion: T	entative Tracts 31268 &			_ <b></b> _	Elevati		1674	-				
Job N	o.: 28	89-03	Client: Richmon	d American		Date:		8/18/0	)3				
Drill N	Method:	Hollow-Stem Auger	Driving Weight:	140 lbs / 30 in	Logged By:			: DPO/	DPO/EP				
				·	w	Sam		-	- T .	atory Tests	1		
Depth (Feet)	Lith- ology	М	aterial Description		a t e r	Blows		Conter		Dry Density (pcf)	Other Lab Tests		
						8 9		-			-		
 		<u>Silty Sand (SM)</u> : Reddish- sand; very slight pinhole p	brown; slightly moist; d prosity.	ense; fine-grained		6 9 16		-		e .			
- 30 -		<u>Silty Sand (SM)</u> : Yellowis sand; very slight pinhole p	h-brown; slightly moist; prosity.	dense; fine-grained		7 7 9							
		Total Depth = 31.5 Feet No Groundwater Encounte	red.										
		<b> </b>		<u>.</u>	-								
								-					
					-								
	_ <u></u>		······································		1	I		<b>l</b>			ATE A		

	Fentative Tracts 31268 &				evatio		1671		
Job No.: 2	289-03	Client: Richmond A	American	Da	te:		8/18/03		
Drill Method	i: Hollow-Stem Auger	Driving Weight: 1	40 lbs / 30 in	Lo	gged	By:	DPO/EP		
				w-	Samp			oratory Test	T
Depth Lith- (Feet) ology		aterial Description		t e	lows Per -inch	$\begin{array}{c c} C & B \\ o & u \\ r & 1 \\ e & k \end{array}$	Moisture Content (%)	Dry Density (pcf)	Othe Lab Test
	ALLUVIUM (Qal) Silty Sand (SM): Pale olive some caliche filled pores.	e-gray; dry; loose; fine-grair	ned sand; porous,		-			-	
				- And	4 4 5	X	4.4	96. <b>9</b>	SO p <del>l</del> RE
- 5	Silty Sand (SM): Pale olive medium-grained sand, less	e-gray; dry; loose; fine-grain silt.	ned sand; some		4 5 8		4.3	97.7	CI
	Silty Sand (SM): Pale olive sand; some silt inclusions,	e-gray; dry; medium dense; porous.	fine-grained		5 6 7		4.9	106.8	
- 10	@11 Feet: some medium- some caliche fil	to coarse-grained sand, mo led pores.	derately porous,		5 6 9		4.7	106.8	со
- 15	@14 Feet: Dry, some grav	rel up to 2.0".		a	5 7 8		1.7	118.6	
	Sand (SP): Pale gray; dry; coarse-grained sand; grave	medium dense to dense; fin I up to 1.5".	e- to		7 11 14		1.1	100.0	
- 20 -	@20 Feet: Becomes dense	e to very dense, gravel up to	0.5".		9 11 12		1.2	113.1	
	<u>Silty Sand (SM)</u> : Pale tan- fine-grained sand; rare silt	gray; dry; medium dense to inclusions, some pinhole po	dense; orosity.		4 5 7				

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loh Nia :	90.02	Client: Richmon	lley, Calif.		Date:		8/18/03		
	289-03								
Drill Method	: Hollow-Stem Auger	Driving Weight:	140 lbs / 30	) in	Logged		DPO/EP	ooratory Tests	
Depth Lith- (Feet) ology	N	laterial Description			t Per	CB ou r1 ek	Moisture Content (%)	Dry Density (pcf)	Othe Lat Test
-	<u>Clayey Sand (SC)</u> : Dark by fine-grained sand; some m some caliche filled pores.	rown; moist; dense to ve edium-grained sand, sli	ery dense; ght pinhole por	osity,	6 12 14		6.2	117.8	
	Total Depth = 27.5 Feet No Groundwater Encounte	ered.							

Project:		loreno 192			-			B- 6					
Locatio	n: <b>T</b>	entative Tracts 31268 &	31269, Moreno Val	ley, Calif.	E	Elevatio	on:	1666					
Job No.	: 28	89-03	Client: Richmon	d American	I	Date:		8/18/03					
Drill M	ethod:	Hollow-Stem Auger	Driving Weight:	140 lbs / 30 in	I	Logged	By:	DPO/EP					
			<u></u>		w	Samp			boratory Test	\$			
Depth (Feet)	Lith- ology	M	aterial Description		a t e r	Blows Per 6-inch	CB url ek	Moisture Content (%)	Dry Density (pcf)	Other Lab Tests			
		ALLUVIUM (Qal) Silty Sand (SM): Pale olive sand; some gravel up to 0.7	e-gray; dry; medium den 75", slight pinhole poros	se; fine-grained ity.									
5						11 11 11		3.9	111.2				
		@7 Feet: Becomes very po	prous, increase in silt co	ntent.		5 5 6		8.6	92.7	со			
- 10		<u>Silty Sand (SM)</u> : Pale olive sand; some silt inclusions,	e-gray; dry; medium den slightly less porous, mir	se; fine-grained nor caliche stringers.	-	4 7 8		5.0	100.9				
- 15 -		@13 Feet: Trace increase	in moisture and caliche	content.		6 7 10		7.7	99.2				
		@16 Feet: Porous, pods of	f medium- to coarse-gra	ined sand.		4 6 6		2.6	101.1	со			
20 —		Sand (SP): Pale brownish- to fine-grained sand; micac	gray; dry; medium dense eous.	e to dense; very fine-	-	6 7 6		3.0	83.9	COI			
		@22 Feet: Some pods of c	oarse-grained sand and	fine-grained gravel.		4 5 5		3.0	98.3				
					1	L			PLA	TE A-			

Locatio	on: T	entative Tracts 31268 &	31269, Moreno Val	ley, Calif.	E	levati	on:	1666				
Job Nc	o.: 28	89-03	Client: Richmon	d American	D	ate:		8/18/03				
Drill M	1ethod:	Hollow-Stem Auger	Driving Weight:	140 lbs / 30 in	L	ogged	By:	DPO/EP				
			<u></u>		w-	Sam	ples	Lat	oratory Tests	s		
Depth (Feet)	Lith- ology	Μ	aterial Description		a t e	Blows Per 6-inch	CB ou rI ek	Moisture Content (%)	Dry Density (pcf)	Othe Lab Test		
- 30		<u>Silty Sand (SM)</u> : Pale olive sand; some medium- and c @28 Feet: Becomes slight <u>Sandy Clay (CL)</u> : Dark rec slight pinhole porosity, fev Total Depth = 31.5 Feet	oarse-grained sand, som	e pinhole porosity.		4 5 9 5 6 8 5 7 9						
		No Groundwater Encounte	ored.									
1			· · · · · · · · · · · · · · · · · · ·				ıt		PLA	TE A.		

Job No.: 2	289-03	Client: Richmon	d American		Date:		8/19/03 DPO/EP			
Drill Method	: Hollow-Stem Auger	Driving Weight:	140 lbs / 30 in	I	logged	By:				
		<u></u>		w	Sam	·	Lat	oratory Test	<u>r</u>	
Depth Lith- (Feet) ology		terial Description		a t e r	Blows Per 6-inch	CB ou rl ek	Moisture Content (%)	Dry Density (pcf)	Othe Lab Tests	
	ALLUVIUM (Qal) Silty Sand (SM): Pale olive- sand; some medium- and co	gray; slightly moist; lo arse-grained sand, moo	ose; fine-grained lerate porosity,.							
5 -					3 4		5.4	102.6		
					4					
10	@8 Feet: Becomes slightly fine-grained grave	more moist, very sligh l.	t porosity, some		5 8 11		5.6	106.9		
	Silty Sand (SM): Pale olive- fine-grained sand; some mee pinhole porosity.	gray; dry to slightly m lium- and coarse-grain	oist; medium dense; ed sand, slight		5 6 10		3.7	101.0	COI	
15 -	@14 Feet: Some silt inclusi roothairs.	ons, becomes moderat	ely porous, rare		5 6 10		6.8	104.5		
	@17 Feet: Becomes moder to 1/8".	ately porous to porous,	with some pores up		4 5 8		4.5	99.6	со	
20 -	@20 Feet: Decrease in porc	sity and silt content.			5 7 9		5.6	99.8		
					4 5 7		3.8	99.0	CO	

Projec	t: M	loreno 192			В	oring	No.:	<b>B-</b> 7		
Locati	on: T	entative Tracts 31268 &	31269, Moreno Val	ley, Calif.	E	levati	on:	1674		
Job No	o.: 28	39-03	Client: Richmon	d American	E	Date:		8/19/03		
Drill N	Method:	Hollow-Stem Auger	Driving Weight:	140 lbs / 30 in	L	ogged	By:	DPO/EP	)	
		<u></u>	· · · · · · · · · · · · · · · · · · ·	· · · · ·	w	Sam			poratory Tests	r==
Depth (Feet)	Lith- ology	Ma	terial Description	215 ALMAN	a t e	Blows Per 6-inch	CB ou rI ek	Moisture Content (%)	Dry Density (pcf)	Other Lab Tests
		Interbedded Sand and Silty to slightly moist; dense; fine to 1.0", no visible porosity.	Sand (SP/SM): Pale of e- to coarse-grained sar	ive-gray to gray; dry nd; Some gravel up		7 9 10		2.3	108.4	
- - 30 -		Silty Sand with trace Clay ( fine-grained sand; some coa pores caliche-filled.	<u>SC/SM)</u> : Reddish-brov urse-grained sand, sligh	vn; moist; dense; t porosity, some		6 9 11				
		Total Depth = 30.5 Feet No Groundwater Encounter	ed.							
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			Define Cast	echnical, Inc.						

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Project: Moren	On:Tentative Tracts 31268 & 31269, Moreno Valley, Calif.0.:289-03Client:Richmond American		E	Boring	No.:	B- 8			
Location: Tentat	tive Tracts 31268 &	31269, Moreno Val	ley, Calif.	F	Elevatio	on:	1678		
Job No.: 289-03	; ;	Client: Richmon	d American	ſ	Date:		8/19/03		
Drill Method: Holl	rill Method: Hollow-Stem Auger       Driving Weight:       140         eph       Lith- ology       Material Description         ALLUVIUM (Oal)       Silty Sand (SM): Pale olive-gray; dry; loose; fine-grained silt inclusions, rare gravel up to 3/8", porous.         5       Silty Sand (SM): Pale olive-gray; dry to slightly moist; lo fine-grained sand; less silt inclusions, slightly porous.         0       Silty Sand (SM): Pale olive-gray; dry to slightly moist; medianed sand; some medium- and coarse-grained sand fine-grained gravel, moderate porosity.	140 lbs / 30 in	I	Logged By: DPO/EP					
				w	Sam			oratory Test	1
• 1 1	Ma	terial Description		a t e r	Blows Per 6-inch	ou rl	Moisture Content (%)	Dry Density (pcf)	Other Lab Tests
Silt	y Sand (SM): Pale olive-	-gray; dry; loose; fine-g p to 3/8", porous.	rained sand; some		3 3 4		4.7	98.0	
— 5 — <u>Silt</u> — <u>Silt</u>	y Sand (SM): Pale olive- grained sand; less silt in	gray; dry to slightly mo nclusions, slightly poro	oist; loose; us.		4 5 8		4.1	100.0	
fine	-grained sand; some me	dium- and coarse-grain	pist; medium dense; ed sand, rare		6 7 9		6.3	101.0	
					4 5 6		3.8	104.6	CON
— 15 — <u>Silt</u> —	<u>y Sand (SM)</u> : Pale olive- -grained sand; decrease	gray; dry to slightly mo in silt content, slight to	oist; medium dense; moderate porosity.		5 5 7		3.2	106.2	CON
	<u>d (SP)</u> : Pale gray; dry; n	nedium dense to dense;	fine-grained sand.	• -	4 5 6		1.7	97.9	
- 20 - <u>Silty</u> - @2 <sup>2</sup>	<u>y Sand (SM)</u> : Pale tan; d l; some fine-grained gra	ry to slightly moist; de vel, slight pinhole porc	nse; fine-grained sity.		4 4 6		2.4	105.6	CON
- @24	4 Feet: Some silt inclusi	ions, slight porosity.	. · ·		6				
	<u> </u>				<u> </u>				TE A-1

cation:         Tentative Tracts 31268           b No.:         289-03	G 31207, MUICHU V 41	1079 CH111.		чеуян	on.	1678			
				Elevati			· · · · · · · · · · · · · · · · · · ·		
	Client: Richmon			Date:		8/19/03			
ill Method: Hollow-Stem Auger	Driving Weight:	140 lbs / 30 in		Logged		DPO/EP			
			Wa	Sam Blows	Y"	Lab Moisture	oratory Tests Dry	Oth	
pth   Lith-	Material Description		t e	Per	o u r l e k	Content	Density	La	
et) ology	· · · · · · · · · · · · · · · · · · ·		r.	6-inch 7	e k	(%)	(pcf)	Tes	
				9					
(a) 7 Feet: Becomes mo	ttled pale tan to gray, some	e caliche		7					
	the pure tail to gray, some	e currene.		10					
	. <u> </u>		-	12					
Total Depth = 28.5 Feet									
No Groundwater Encour									
		<u></u>	_						
						-			
							-		
		-							
							, 		
							PLA		

Project: N	Aoreno 192	••••••••••••••••••••••••••••••••••••••		Ē	Boring	No.:	B- 9				
Location: 1	Centative Tracts 31268 & .	31269, Moreno Val	ey, Calif.	E	Elevatio	on:	1682				
Job No.: 2	89-03	Client: Richmon	l American	ſ	Date:		8/19/03				
Drill Method	: Hollow-Stem Auger	Driving Weight:	140 lbs / 30 in	1	Logged	By:	DPO/EP				
				w	Samp	oles	La	boratory Test	s		
Depth Lith- (Feet) ology	Ma	terial Description		a t e r	Blows Per 6-inch	CB ou rI ek	Moisture Content (%)	Dry Density (pcf)	Other Lab Tests		
	ALLUVIUM (Qal)         Silty Sand (SM): Pale olive- sand; some fine-grained gra         Silty Sand (SM): Pale olive- fine-grained sand; some me- fine-grained gravel, modera         Silty Sand (SM): Pale olive- fine-grained gravel, modera         Silty Sand (SM): Pale olive- fine-grained gravel, modera         Silty Sand (SM): Pale olive- fine-grained gravel, little or         @13 Feet: Slight porosity.	gray; slightly moist; mo dium- and coarse-grain te porosity, caliche strin gray; slightly moist; mo dium- and coarse-grain	edium dense; ed sand, rare ngers.		6 7 7 4 6 7 4 5 5 4		4.6 4.9 3.4 2.4	104.1 98.9 99.0 109.2			
- 15	<u>Gravelly Sand (SP)</u> : Pale oli coarse-grained sand; some g	ive-gray; slightly moist; gravel up to 1.25".	dense; fine- to	-	6 5 7 13 13		1.2	113.9			
- 20 -	Silty Sand (SM): Pale olive- dense; fine-grained sand; m	gray; slightly moist to r oderate porosity.	noist; medium	-	4 6 9		12.1	100.0	CON		
	<u>Silty Sand (SM)</u> : Pale olive- fine-grained sand; some me medium- and coarse grained	dium- and coarse-grain	edium dense; ed sand, lenses of		6 8 8		4.1	103.6			
			<u>, 1997 - 1997 - 1998 - 1988 - 1988 - 1988 - 1988 - 1988 - 1988 - 1988 - 1988 - 1988 - 1988 - 1988 - 1988 - 198</u>		l	<u>L_L_</u>		PLA	TE A-1		
		Potra Goota	chnical, Inc.								

Project	t: <b>M</b>	loreno 192			Bor	ing No.:	B- 9			
Locatio	on: T	entative Tracts 31268 &	31269, Moreno Val	ley, Calif.	Elev	vation:	1682			
Job No	o.: 28	89-03	Client: Richmon	d American	Date	e:	. 8/19/03			
Drill M	An interview of the end	140 lbs / 30 in	Log	ged By:	DPO/EP					
		<u></u>			w	Samples	Lai	boratory Test	s	
		М	aterial Description		a Blo	ows C B		Dry	Other	
Depth (Feet)					le l'	er r l nch e k	Content	Density (pcf)	Lab Tests	
(1000)		Silty Sand with Clay (SM)	: Mottled olive and olive	gray; moist; dense;		5				
-		fine-grained sand; slight po	prosity, some caliche fill	ed pores.		0	-			
_						2	4			
							1			
-										
- 30 —								102.7	]	
		medium-grained sand; som	n-brown; moist; very still ne fine-grained sand incl	usions, slight		5 5	9.2	102.7		
-		pinhole porosity.	-			3				
-							-			
							4			
-							4			
- 35 —		Clayey Sand (SC): Reddisl	n-brown; moist; very stil	f/dense; fine-grained		3	1			
-		sand; some medium-graine	ed sand.			0				
_						2				
-							1		-	
-										
- 40 —		Claury Sand (SC), Daddid	hrown moist stiff to 1			\ <u> </u>	4			
		fine-grained sand; some m	edium- and coarse-grain	ed sand, slight		6				
_		pinhole porosity, caliche fi	lled pores.			6			8	
-							1			
-							4			
_		,					-			
4.0										
- 45		<u>Clayey Silt (ML)</u> : Pale oliv few coarse-grained sand, s	e-gray; moist; stiff; som	e fine-grained sand,		0		1		
-		rew coarse-gramed sand, s	ngin porosity, some can	the fifted poles.	1 1	2 6				
- :							+			
							-			
- :							1			
							1			
								PLA'	TE A-1	

Job No	Method: Hollow-Stem Auger	Client: Richmon		† r	Date:		8/19/03				
	h Lith- ology Silty Sand (SM): Reddi fine-grained sand; some porosity. Total Depth = 51.5 Fee		Driving Weight:	140 lbs / 30 in		logged	Bv	DPO/EP			
			Driving weight.	140 1057 00 m		Sam	-		oratory Tests		
Depth Feet)			aterial Description		W a t e r	Blows Per 6-inch	C B o u r l e k	Moisture Content (%)	Dry Density (pcf)	Other Lab Tests	
		<u>Silty Sand (SM)</u> : Reddish- fine-grained sand; some m porosity.	brown; moist; medium c edium- and coarse-grain	lense to dense; ed sand, slight		8 14 24					
			red.								
									PLAT		

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Projec	t: M	loreno 192			E	Boring	No.:	<b>B-10</b>				
Locati	on: T	entative Tracts 31268 &	31269, Moreno Val	ley, Calif.	E	Elevati	on:	1658				
Job No	o.: 28	89-03	Client: Richmon	d American	I	Date:		8/19/03				
Drill N	Method:	Hollow-Stem Auger	Driving Weight:	140 lbs / 30 in	I	Logged	By:	DPO/EP				
			<u></u>		w	Sam	ples	Lab	Laboratory Tests			
Depth (Feet)	Lith- ology	Ma	aterial Description		a t e r	Blows Per 6-inch	CB ou r1 ek	Moisture Content (%)	Dry Density (pcf)	Other Lab Tests		
		ALLUVIUM (Qal) Silty Sand (SM): Pale tan; c and coarse-grained sand, so	lry; loose; fine-grained me pinhole porosity, fe	sand; some medium- w roothairs.								
						3		4.2	105.0			
_ 5 -						5 5						
		<u>Silty Sand (SM)</u> : Pale gray; porosity.	dry; loose; fine-grained	I sand; slight		3 4 5		3.9	101.7			
10 		<u>Silty Sand (SM)</u> : Pale yello dense; fine-grained sand; so stringers.	wish-tan; dry to slightly ome pinhole porosity, m	moist; medium inor caliche		4 6 9		7.4	100.5	CON		
 15		<u>Silty Sand (SM)</u> : Pale yello dense; fine-grained sand; m	wish-tan; dry to slightly oderate porosity, minor	moist; medium caliche.		4 5 6		6.2	99.1			
		<u>Silty Sand (SM)</u> : Pale gray; slightly porous.	dry; medium dense; fin	e-grained sand;		3 5 7		3.4	99.5			
20		<u>Silty Sand (SM)</u> : Pale gray; fine-grained sand; some me fine-grained gravel, some si	dium- and coarse-grain	ed sand, some		4 6 9		7.1	96.3	CON		
		<u>Clayev Sand (SC)</u> : Reddish sand; slight porosity, very n	brown; slightly moist; hinor caliche.	dense; fine-grained		7 11 13		9.3	111.0			
l	<u> </u>				L		<u> </u>		PLA	TE A-21		
- 20 -				echnical, Inc.					PLA	T]		

Project:	Moreno 192			F	Boring	No.:	B-10			
Location:	Tentative Tracts 31268 & 3	31269, Moreno Val	ley, Calif.	E	Elevati	on:	1658		<u> </u>	
Job No.:	289-03	Client: Richmon	d American	I	Date:		8/19/03			
Drill Metho	d: Hollow-Stem Auger	Driving Weight:	140 lbs / 30 in	I	logged	l By:	DPO/EP			
		<u> </u>		w	Sam		Laboratory Tests			
Depth Lith- (Feet) ology		terial Description		a t e r	Blows Per 6-inch	o u r l	Moisture Content (%)	Dry Density (pcf)	Other Lab Tests	
-	<u>Sandy Clay (CL)</u> : Yellowish some medium- and coarse-g caliche stringers.	-brown; moist; stiff; fi rained sand, very sligh	ne-grained sand; t porosity, minor		11 17 17		6.4	119.0		
- - 30 - - -	<u>Clayev Sand (SC</u> ): Yellowis very slight pinhole porosity,	h-brown; moist; dense some caliche stringers	, fine-grained sand;		8 11 15					
- - 35 - - -	@35 Feet: Becomes very de	ense, no visible porosit	y.		16 22 26					
- - 40 - - -	@40 Feet: Becomes dense,	slight to moderate por	osity.		8 17 22					
- - 45 - - -	Silty Sand (SM): Pale gray; coarse-grained sand; some f	slightly moist; dense; f ine-grained gravel, no	ine- to visible porosity.		7 10 11					
-			<u></u>					DY A	Г <b>F</b> А_?	
		Petra Geote	echnical, Inc.					PLA	ſE.	

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locatio	on: <b>T</b>	entative Tracts 31268 &	31269, Moreno Val	ley, Calif.	I	Elevati	on:	1649			
ob No	.: 28	89-03	Client: Richmon	d American	I	Date:		8/19/03			
Drill M	lethod:	Hollow-Stem Auger	Driving Weight:	140 lbs / 30 in	I	Logged	By:	DPO/EP			
		· · · · · · · · · · · · · · · · · · ·		w	Sam	1		poratory Tests			
Depth Feet)	Lith- ology	М	aterial Description		a t e r	Blows Per 6-inch	o u	Moisture Content (%)	Dry Density (pcf)	Oth La Te	
						11 13					
30 —		@29 Feet: Some caliche fi	illed pores.			8 12					
50				<u> </u>		14					
		Total Depth = 30.5 Feet No Groundwater Encounte	red.		an a						
						-					
	- - -				-						
									PLA		

_ocati		entative Tracts 31268 &				Elevati	- • • •	1652			
ob No	o.: 28	89-03	Client: Richmon	d American		Date:		8/19/03			
Drill N	Aethod:	Hollow-Stem Auger	Driving Weight:	140 lbs / 30 in	Logged By:			DPO/EP			
					w	Sam		Laboratory Tests			
Depth	Lith-	Ma	aterial Description		a t e	Blows Per	C B o u r I	Moisture Content	Dry Density	Oth La	
Feet)	ology	·			r	6-inch	r Î e k	(%)	(pcf)	Te	
		(a)26 Feet: Becomes dense	to vor dansa some fir	e-grained gravel		6					
		some sand inclus	sions.	e-gramed graver,		9					
						12					
•											
30 —		Silty Sand with Clay (SM): dense; fine-grained sand; so	Yellowish-brown; moi ome caliche.	st; dense to very	· -   .	8 17					
				_	4	22					
		Total Depth = 31.5 Feet No Groundwater Encounter	red.								
					].						
										-	
	<u></u>					l	l		PLA	і ГБ /	



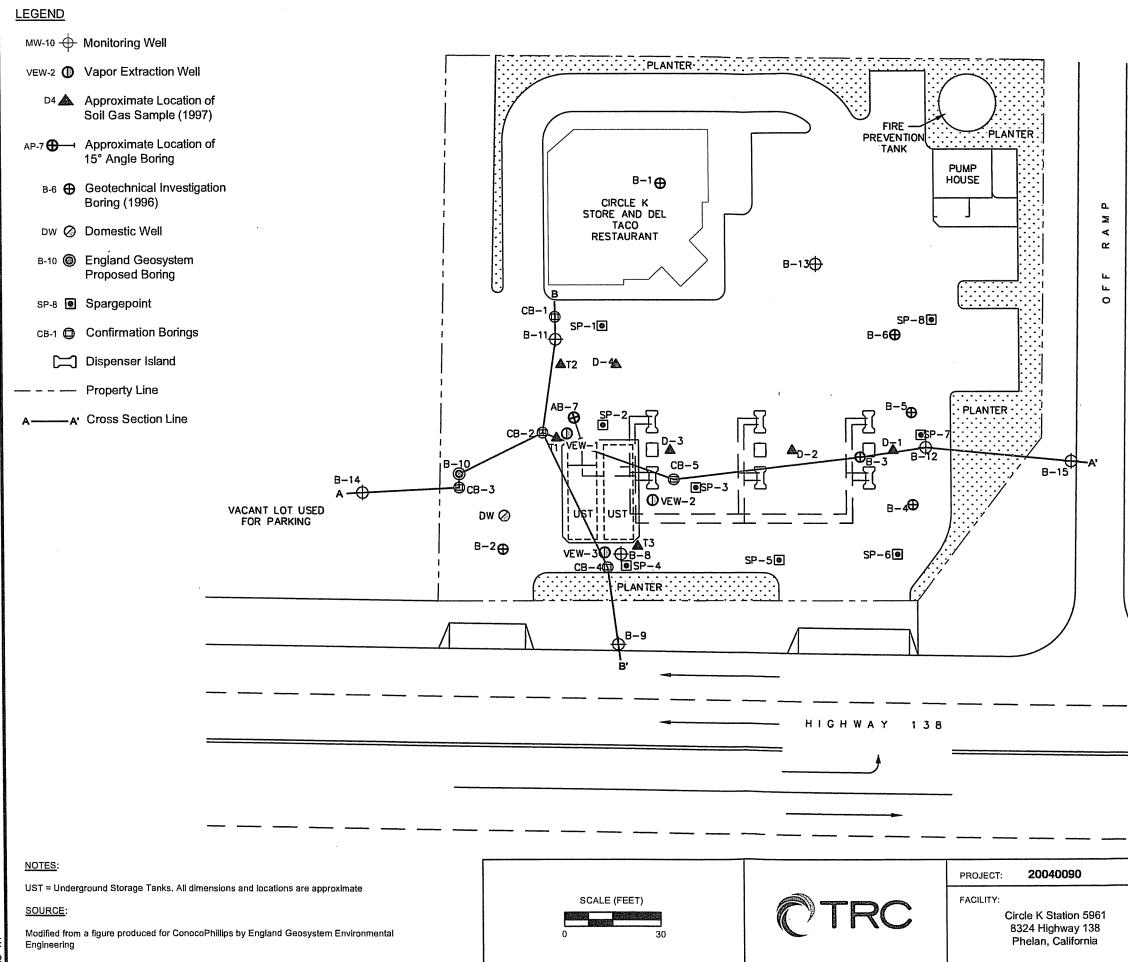
# **APPENDIX C2**

**SWRCB GeoTracker Database** 



## **APPENDIX C2.1**

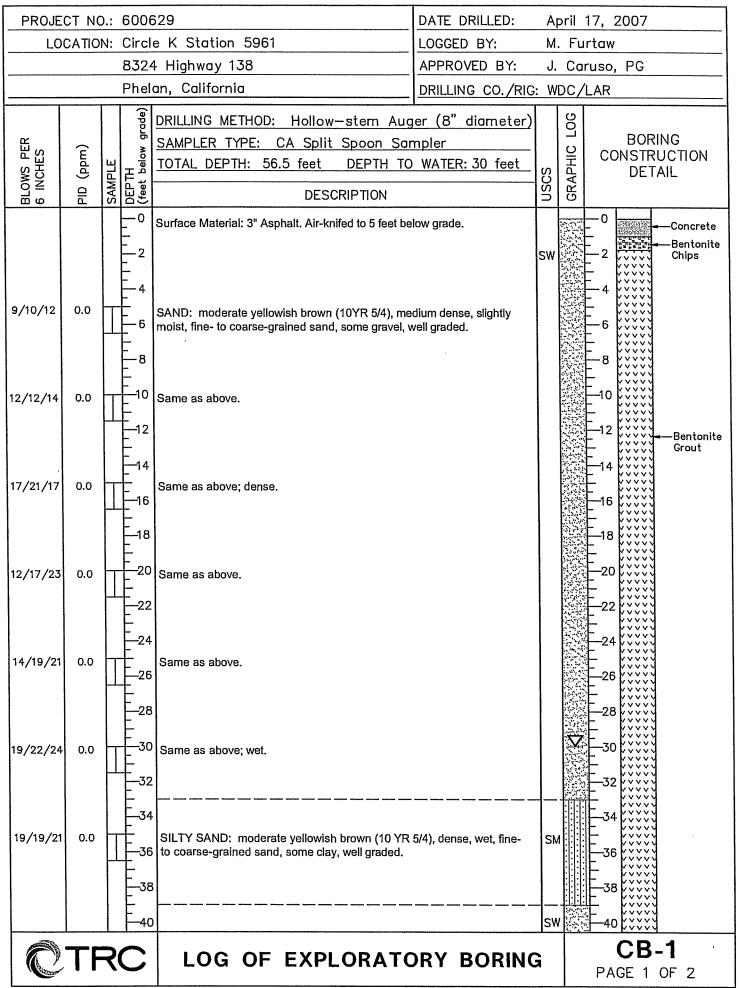
**Circle K Station 5961** 



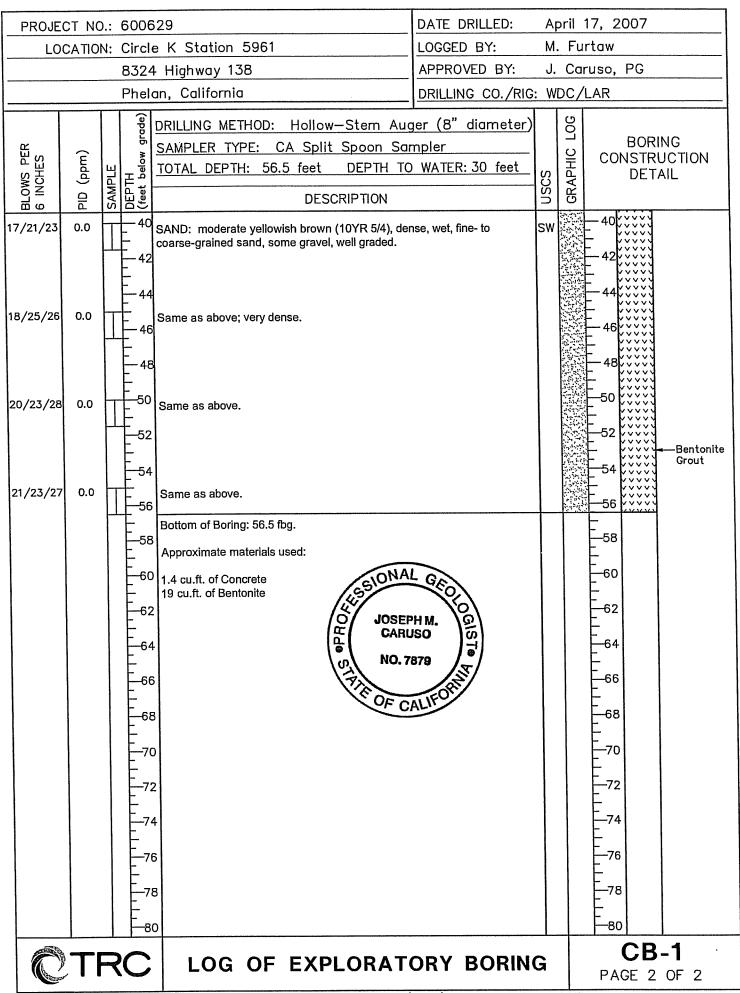


Ν

**FIGURE 2** 

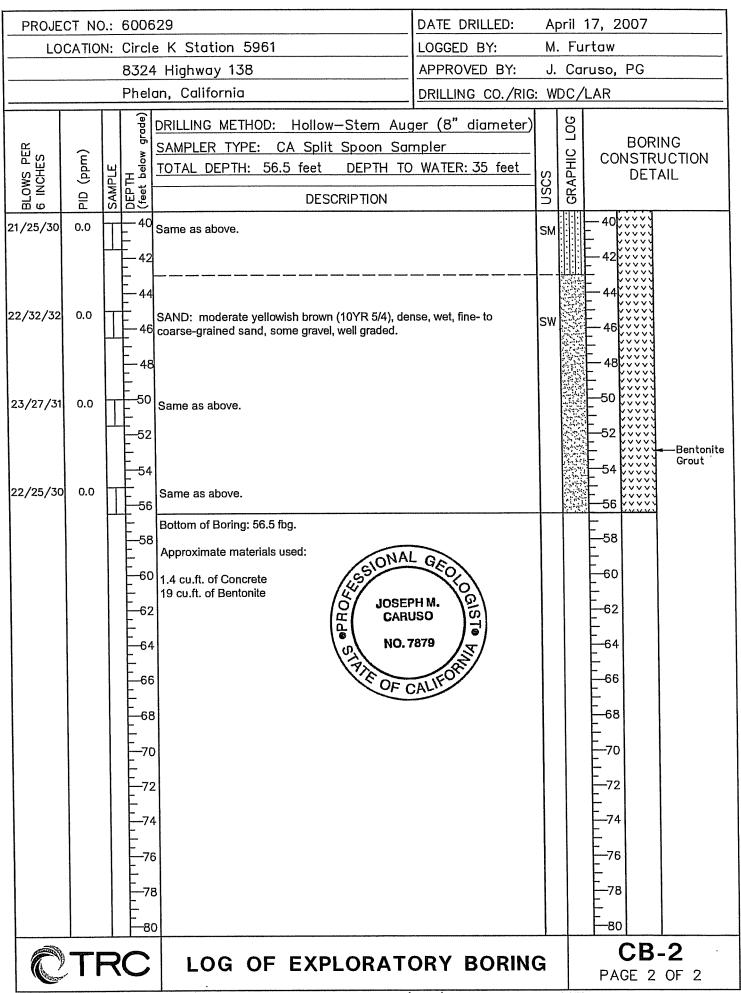


N: \PROJECTS \76PROD \5961 \GRAPHICS \5961\_BLOG\_CB-1 through CB-5.dwg; CB-1 (1 of 2), May OB, 2007-1:59pm kdrilling

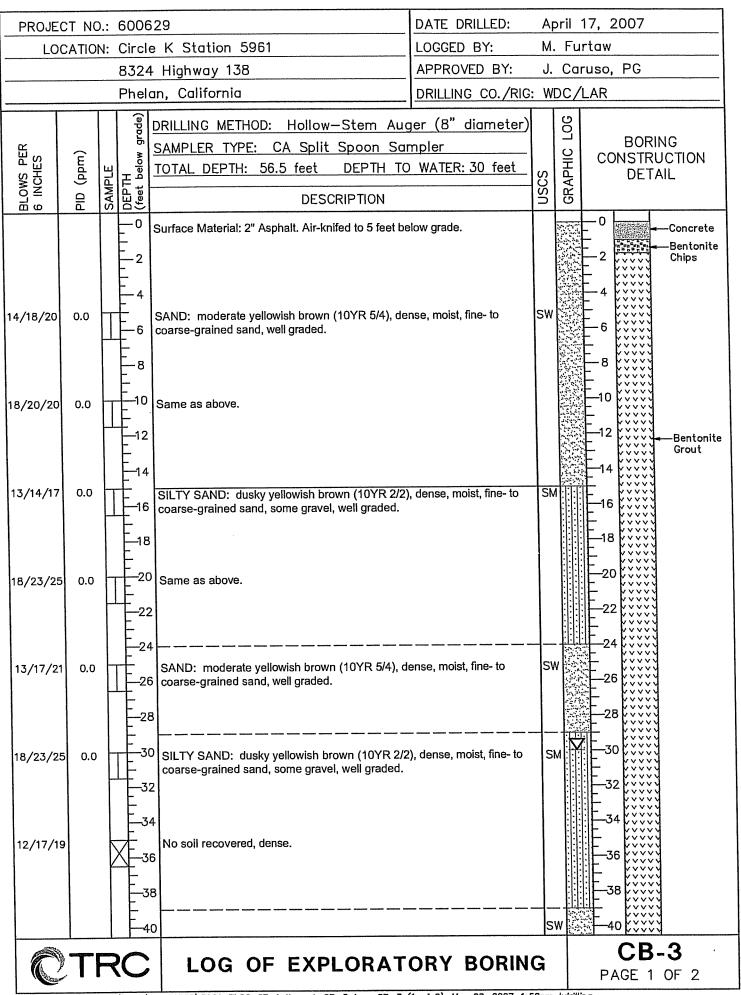


N: \PROJECTS \76PROD \5961 \GRAPHICS \5961\_BLOG\_CB-1 through CB-5.dwg; CB-1 (2 of 2), May OB, 2007-1:59pm kdrilling

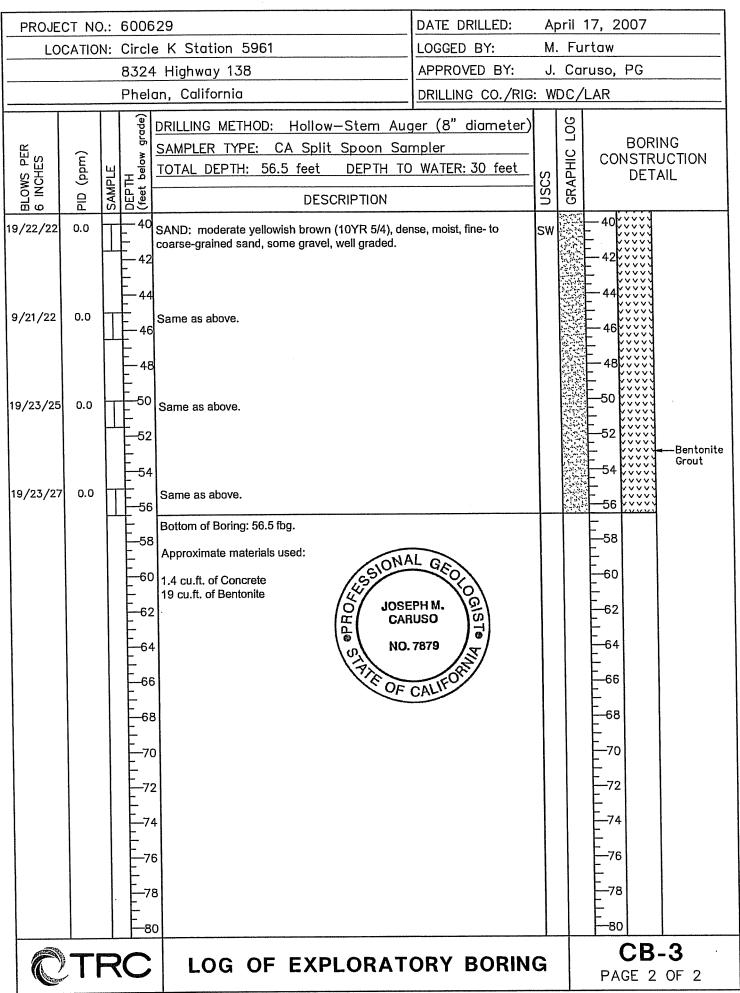
PROJE	CT NC	).: 600	629	DATE DRILLED:	Ap	oril 17, 2007							
LO	CATIO	N: Circ	le K Station 5961	LOGGED BY:	М.	Furtaw							
		832	4 Highway 138	APPROVED BY:	J.	Caruso, PG							
		Phe	Ian, California	DRILLING CO./RIG	: W[	C/LAR							
BLOWS PER 6 INCHES	PID (ppm)	SAMPLE DEPTH (feet below arade)	DRILLING METHOD: Hollow-Stem Aug SAMPLER TYPE: CA Split Spoon Sa TOTAL DEPTH: 56.5 feet DEPTH TO DESCRIPTION										
5/7/10 18/14/18 20/23/24	0.0		Surface Material: 3" Asphalt. Air-knifed to 5 feet be SAND: moderate yellowish brown (10YR 5/4), me fine- to coarse-grained sand, some gravel, well gra Same as above; dense.	edium dense, moist, aded.	SW								
20/25/28			<ul> <li>decomposed black wood material.</li> <li>Same as above; moderate yellowish brown (10YF</li> </ul>			- 16 - 16 - 18 - 18 - 20 - 20 - 22 - 22 - 24 - 24							
18/23/26 19/27/28			<ul> <li>Same as above.</li> <li>Sill TY SAND: moderate yellowish brown (10YR to coarse-grained sand, some gravel, well graded 2</li> </ul>		 SM								
22/23/2	7 0.0		4 Same as above; wet. 8 0										
C		RC	LOG OF EXPLORAT	ORY BORING	à	CB-2 PAGE 1 OF 2							



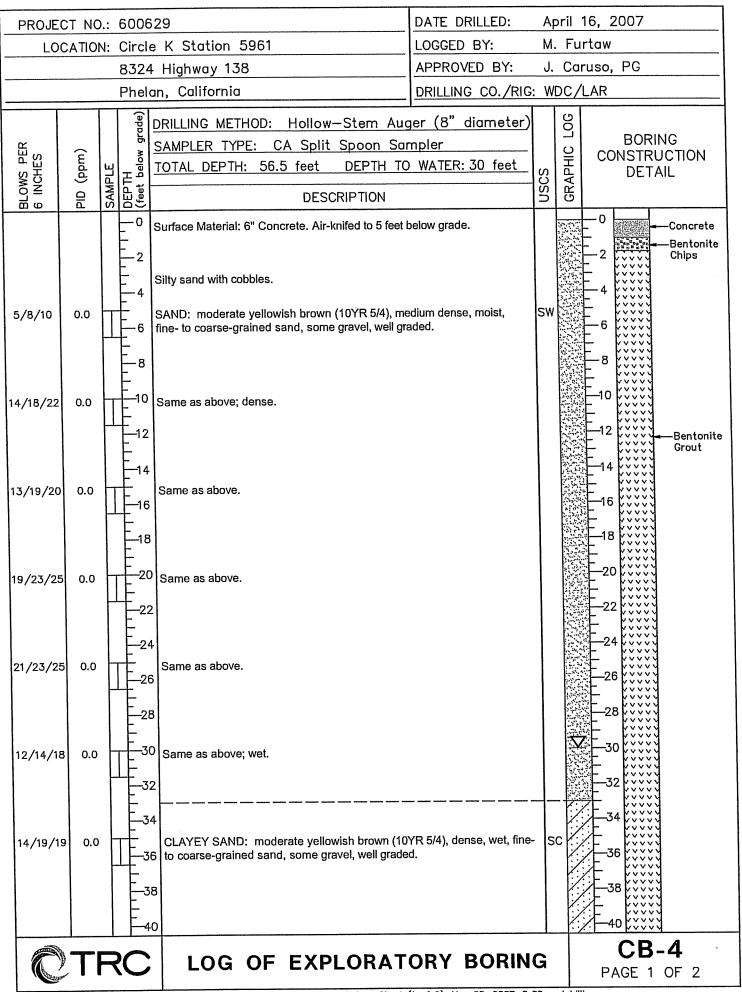
N:\PROJECTS\76PROD\5961\GRAPHICS\5961\_BLOG\_CB-1 through CB-5.dwg; CB-2 (2 of 2), May OB, 2007-1:59pm kdrilling



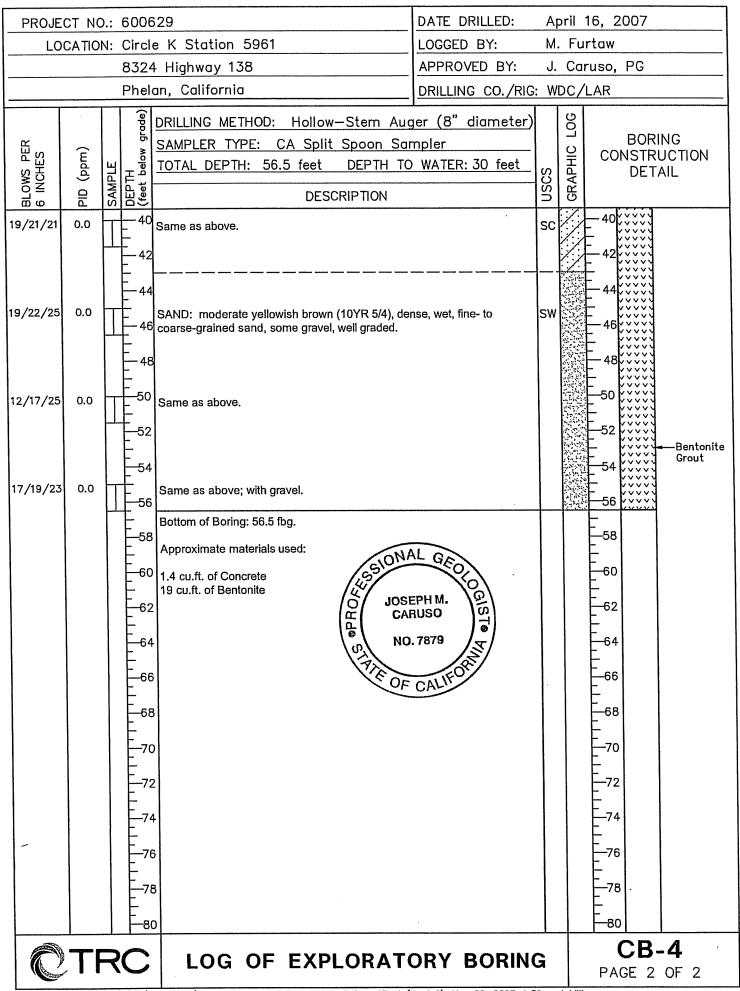
N:\PROJECTS\76PROD\5961\GRAPHICS\5961\_BLOG\_CB-1 through CB-5.dwg; CB-3 (1 of 2), May OB, 2007-1:59pm kdrilling



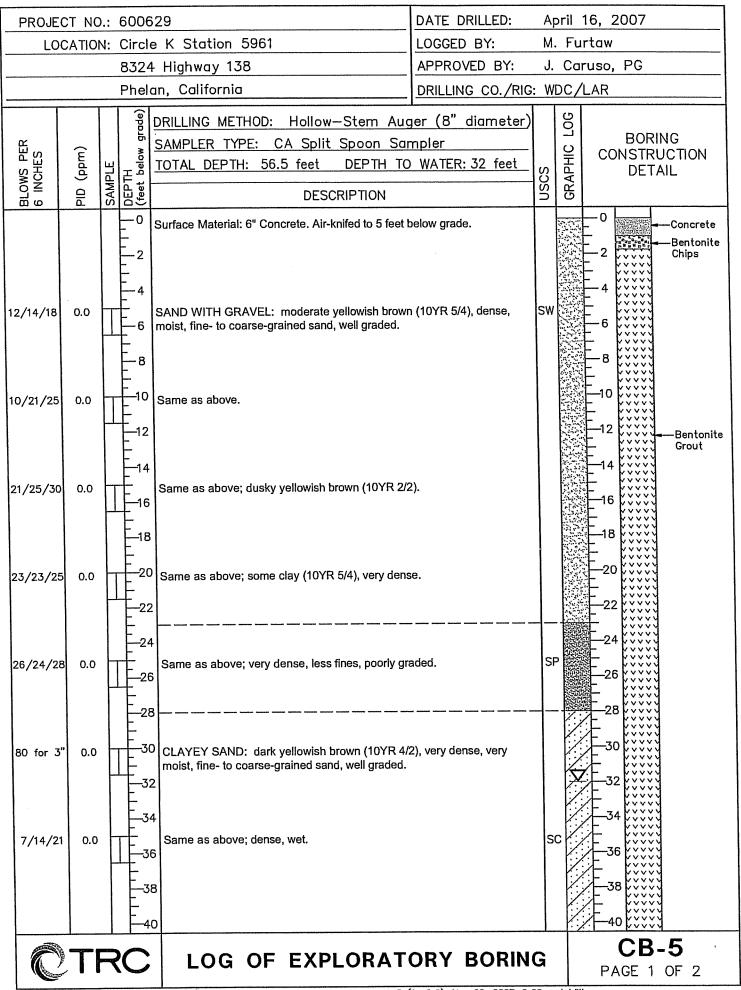
N:\PROJECTS\76PROD\5961\GRAPHICS\5961\_BLOG\_CB-1 through CB-5.dwg; CB-3 (2 of 2), May OB, 2007-1:59pm kdrilling



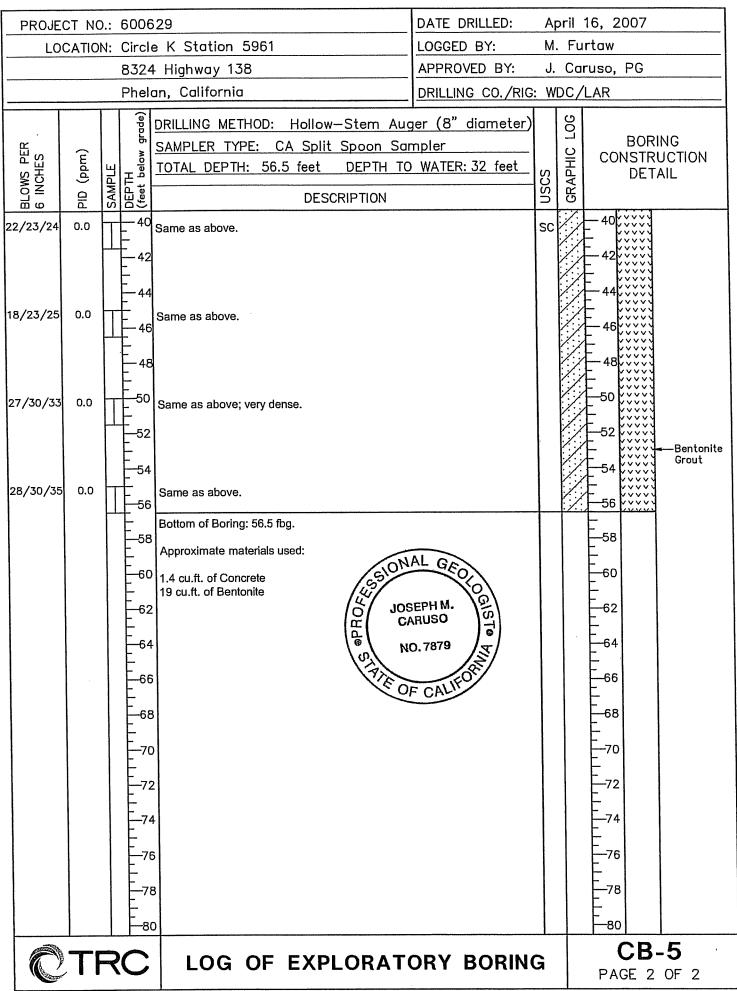
N: \PROJECTS \76PROD \5961 \GRAPHICS \5961\_BLOG\_CB-1 through CB-5.dwg; CB-4 (1 of 2), May OB, 2007-2:22pm kdrilling



N: \PROJECTS \76PROD \5961 \GRAPHICS \5961\_BLOG\_CB-1 through CB-5.dwg; CB-4 (2 of 2), May OB, 2007-1:59pm kdrilling



N: \PROJECTS \76PROD \5961 \GRAPHICS \5961\_BLOG\_CB-1 through CB-5.dwg; CB-5 (1 of 2), May OB, 2007-2:22pm kdrilling

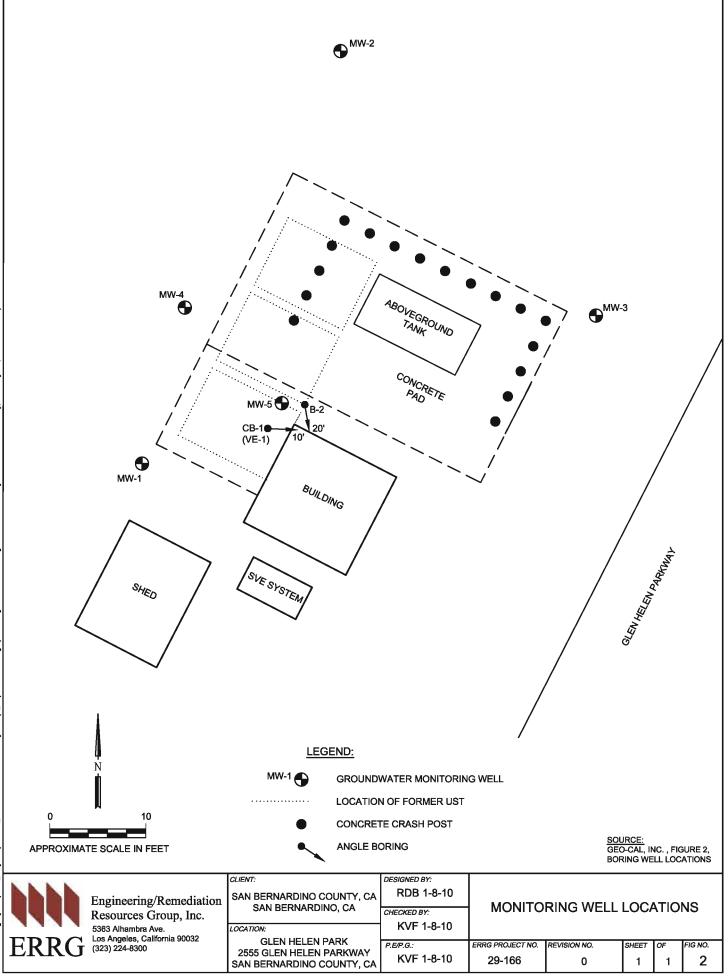


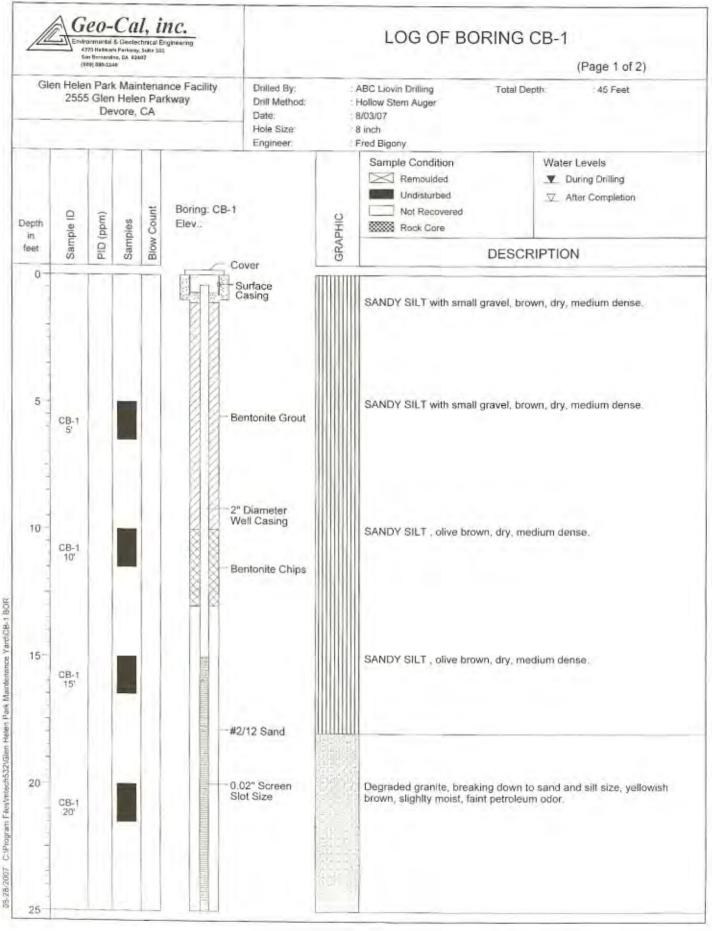
N:\PROJECTS\76PROD\5961\GRAPHICS\5961\_BLOG\_CB-1 through CB-5.dwg; CB-5 (2 of 2), May OB, 2007-1:59pm kdrilling



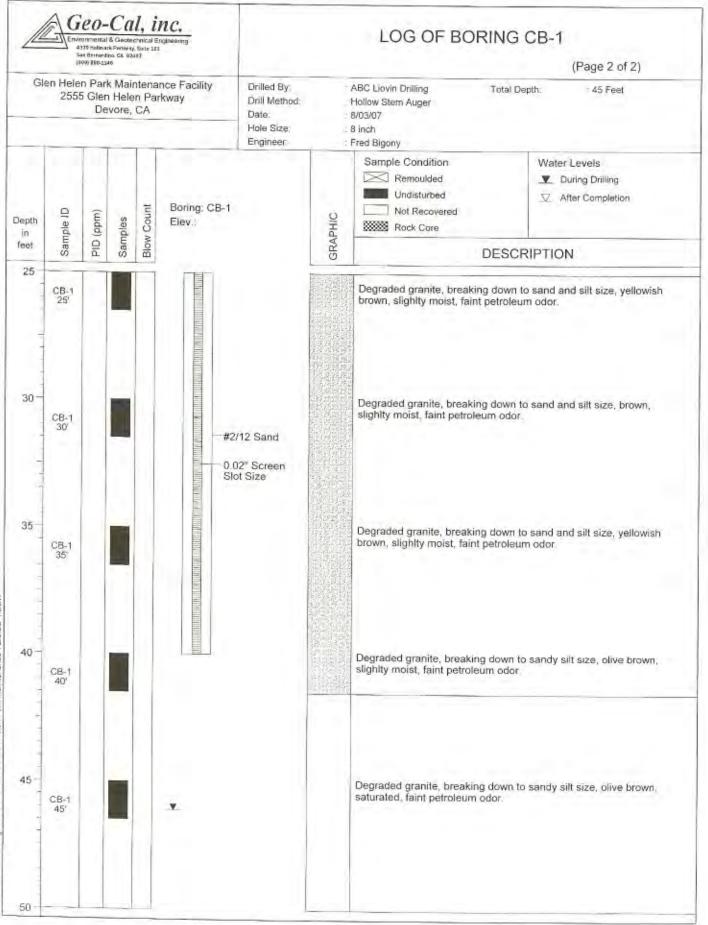
## **APPENDIX C2.2**

**Glen Helen Regional Park** 

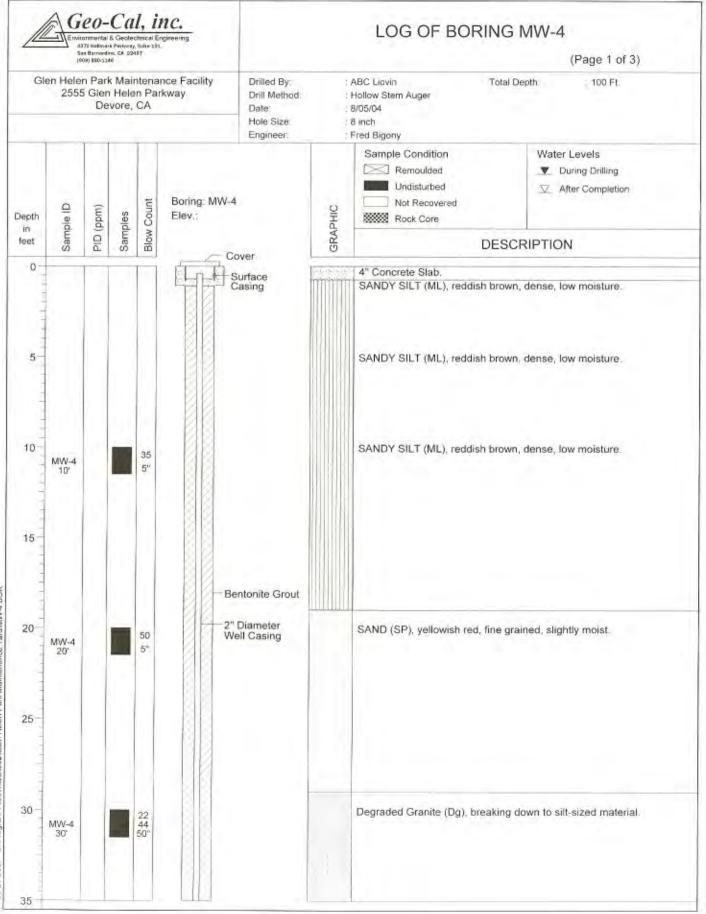




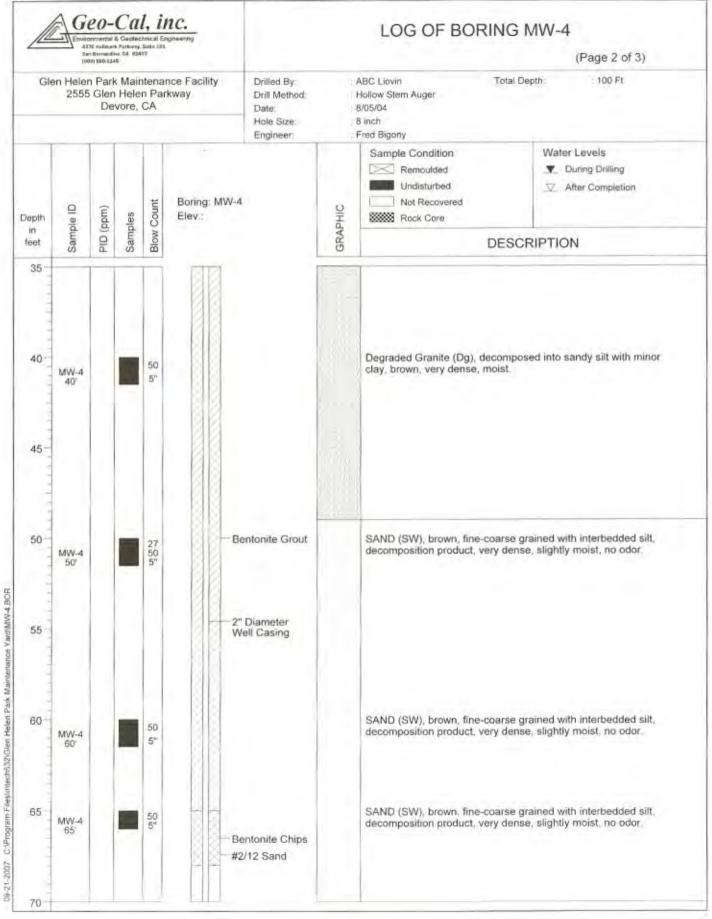
Helen Park Mantenance Yard/CE-1 BOR C:/Program Files/mtech532/Glen



08-28-2007 C. Program Files/mtech5324Gien Halen Park Maintenance YardiCB-1.BOR



09-21-2007 CMProgram Filestmech532/Glen Helen Park Maintenance YardMMM-4 BOR



#### Project: Glen Helen SVE System

Project Location: San Francisco, CA

Project Number: 29-166

Log of Boring MW-5

Sheet 1 of 2



Date (s)	Logged	Checked
Drilled 11/12/2009	By K. Foley	By J. Steller
Drilling	Drill Bit	Total Depth
Method Air Rotary Casing Hammer	Size / Type 9	Drilled (feet) 95.0
Drill Rig	Drilled	Hammer Weight /
Type Shram 660	By Test America	Drop (lb/in)
Groundwater	Date	Approx. Surface
Depth (feet) 80.0	Measured 11-12-2009	Elevation (feet)
Location Glen Helen Regional Park: 2555 Glen Heler	n Parkway, Devore, CA	Borehole Backfill Monitoring Well

feet.	Sample Name	Sample Type	Sample	Corrected Blows/Foot	Lithology	Soil Classification			Dry Unit Weight, pcf	Lab Tests / Remarks
_						ML	Hand Augered to 5 feet	Water Content %		Sample from cyclone
-							<ul> <li>Sandy SILT; dark yellowish brown; medium stiff; dry; trace</li> <li>gravel; fine-medium grained sand</li> <li>–</li> </ul>			Sample from cyclone
10—	MW-5-A	SS	$\boxtimes$	36						11-11.5 feet poor recovery
-							 			Sample from cyclone
20-	MW-5-B	SS	$\boxtimes$	>50			Sandy SILT; dark yellowish brown; dry; very stiff; medium-coarse grained sand			21-21.5 feet poor recovery
- - 30-						SM				Sample from cyclone
-	MW-5-C	SS	$\times$	>50			fine gravel to 3/8"  			30-31.5 feet no recovery
-						ML	_ Sandy SILT; yellowish brown; moist; stiff; fine-grained sand _			Sample from cyclone
40	MW-5-D	SS	$\times$	>50			<ul> <li>SILT; olive brown; moist; with gravel composed of gray and white decomposed granite</li> <li></li></ul>			40-40.5 feet no reocvery
-						SM	<ul> <li>Silty SAND; dark yellowish brown; moist; dense; fine to</li> <li>medium grained</li> </ul>			Sample from cyclone
50	MW-5-E	SS	$\times$	>50		GP	GRAVEL; white and gray with olive brown; silt and sand; very dense 			50-503.5 feet no recovery
-						SM	<ul> <li>Silty SAND with gravel; dark olive brown; moist; very dense; gravel is white and gray decomposed granite</li> <li>–</li> </ul>			Sample from cyclone

GINT.GPJ 1/22/10

-ENGINEERING / REMEDIATION RESOURCES GROUP, INC<del>.</del>

#### Project: Glen Helen SVE System

Project Location: San Francisco, CA

Project Number: 29-166

## Log of Boring MW-5

Sheet 2 of 2



Depth, feet	Sample Name	Sample Type	Sample	Corrected Blows/Foot	Lithology	Soil Classification	MATERIAL DESCRIPTION	Water Content %	Dry Unit Weight, pcf	Lab Tests / Remarks
	MW-5-F	SS	$\times$	>50		GP	GRAVEL; white and gray decomposed granite with olive brown; silt and sand; moist; very dense	-		
-						SM	- Silty SAND; dark olive brown; moist; very dense	-		Sample from cyclone
- 70— -	MW-5-G	SS	$\times$	>50			- No recovery -	-		No recovery
-							- · · ·	-		Sample from cyclone
80—	MW-5-H	SS	$\times$	>50			Silty SAND with gravel; dark olive brown; wet; very dense; gravel composed of pink, white, and gray decopmposed granite	-		80-80.5; 81-81.5 feet no recovery
-						GP	GRAVEL; pink-white and gray decomposed granite; moist; with olive-brown silt; fine grained gravel; very dense	-		Sample from cyclone
90	MW-5-I	SS	$\times$	>50		SM	Silty SAND with gravel; olive brown; wet; very dense; fine-medium grained; gravel composed of pink, white, and gray decomposed granite	-		90-90.5 feet no recovery
-					<u>64646</u>		Bottom of boring at 95.0 feet	-		
100								-		
-								-		
110 <del>-</del> -								-		
-								-		
120— -								-		
-								-		
- 130—								-		
130—				—EI	NGIN	IEERII	NG / REMEDIATION RESOURCES	GRO	DUP,	Figure 1



# **APPENDIX C2.3**

Mobil #18 Sterling Ave GeoTracker Case I.D. T0607100246

		ENVIRC	ONMENT	AL			В	ORING	LOG	
6	<b>TILE</b>		ിന		Drill I	Rig:	hang auger	Date Drilled:	3-3-92	Logged By:
		2750 Long Bea	Signal F ch, CA 9		Borin	g Dia:	<u>5</u>	Boring Numbe	r: B1	T. Brown
San Type	nple Blow Count	Depth Feet	Well Const.	Casin n (in.) 1020	Elev. Feet	Soil Type		Description and	d Remarks	Υ
		- -	////		1		CONCRETE			
		5 -				:*::: SM	SILT, brown, ODOR, OVA		ght plasticity, lo	ose, trace clay, NO
		- 10 -					damp, no pla	ne to fine grained sticity, loose, trad	ce silt, NO ODO	
							Total depth a	pprox. 10 feet bg	\$.	
				2						
Compl Borir			ntonite	chip	s, 6" c	oncrete ca	ıp.	25699 E	tation 18 HVF Baseline Road Id, California	
革			Ţ	¥				Project No	<b>.</b> 29506.00	page 1 of 1

ENVIRONMENT			ВС	RING LOG	••
ดากับสามา	Drill	ll Rig:	T	Date Drilled: 3-2-92	Logged By:
2750 Signal P Long Beach, CA 9	kwy. Bori	ring Dia:	1MCO 2400 6 3/4"	Boring Number: B2	T. Brown
SampleDepthWellTypeBlowFeetConst.CountCountConst.Const.	C a i Elev n g (in.) 1020			Description and Remarks	1. BIOWN
10-15-15       5         6-8-9       10			slightly damp, s 0 ppm SAND, light bro	ome sand, very fine grained, g slight plasticity, loose, trace cl own, fine to medium grained, g no plasticity, loose, trace silt, f feet bgs.	ay, NO ODOR, OVA
Completion Notes: Boring filled with bentonite	chips, 6" a	asphalt patch	n.	SITE: Mobil Station 18 HVF 25699 Baseline Road Highland, California	
¥ ¥				Project No. 29506.00	page 1 of 1

ENV	RONMENTAL			В	ORING LOC			
LILEU	ngu	Drill F	Rig: S	SIMCO 2400	Date Drilled: 3-	e Drilled: 3-2-92		
	750 Signal Pkw Beach, CA 908	06 Borin	ig Dia:	63/4"	Boring Number:	B3	T. Brown	
Sample Dep Type Blow Fee Count	t Const.	C a s i Elev. n g Feet in.) 020	Soil Type		Description and Rema	arks		
10-10-10 - 5 10-18-18 - 10			ML SM	slightly damp 0 ppm SAND, light b	some sand, very fine gra , slight plasticity, loose, prown, fine to medium gr	trace cla rained, gr	y, NO ODOR, OVA	
10-13-38 - 15 20-20-30 - 20			SW	SAND, light b grained, grain feldspar and	o, loose, trace silt, NO O prown to yellowish orang ns subrounded, slightly o quartz present, NO ODO pove, OVA 0 ppm	je, mediu damp, no	im to coarse plasticity, loose,	
				Total depth 2	-			
Completion Notes Boring filled with		hips, 6" a	sphalt pato	ch.	SITE: Mobil Station 25699 Baseline Highland, Cali	e Road		
¥	¥			- -	Project No. 29	506.00	page 1 of 1	

		NMENTA	L				В	ORING LO	RING LOG			
	M	ີເມ	ſ	Drill I	Rig:		SIMCO 2400	Date Drilled:	3-3-92	Logged By:		
	2750 Long Bead	Signal Pk ch, CA 90	806	Borin	g Dia		6 3/4"	Boring Number:	B4	T. Brown		
Sample Type Blow Count	Depth Feet	Well Const.	C a s i g (in.) 10 20	Elev. Soil Feet Type E				Description and R	escription and Remarks			
20-20-20	5					ML		some sandy, very fi , slight plasticity, loc				
20-20-20	- 10 -							rown, fine grained, g sticity, loose, trace s				
Completion N Boring filled		ntonite c	chips	, 6" CC	oncre	te pa	tch.	SITE: Mobil Statio 25699 Base Highland, (	eline Road			
室		Ŧ					2	Project No.	29506.00	page 1 of 1		

$\bigcirc$	ENVIRO	NMENTA				В	OR	RING LOG			
<u> </u>	MQ	മ		Drill F	Rig:	SIMCO 2400	Date	Drilled:	3-3-92	Logged By:	
	2750 Long Bead	Signal Pk ch, CA 90		Borin	g Dia:	6 3/4"	Bori	ng Number:	B5-VEW1	T. Brown	
Sample Type Blow Count	Depth Feet	Well         C           S         I           Const.         g           (in.)         10 20		Elev. Feet	Soil Type		Description and Remarks				
7-7-5 7-5-5					SM	slightly damp SAND, light t slightly damp	o, sligh prown,	t plasticity, loc	n grained, gr	ains subrounded, PR, OVA 0 ppm ains subrounded, DQR, OVA 500	
10-15-20	- 15 - - 15 - 					SW	SAND, light t grained, grain	ns sub	to yellowish or rounded, sligh present, ODC	itly damp, no	plasticity, loose,
12-19-17	- 20 -					SAND, as ab	ove, O	VA 1000 ppm			
22-20-25	- 25 -					SAND, as ab	ove, O	VA 500 ppm			
22-25-30	- 30					SAND, as ab	ove, O	VA 200 ppm			
22-20-30	- 35 -					SAND, as ab	AND, as above, OVA 200 ppm				
20-20-30	- 40					SAND, as ab	ove, O	VA 200 ppm			
24-36-28	- 45			4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4		SAND, as abo	ove, da	amp, OVA 30 p			
Completion N	lotes:				I	I		SITE:			
0.02" PVC c	asing, th y sand 4	en 6' ol 5.5-5' t	f 2" b	olank F	VC casin	Set 40' of 2" slotte g. Backfilled with ncrete traffic rate	n	Mobil Stati 25699 Bas Highland, (	eline Road		
Ā		Ţ	7					Project No.	29506.00	page 1 of 2	

	· · · _	ONMENT	AL	BORING LOG									
<b>L</b> YIE	W	שמש		Drill I	Rig:		SIMCO 2400	Date	e Drilled:	3-3-92	Logged By:		
	2750 Long Bea	Signal f ch, CA 9	Boring Dia:			6 3/4"	Bori	ing Number:	B5-VEW1	T. Brown			
Sample Type Blow Count	Depth Feet	Weil Const.	C a s i g (in.) 1020	Elev. Feet	So Tyr		Description and Remarks						
20-20-10				+			SAND, as al	bove. w	vet, OVA 0 ppn	า			
		T-#-					1		. 51 feet bgs.	-			
									8				
								T					
Completion N	lotes:								SITE:				
0.02" PVC c	asing, th y sand 4	nen 6' o 15.5-5'	of 2" b	olank F	VC ca	asin	Set 40' of 2" slott g. Backfilled wit ncrete traffic rate	h	Mobil Stati 25699 Bas Highland, (	eline Road			
꼬		Ţ	<b>X</b>					ľ	Project No.	29506.00	page 2 of 2		

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$\bigcap$	ENVIRO		AL				В	OR	RING LOG			
Lin	W	៣	Ī	Drill I	Rig:		CME 75	Date	e Drilled:	3-3-92	Logged By:	
	2750 Long Bead	Signal P ch, CA 9	Borin	g Di	a:	8"	Bori	ng Number:	B6	T. Brown		
Sample Type Blow Count	Depth Feet	Well i Const. g 1020		Elev. Feet				Description and Remarks				
4-5-5	- 5					ML	OVA 0 ppm	orown, isticity	fine grained, g	grains subrou	ose, NO ODOR, unded, slightly R, OVA 0 ppm	
Completion N Boring filled		ntonite	chips		SITE: Mobil Stati 25699 Base Highland, (	eline Road						
꼬		1	Ļ						Project No.	29506.00	page 1 of 1	

$\square$	-	NMENT	AL				В	ORING L	RING LOG			
<b>L</b>	mi	ក្រា		Drill I	Rig:		CME 75	Date Drilled:	ate Drilled: 3-3-92 L			
	2750 Long Bead	Signal P ch, CA 90	Borin	ng Di	a:	8"	Boring Number:	B7	T. Brown			
Sample Type Blow Count	Depth Feet	Well Const.	C a s i n g (in.) 1020	Eev. Feet	1	ioil /pe		Description and	Remarks			
5-4-5 4-5-5	- 5					ML	slightly dam SAND, light i slightly dam ppm	some sand, very fil o, slight plasticity, ic brown, fine to medit o, no plasticity, loos approx. 10 feet bgs.	um grained, gu e, trace silt, N	OR, OVA 0 ppm		
Completion N Boring filled		ntonite	chip	25699 Ba	tion 18 HVF seline Road California							
<u> </u>		Ţ			Project No.	29506.00	page 1 of 1					

13.

$\bigcirc$	ENVIRO	ONMENT	AL				В	OR	RING LOG			
SIJ C	mi	ក្រា		Drill F	Rig:		CME 75	Dat	e Drilled:	Logged By:		
	2750 Long Bea	Signal P ch, CA 9	0806	Borin	g Dia	1:	8"	Bor	ing Number:	B8	T. Brown	
Sample Type Blow Count	Depth Feet	Well Const.	Ca sing (in.) 102(	Elev. Feet	Feet Type		Description and Remarks					
						ML SM SW	slightly damp ppm SAND, light b slightly damp ppm SAND, light b grained, grain feldspar and SAND, as abo SAND, as abo	o, sligi prown o, no p prown ns sul quart ove, (	ht plasticity, loo h, fine to medium plasticity, loose, h to yellowish or brounded, sligh z present, NO C DVA 0 ppm CVA 0 ppm c. 25 feet bgs.	n grained, g n grained, g , trace silt, N ange, mediu tly damp, no	plasticity, loose,	
Completion N Boring filled		ntonite	chip	s, 6" a	sphal	t pato	ch.	SITE: Mobil Station 18 HVF 25699 Baseline Road Highland, California				
¥ ¥									Project No.	29506.00	page 1 of 1	

		ONMENT.	AL		BORING LOG									
<u> </u>	W	ിന	Ī	Drill F	Rig:	CME 75	Date Drilled: 3-3-92	Logged By:						
	2750 Signal Pkwy. Long Beach, CA 90806		Borin	g Dia:	8"	Boring Number: B9	T. Brown							
Sample Type Blow Count	Blow Feet Const. n g		Elev. Feet	Soit Type	Description and Remarks									
					ML	ASPHALT								
5-5-5	- 5 -		-		:*::: SM	slightly dam	, some sand, very fine gralned p, slight plasticity, loose, trace							
5-5-10	- 10						brown, fine to medium grained p, no plasticity, loose, trace sil							
9-9-10	- 15 -					grained, grai	brown to yellowish orange, mo ins subrounded, slightly damp quartz grains abundant, NO (	, no plasticity, loose,						
18-16-28	- 20					SAND, as ab	ove, OVA 0 ppm							
7-4-3	- 25					SAND, as ab	ove, OVA 0 ppm							
10-10-10	- 30					SAND, as ab	ove, OVA 0 ppm							
10-10-10	- 35 -					SAND, as ab	SAND, as above, OVA 0 ppm							
10-10-14	- 40 - 40					SAND, as ab	ove, OVA 0 ppm							
14-14-21	- 45 -					SAND, as above, OVA 0 ppm Total depth 45 feet bgs.								
Completion N	otes:	d		I		······································	SITE:							
Boring filled with bentonite chips, 6" asphalt patch.							Mobil Station 18 HVF 25699 Baseline Road Highland, California							
₽		Ţ	Ç				Project No. 29506.0	0 page 1 of 1						

$\bigcap$	ENVIRO	NMENT	AL.	ВО					RING LOG			
	W	ក្រា		Drill F	Rig:		CME 75	Date	Drilled:	3-2-92	Logged By:	
	2750 Long Bear	Signal P ch, CA 90		Borin	g Dia	a:	8"	Borir	ng Number:	B10	T. Brown	
Sample Type Blow Count	Depth Feet	Well Const.	C a s i n g (in.) 1020	Elev. Feet		ioil /pe		Desci	ription and R	emarks		
10-14-15	- 5					ML SM	slightly damp 0 ppm SAND, light b	o, slight brown,	plasticity, loo	n grained, gr	ains subrounded, y, NO ODOR, OV/ rains subrounded,	
9-14-20	- 15 -					SW	ppM SAND, light b grained, grain feldspar and	brown t ns subr quartz	o yellowish or ounded, sligh present, NO (	ange, mediu tly damp, no	plasticity, loose,	
20-25-30	- 25 -						SAND, as ab SAND, as ab	SAND, as above, OVA 0 ppm				
20-25-30 20-25-30	- 30						SAND, as abo SAND, as abo					
10-13-37	- 40						SAND, as abo Total depth a					
Completion N Boring filled		ntonite	chip	s, 6" a:	spha	lt pate	ch.		SITE: Mobil Stati 25699 Base Highland, (	eline Road		
査		1	Ľ						Project No.	29506.00	page 1 of 1	

		NMENT	AL.			÷	B	BORING LOG			
<u> <u> </u></u>	M	മ		Drill I	Rig:		Hand auger	Date Drilled:	3-2-92	Logged By:	
	2750 Long Bea	Signal F ch, CA 9	0806	Borin	ig Dia	a:	4"	Boring Number:	B11	T. Brown	
Sample Type Blow Count	Depth Feet	Well Const	C a i n g (iñ.) 1020	Eev. Feet		oil /pe		Description and	Remarks	ň	
	5					ML	SILT, brown, SAND, as abo	slightly damp, loos ove, OVA 0 ppm pprox. 10 feet bgs		DOR_OVA 0 ppm	
Completion N Boring filled		ntonite	chip	s, 6" c	oncre	ete pa	atch.	25699 Ba	ation 18 HVF Iseline Road , California	5. 	
¥			<b>Ç</b>					Project No.	29506.00	page 1 of 1	

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$\square$	ENVIRO	ONMENT	AL				B	BORING LOG					
	W	۲. آلگا		Drill I	Rig:		SIMCO 2400	Date Drilled:	3-2-92	Logged By:			
	2750 Long Bea	Signal F ch, CA 9	0806	Borin	ıg Dia	1:	8"	Boring Number:	B12	T. Brown			
Sample Type Blow Count	Depth Feet	Well Const.	Casing) (in.) 102(	Elev. Feet	Sa Tyj			Description and F	lemarks				
		////		1		ML	ASPHALT						
5-5-5	- 5		-			SM	slightly damp,	some sand, very fin slight plasticity, loo	e grained, gra ose, trace cla	ains subrounded, y, NO ODOR, OVA			
10-10-12	- 10					SW		SAND, light brown, fine to medium grained, grains subrounded, slightly damp, no plasticity, loose, trace silt, NO ODOR, OVA 0 ppm					
10-22-25	- 15				<u></u>		SAND, light bi grained, grain	rown to yellowish o s subrounded, sligh quartz grains abund	ntly damp, no	plasticity, loose,			
							Total depth ap	oprox. 15 feet bgs.					
			1										
Completion N Boring filled		ntonite	chips	s, 6" as	sphalt	patc	n. Mobil Station 18 HVF 25699 Baseline Road Highland, California						
호		Ţ	:					Project No.	29506.00	page 1 of 1			

	ONMENTAL				В	ORING L	OG		
	ាំកា	Drill F	Rig:		CME 75	Date Drilled:	3-3-92	Logged By:	
	0 Signal Pkwy. ach, CA 90806	Borin	g Dia	1:	8 <sup>#</sup>	Boring Number:	B13	T. Brown	
Sample Depth Type Blow Feet Count	Well C S Const. g (in.) 102	Elev. Feet	1	oil pe		Description and I	Remarks		
- 5 -				ML	ODOR, OVA ( SAND, light b damp, no plas	) ppm	ained, grains s silt, NO ODC	ose, trace clay, NO subrounded, slightly DR, OVA 0 ppm	
Completion Notes:	<u> </u>		[			SITE:			
Boring filled with b	entonite chip	s, 6" a:	sphal	t patc	h.	3 C	tion 18 HVF seline Road California		
¥	Ţ				Project No.	29506.00	page 1 of 1		

	ENVIRC	ONMENTAL			В	ORI	ORING LOG				
<u> </u>	mi	ന	Drill I	Rig:	CME 75	Date D	)rilled:	3-4-92	Logged By:		
		Signal Pkw ch, CA 908		g Dia:	10"	Boring	Number:	B14-VEW2	T. Brown		
Sample Type Blow Count	Depth Feet	Well Const.	C a s i Elev. n g Feet n.) 0.20	Soil Type		Descrij	ption and F	lemarks			
5-5-7	- 5		(A	ML	OVA 0 ppm				ose, NO ODOR,		
7-7-7					SILI, some si damp, slight				rounded, slightly 0 ppm		
7-10-10	- 15 -				<b>SAND</b> , light b slightly damp				ains subrounded, OVA 0 ppm		
10-10-10	- 20			SW	grains subrou	SAND, light brown to yellowish orange, fine to coarse grained, grains subrounded, slightly damp, no plasticity, loose, feldspar and quartz grains present, NO ODOR, OVA 0 ppm					
10-15-15	- 25				SAND, as abo	ove, <u>OD(</u>	<u>OR,</u> OVA 10	0 ppm			
10-10-20	- 30				SAND, as abo	ove, <u>QD(</u>	<u>2r</u> , ova 10	0 ppm			
10-10-24	- 35				SAND, as abo	ove, <u>QD(</u>	<u>OR,</u> OVA 10	0 ppm			
7-12-19	- 40				SAND, as abo	ove, <u>OD(</u>	<u>or</u> , ova 20	ppm			
15-23-28	- 45 - - 45 -				SAND, as abo	ove, dam	ip, ODOR, (	OVA 10 ppm			
	- 50 -	Ħ	-	SM 9							
Completion N	lotes:		<u>-</u>			s	ITE:				
0.02" PVC c	asing, they sand (	nen 8' of 4 60-6' bgs,	1" blank i	PVC casing	Is. Set 50' of 4" slotted Mobil Station 18 HVF asing. Backfilled with 25699 Baseline Road concrete traffic rated Highland, California						
<b>∑</b>		¥				P	roject No.	29506.00	page 1 of 2		

$\square$	ENVIRC	NMENT	AL	BORING LOG								
<u>Ú</u>	WÎ	៣		Drill I	Rig:	CME 75	Date	e Drilled:	3-4-92	Logged By:		
	2750 Long Bea	Signal P ch, CA 9	0806	Borin	g Dia:	10"	Bor	ing Number:	B14-VEW2	T. Brown		
Sample Type Blow Count	Depth Feet	Well Const.	C a s n (in.) 1020	Elev. Feet	Soil Type		Desc	cription and R	emarks			
8-7-8					e ML			/ fine to mediu oose, some sil		ains subrounded, 0 ppm		
4-7-18	- 55					SILT, brown, ODOR, OVA			ty, medium de	ense, some clay,		
50-50-50							SAND, light brown, fine to coarse grained, grains subrounded, slightly damp, no plasticity, quartz grains present, NO ODOR, OVA 0 ppm					
30-30-30	- 65						, no p	lasticity, dense		ns subrounded, is present, NO		
						Total depth 6			<u></u>			
									٤			
						;						
Completion N	lotes:				- <u>-</u> <sup>[</sup> ].			SITE:				
0.02" PVC c	asing, th y sand 6	nen 8' c 60-6' bç	of 4" t	ank F	VC casing	s. Set 50' of 4" slotted Mobil Station 18 HVF sing. Backfilled with oncrete traffic rated Highland, California						
뵻		Į	Ľ					Project No.	29506.00	page 2 of 2		

	ENVIRC	NMENTA					R	ORING LOG			
A g								I	3-4-92		
				Drill I	Rig:		CME 75	Date Drilled:	MW1	Logged By:	
	Long Bea	Signal Pl ch, CA 90	806	Borin	g Dia	a:	10"	Boring Numb	ber: B16	T. Brown	
Sample Type Blow Count	Depth Feet	Weil Const.	C a s i g (in.) 1020	Elev. Feet		ioil /pe		Description a	nd Remarks		
	5 5					ML	ASPHALT				
5-5-7						- 014	ODOR, OVA (		slight plasticity, I	oose, trace clay, NO	
5-7-7	- 10 -  					SM SW	SAND, light b slightly damp			grains subrounded, NO ODOR, OVA 0	
7-13-13	- 15						grained, grain	ns subrounded,		e, medium to coarse o plasticity, loose, ) pp <b>m</b>	
10-10-15	- 20						SAND, as abo	bove, OVA 0 ppm			
15-20-20	25						SAND, as abo	ove, OVA 0 ppn	n		
10-22-20	30						SAND, as abo	ove, OVA 0 ppn	n		
20-20-30	35	<b>输</b> :某					SAND, as abo	ove, OVA 0 ppn	n		
25-30-29	40						SAND, as abo	ove, damp, OV/	A 0 ppm		
20-20-25	45	1111551		N			SAND, as abo	ove, damp to w	et, OVA 0 ppm		
	- 50 -	. "IT" .			• <b>*•*•</b> •						
Completion		<u> </u>		1	I	I	I	SITE:			
casing. Ba	ckfilled w	vith #3	mon	terey s	sand	75-38	of 4" blank PVC 3' bgs, bentonite ad cover to groun	Mobil Station 18 HVF 25699 Baseline Road Ind Highland, California			
¥		1	¥					Project	No. 29506.00	page 1 of 2	

$\square$	ENVIRO	ONMENT	AL		×	B	OR	ORING LOG				
- Yu	<b>W</b>	മ		Drill F	Rig:	CME 75	Date	Drilled:	3-4-92	Logged By:		
	2750 Long Bea	Signal F ch, CA 9	0806		g Dia:	10"	Bori	ng Number:	B16 MW1	T. Brown		
Sample Type Blow Count		Well Const.	C a s i n g (in.) 102	Elev. Feet	Soil Type		Desc	ription and R	emarks			
casing. Ba	4" slotted ackfilled v	vith #3	PVC	casing terey s	sand 75-38	Total depth of 4" blank PVC 3' bgs, bentonite ed cover to grou		25699 Bas	tion 18 HVF seline Road California			
¥			Ŧ					Project No.	29506.00	page 2 of 2		

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GEOSCIENCES & ENCINEERING IRWIN	BORING	LOG	
Environmental Construction	DRILL RIG:	DATE DRILLED:	LOGGED BY:
Maintenance	MOBILE DRILL B-61	10-15-92	JEREMIAH
42111 Avenida Alvarado,	BORING DIAMETE	R: BORING NUMBER:	STOCK
Temecula, California 92590	8"	B16	
SAMPLE DEPTH WELL FEET CONST	ELEV FEET SOIL		
BLOW	TYPE	DESCRIPTION AND R	
TYPE COUNT			
		SILT: brown with very fine sand, slight plasticity, loose, no	odor
2-2-2 5		SILT: os before, stiff, no odor	8
12-10-9 10			
12-10-9 10	SM	SAND: light brown, fine to media grains subrounded to rou	nded, slightly domn 1
E N		no plasticity, medium der no odor	ise, troce silt
28-30-17 15			
	SW		
30-30-22 - 20	50	SAND: light brown, medium to c and cobbles, grains subro	unded to rounded
		cobbles granitic and meta no plasticity, medium to	morphic, slightly damp very dense, no odor
11-12-9 25		SAND: as before, no odor, OVA :	58 ppm
14-27-36 30		SAND: os before, no odor	
F N			
25-27-28-35		CAND	
		SAND: os before, no odor, OVA 4	4 ppm
9-15-27 40		SAND: os before, no odor	
32-46-50/4 45		SAND: as before, no odor, OVA 5	0.000
			=
12-23-11 50			
COMPLETION NOTES:		SITE:	
BACKFILLED WITH MEDIUM BENTO	NITE CHIPS	FORMER MOBIL STATION	#18HVF
		25699 BASELINE ROAD	#10=hvr
		HIGHLAND, CALIFORNIA	
		PROJECT NO. 29506.02	

	ENGINEERING		WIN	1		G	LC	)G		
		Enviror	nmental	DRILL	RIG:			DATE DRILLED:	L	OGGED BY:
		Constr Aainten		MOBILE	DRILL B	-61		10-15-92		
				BORIN	G DIAM	ETE	 R:	BORING NUMBER:		EREMIAH TOCK
	lvenida A la, Calife			8"				B16		
SAMPLI				ELEV				· · · · · · · · · · · · · · · · · · ·		
	-	FEET	WELL CONST.	FEET	SOIL					
TYPE	BLOW COUNT						CAND	DESCRIPTION AN	ID REN	MARKS
			$\left \right\rangle$			5W	SANL	): as befare, no odor		
			$\left  \right\rangle$							
	6-5-15	55	$\square$			ML	SILT:	brown with some very	fine sor	nd and cloy, wet
_		_	$\left  \right\rangle$					medium plasticity, med OVA 46 ppm	ium sti	ff, no odor
	25-50-55		$\sum$			SW	SAND	):light brown, medium t	o coars	se, some gravel
		 	$\left  \right\rangle$					and cobbles, grains su cobbles granitic and m		
		-	$\bigcirc$		میں اور میں اور			no plasticity, medium	to very	dense, no odor
	38-50/3	65 	$\sum$	,			SANL	): as before, no odor, (	)VA 95	ppm
			$\left  \right\rangle$							
	34-46-50/2	70	$\sum$				SANE	): as before, no odor, (	DVA 62	ppm
							\ 	l desth esse fuise statu 7		
							1010	l depth oppróximately 7	U leet	bgs.
		2								
	8									
			-							
COMPLE	ETION N	OTES:						SITE:		
BACK	FILLED WIT	H MEDIU	IM BENTC	NITE CH	PS			FORMER MOBIL STA 25699 BASELINE R	OAD	8-HVF
								HIGHLAND, CALIFOR	NIA	
								PROJECT NO.	PAGE	2 OF 2
L	·			· · · · · · · · · · · · · · · · · · ·				29506.02		

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		<u> </u>	WIN		RIN	G	LC	G	
		Enviror Constr	imental uction	DRILL	RIG:			DATE DRILLED:	LOGGED BY:
		Mainten		MOBILE	DRILL	8-61	:	10-15-92	JEREMIAH
42111	Avenida A	Alverado		BORIN	G DIAN	1E TE	R:	BORING NUMBER:	STOCK
1	ula, Calif		92590	8"				B17	-
SAMPL	E °	DEPTH FEET	WELL CONST.	ELE V FFF T	SOIL			2	
TYPE	BLOW COUNT				TYP			DESCRIPTION AN	D REMARKS
		SI 1				ML	SILT:	brown with very fine so slight plosticity, loose,	nd, slightly damp no odor
	4-4-4	5				SM	SAND	ilight brown, fine to me grains subrounded to r no plasticity, medium o	ounded, slightly domp
	6-7-7	10	n y					no odor OVA 60 ppm	
	18-22-25					SW	SANE	): light brown, medium t and cobbles, grains su	brounded to rounded
	19-21-22	20						cobbles granitic and n no plasticity, medium OVA 70 ppm	netamorphic, slightly damp to very dense, no odor
	11-12-12	25 					SAND	: os before, no odor O	VA 70 ppm
	25-37-42	30 					SAND	: as before, strong odor	OVA 500 ppm
	19-28-40	35 		x			SAND	as before, strong odor :	X a
	11-6-9	40					SAND	: as before, strong odor	OVA 1000 ppm
	23-28-50/5	45					SAND	: as before, moderate o	dor OVA 200 ppm
	5-7-12	50							
	ETION N						50'	SITE:	
BGS A FROM BACKF FINISH	TILLED WITH ND 16-0' 48-18' BO TILLED WITH ED WITH C ND SURFAC	BGS. SE GS. SET H #3 MOI CONCRETE	T 4" SLC 4" PVC C NTEREY S	TTED 0. CASING F SAND FR	02" PVC ROM 18 OM 50-	) SCF -0'1 16'E	REEN BGS. BGS.	FORMER MOBIL STAT 25699 BASELINE RO HIGHLAND, CALIFORN	AD
		* tuu +						PROJECT NO. 29506.02	PAGE 1 OF 2

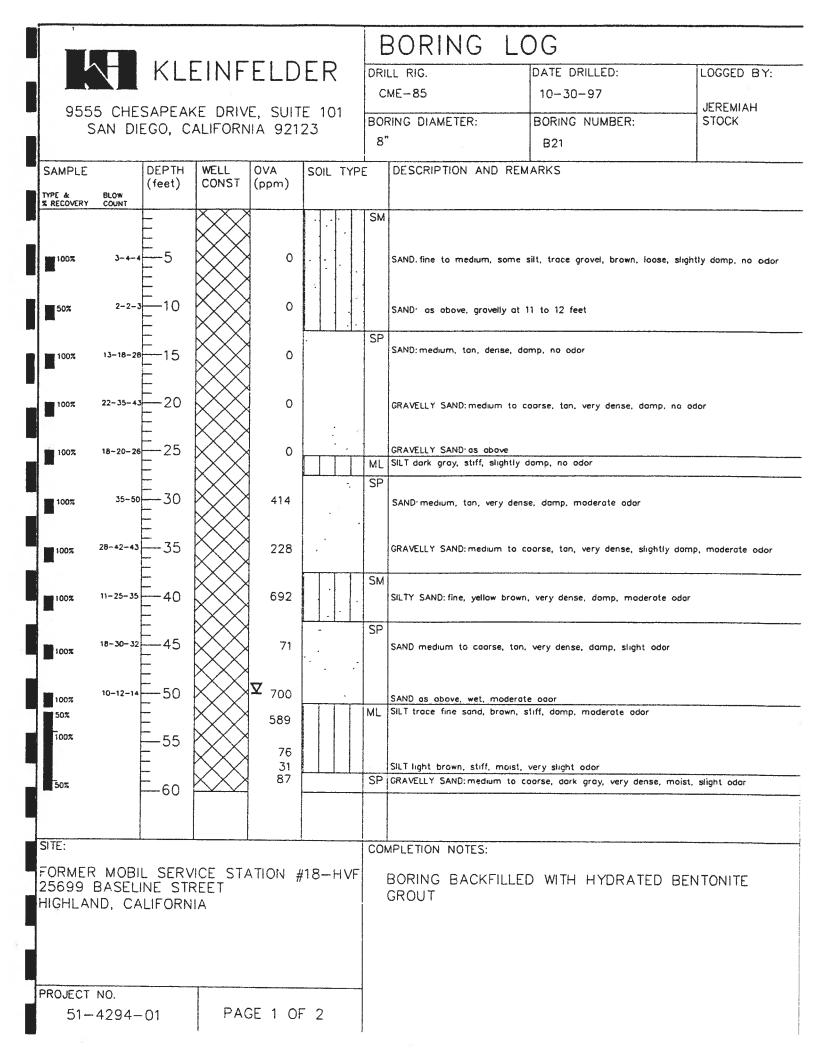
	IRV	WIN	BO	RIN	G	LC	)G		
	Enviror Constr Mainten		1	RIG: DRILL B	-61		DATE DRILLED: 10-15-92		LOGGED BY:
42111 Avenida Temecula, Cali	fornia 🤉	92590	8"	g diam	ETE	<b>ξ</b>	BORING NUMBER: B17		STOCK
SAMPLE	DEPTH	WELL CONST	ELEV FEET	SOIL					
TYPE COUNT					_	SILT:	DESCRIPTION A brown with some very medium plasticity, me OVA 200 ppm	fine s	and and clay, wet
40-50,					SW	SAND	):light brown, medium and cobbles, grains cobbles granitic and no plasticity, mediur OVA 100 ppm	subrour metor	nded to rounded norphic, slightly damp
32-50-50	/5 65					SANE	): as before, slight od	lor OV	A 70 ppm
38-50,	/2 70					SANI	D: as before, no odor	OVA	110 ppm
	75			3		1	D: as before, no odor I depth approximately	75 fee	et bgs.
									2.00
							1		
COMPLETION BACKFILLED WI BGS AND 16-0 FROM 48-18' BACKFILLED WI FINISHED WITH	TH MEDIUI D' BGS. SE BGS. SET TH #3 MC CONCRET	ET 4" SLO 4" PVC NTEREY	OTTED 0 CASING SAND FF	.02" PV FROM 18 ROM 50-	C SCF 5-0' 1 -16' E	REEN BGS. BGS.	SITE: FORMER MOBIL S 25699 BASELINE HIGHLAND, CALIFO	ROAD	#18-HVF
GROUND SURF.							PROJECT NO. 29506.02	PAC	GE 2 OF 2

	BORING	LOG	
Environmento	DRILL RIG:	DATE DRILLED:	LOGGED BY:
Construction	MOBILE DRILL B-61	10-16-92	
Maintenance			JEREMIAH ISTOCK
42111 Avenida Alvarado,	BORING DIAMETE		
Temecula, California 92590	8"	B18	
SAMPLE DEPTH WELL FEET CONS	ELEV T.FEET SOIL		
BLOW	TYPE	DESCRIPTION AND F	REMARKS
	ML	I	slightly damp
6-8-10 5	SW	SAND:light brown, medium to co and cobbles, grains subrou	oarse, some gravel unded to rounded
5-8-10 10		cobbles granitic and meta dimension up to 4", slight medium to very dense, no	morphic with longest ly damp, no plasticity
18-30-50 - 15		SAND: as before, no odor, OVA	47 ppm
45-50/2 20		SAND: as before, cobbies up to OVA 500 ppm	6", strong odor
12-13-26 25		SAND: as before, very slight odo	r, OVA 60 ppm
46-40-42 30		SAND: as before, very slight odo	r, OVA 60 ppm
16-22-38 35		SAND: as before, no odor, OVA	90 ppm
30-40-50 - 40		SAND: as before, very slight odo	r, OVA 85 ppm
19-32-50/3 45		SAND: as before, slight odor, Ov	/A 64 ppm
60-20-25 50		B	
COMPLETION NOTES:		SITE:	A
BACKFILLED WITH MEDIUM BENT BGS AND 17.5-0' BGS. SET 4" FROM 30-20' BGS. SET 4" PVC BACKFILLED WITH #3 MONTEREY FINISHED WITH CONCRETE AND T GROUND SUBFACE	SLOTTED 0.02" PVC S CASING FROM 20-0' SAND FROM 32-17.5	CREEN 25699 BASELINE ROAD BGS. HIGHLAND, CALIFORNIA BGS.	
GROUND SURFACE.		PROJECT NO. 29506.02	AGE 1 OF 2

GE		L	WIN	1	RIN	G	LC	)G			
		Enviror	nmental	DRILL	RIG:			DATE DRILLED	:	LOGG	ED BY:
		Constr	uction	MOBILE		361		10-16-92			
		Mainten	ance	DODINI						JEREMI STOCK	
42111	Avenida A	lvarado	,	BORIN	j dian	1E IE	R:	BORING NUMB	ER:		
Temeo	eula, Califo			8"				B18			
SAMP	LE	DEPTH FEET	WELL CONST.	ELEV	soil						
	BLOW				TYP			DESCRIPTIO			(5
TYPE	COUNT					ML		brown with some		·	
			$\left  \right\rangle$					medium plasticity	, medi	um stiff, no	odor
		 	$\land \land \land$								
	15-50/5	55	$\bigcirc$				SILT:	as before, no od	or, OVA	A 64 ppm	
			$\left  \right\rangle$			SW	SANI	);light brown, me	dium t		ome gravel
			$\left  \right\rangle$					ond cobbles, gro	oins su	brounded to	o rounded
	43-50/5	60	$\bigcirc$	1	8			cobbles granitic no plasticity, m			
								OVA 60 ppm			
	23-44-50	65					SAN	D: as before, no (	odor. (		n
							<u> </u>	depth opproximo			
									,	- -	
,											r T
				-							22
											× .
COMP	LETION N	OTES.						SITE:		<u>.</u>	
	FILLED WITH		BENTON	NITE CHIE	S FROM	1 65-	-32'				
BGS	AND 17.5-0	)' BGS. S	SET 4" S	LOTTED (	0.02" P	VC S	CREEN	FORMER MOB 25699 BASEL			VF
BACK	30-20' BO FILLED WITH	GS. SET   #3 MOI	4" PVC ( NTEREY S	CASING F	ROM 20	)0' 17.5'	BGS. BGS.	HIGHLAND, CA			
FINISH	ED WITH C	ONCRETE	AND TR	AFFIC RA	ATED C	OVER	TO				
GRUU	ND SURFAC	Ε.						PROJECT NO.			
								29506.02		PAGE 2 OF	2

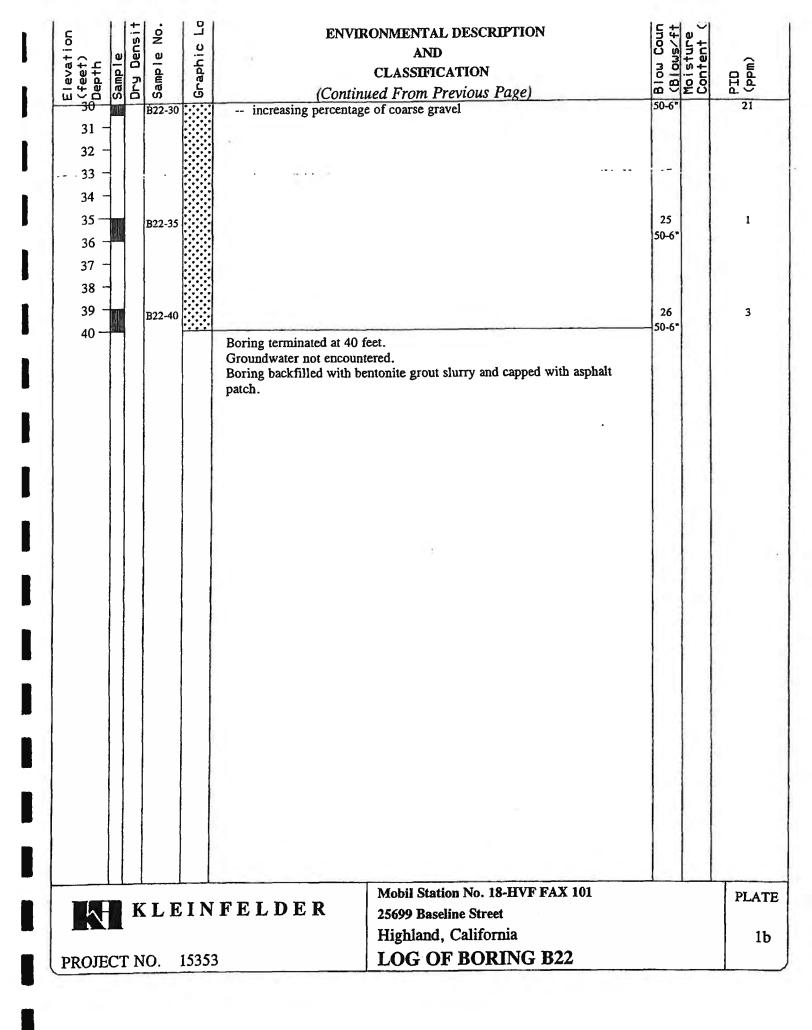
CEOSCIENCES & ENGINEERING		WIN	}	RIN	3	LO	G		
	Enviror Constr Mainten			RIG: drill b-	-61		DATE DRILLED: 0-16-92		LOGGED BY:
42111 Avenida Temecula, Calif	Alvarado		BORIN( 8"	g DIAM	ETEF		ORING NUMBER: 20		
SAMPLE BLOW TYPE COUNT	DEPTH	WELL CONST.	ELEV FEET	SOIL TYPE			DESCRIPTION AN	ND RE	MARKS
					ML		brown with very fine s slight plasticity, loose, OVA 120 ppm	sand, s	lightly damp
4-7-12 7-9-11	□ □ □ □ □ □ □ □				:		light brown, fine with no plasticity, medium OVA 78 ppm as before, no odor C	dense	e, no odor
17-30-3					SW	SAND	: light brown, medium and cobbles, grains s cobbles granitic and no plasticity, medium OVA 110 ppm	subrour metom	nded to rounded Porphic, slightly domp
13-10-1	25 		- - -			SAND:	as before, no odor (	)VA 70	ppm
18-19-25	30					SAND:	as before, no odor (	DVA 18	0 ppm
14-20-23	35					SAND:	as before, no odor (	)VA 82	ppm
11-10-34	40				0	SAND:	os before, no odor (	DVA 90	) ppm
21-20-19						SAND:	os before, no odar (	OVA 12	0 ppm
COMPLETION I BACKFILLED WIT		BENTON	NITE CHIF	2S			SITE: FORMER MOBIL ST 25699 BASELINE R HIGHLAND, CALIFOR	ROAD	#18-HVF
							PROJECT NO. 29506.02	PAG	GE 1 OF 2

	OSCIENCES	IR	WIN	BO	RIN	G	LC	)G		
		Enviro	nmental	DRILL	RIG:			DATE DRILLED:		LOGGED BY:
			ruction	MOBILE	DRILL B	-61		10-16-92		
		Mainten	ance							JEREMIAH STOCK
42111	Avenida /	Alvarado	•	1	g dian	IETE	R:	BORING NUMBER:		BTOCK
	ula, Calif		92590	8"				B20		
SAMPI	LE	DEPTH		ELEV						
	DL OW	FEET	CONST		SOIL					
TYPE	BLOW COUNT		ļ				<u> </u>	DESCRIPTION A		
						5W	SANL	):as before, no odor (	DVA 120	) ppm
	5-6-10	55	$\wedge \wedge \wedge$			L NAT	CIL T.	brown with some very	fine co	nd and clay wet
								):light brown, medium	lium sti	ff, no odor
	24-29-23	60					SANL	and cobbles, groins s cobbles granitic and i no plosticity, medium	ubround metamo	ded to rounded orphic, slightly domp
		F	$ \langle \rangle \rangle$					OVA 96 ppm		
100	25-33-40	65	$ \rightarrow \rightarrow \rightarrow$				·	D: as before, no odor		
							Total	depth opproximately 6	55 feet	bgs.
										19. v
					}	· .				
										3
										*
COMP	LETION N	L INTES:	1			<u> </u>	L	SITE:		-
1	FILLED WIT		M RENTO	NITE CHI	PS					110 UVE
	VIII		M DENTO:					FORMER MOBIL ST 25699 BASELINE HIGHLAND, CALIFO	ROAD	#10—ПVГ
								PROJECT NO. 29506.02	PAG	E 2 OF 2
L		·····	<u></u>					23300.02		



						=	E	BORING L	OG	
			KL	EINF	ELD	ER		LL RIG.	DATE DRILLED:	LOGGED BY:
	955	5 CHF	SAPFAK	KE DRIV	F SUIT	E 101		ME-85	10-30-97	JEREMIAH
				ALIFORN			80 8	RING DIAMETER.	BORING NUMBER:	STOCK
	0.41.101.00		DEPTH	1 140-1 1					B21	
	SAMPLE	BLOW	(feet)	WELL CONST.	OVA (ppm)	SOIL TYP	E,	DESCRIPTION AND RE	MARKS	
	SOZ	COUNT	<u> </u>		28		SP			
				$\bowtie$	9			SAND: medium to coorse, so	ome gravel, dark gray, very dense, m	oist. slight odor
	50%		65	$\mathbb{X}$	9			SAND: fine to coorse, some	gravel, brown, very dense, moist, no	odor
	50%		- 70	$\bigotimes$		•				
			Ē	$\bigotimes$	4 6		CI I			
	1007	30-50	E 75	$\bowtie$	0		SM	SILTY SAND fine, ton, very o	dense, slightly damp, no odor	
			E	$ \rangle\rangle\rangle$			SP			
	100%	28-50	80	$\times$	<b>Z</b> 0		SP	SAND fine to medium, trace	e silt, tan, very dense, wet, na odor	
				$\mathbb{X}$		· · ·				
	100%	26-36-50	- 85	$\bowtie$	0	A		SAND: fine to coarse, with li	enses of silt, brown, very dense, wet	, no odor
		=		$\times$			ML	SANDY SILT orange brown, v	rery stiff, damp, no odor	
	100%	32-50	90	<u>x_x_x</u>	0					
								v		
			-							
				-						
						ос.			54	
								18		
	SITE:	1100			-		CON	PLETION NOTES:		
12	25699 E	BASELI	NE STR	EET	TION #	18-HVF		BORING BACKFILLE GROUT	ED WITH HYDRATED BE	NTONITE
H	IIGHLAN	D, CA	LIFORNI	А				SILUU I		
									÷	
F	ROJECT	NO:								
	51-4	4294-	01	PAG	E 2 OF	2				
						- 2 2 S S	·· - ·	2		

Drilled By: Drilling Method: Logged By:	Cascade Drilling Hollow Stem Auger Virginia Moore	<ul><li>Date Measured:</li><li>8" Reference Elevation:</li><li>Datum:</li></ul>	4/17/02 N.A. N.A.	
Elevation (feet) Depth Sample Dry Density Sample No. Gräphic Log	ENVIR	CONMENTAL DESCRIPTION AND CLASSIFICATION	Blow Count (Blows/ft.) Moisture Content (%)	(mqq)
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	SILTY SAND (SM): brow	wn, fine grained, dense, no odor		
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	HOLE CLEARED TO 5	FEET BEFORE SAMPLING		
8 - 9 - 10 - 11 - 12 - 13 -	SAND (SW): light brown, odor	, moist, loose, medium to coarse grained, no	31	0
14 - 15 - 16 - 17 - 18 -			31 50-6*	8
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	brown, dense, fine to a	medium grained, no odor	35 50-6*	0
23 - 24 - 25 - 26 - 27 - 28 -	rock lodged in shoe of	sampler	50-6*	0
29	FELDER	Mobil Station No. 18-HVF FAX 101 25699 Baseline Street		PLATI
PROJECT NO. 15353		Highland, California LOG OF BORING B22		



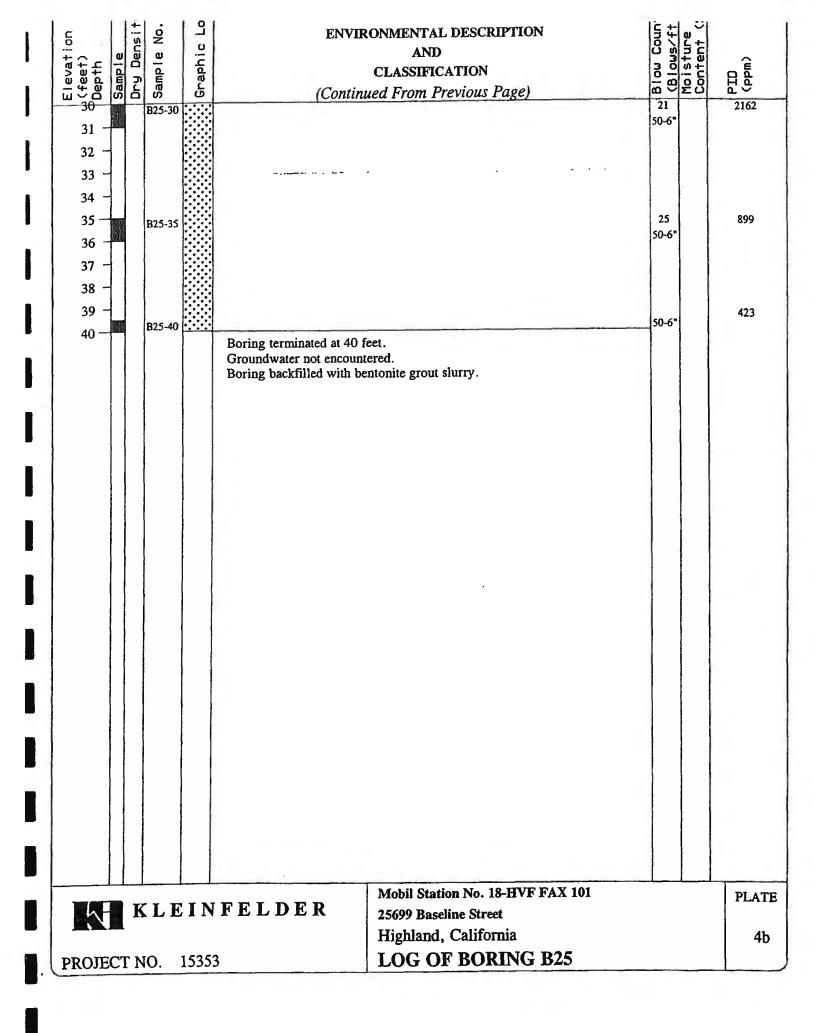
Date Dillou.	Casaada Drilling	Date Measured:	4/17/02	
Drilled By:	Cascade Drilling Hollow Stem Auger 8		4/1//02 N.A.	
Drilling Method: Logged By:	Virginia Moore	Datum:	N.A.	
				<u> </u>
Elevation (feet) Depth Sample Dry Density Sample No. Graphic Log		NMENTAL DESCRIPTION AND LASSIFICATION	Blcw Count ( <u>Blows/ft.</u> ) Moisture Content (%)	
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	SAND (SP): brown, moist, o	dense, medium grained, no odor ET BEFORE SAMPLING		•
9 - 10 - 11 - 12 - 13 - 14 - 14 - 10 - 12 - 13 - 14 - 10 - 10 - 10 - 10 - 10 - 10 - 10	SAND (SW): brown, moist,	loose, some gravel, no odor	10 50-6"	0
15 16 17 18 19 -	increasing percentage of	gravel	23 50-6*	2
20 - B23-20 21 - 22 - 23 - 23 - 23 - 23 - 23 - 23 -			50-6*	7
24 - 25 - 26 - 27 - 28 - 29 -	SAND (SP): brown, moist, lo	oose, medium grained, odor present	32	1291
30 KLEIN		obil Station No. 18-HVF FAX 101 699 Baseline Street	_111	PLATE
PROJECT NO. 15353	H	ighland, California OG OF BORING B23		2a

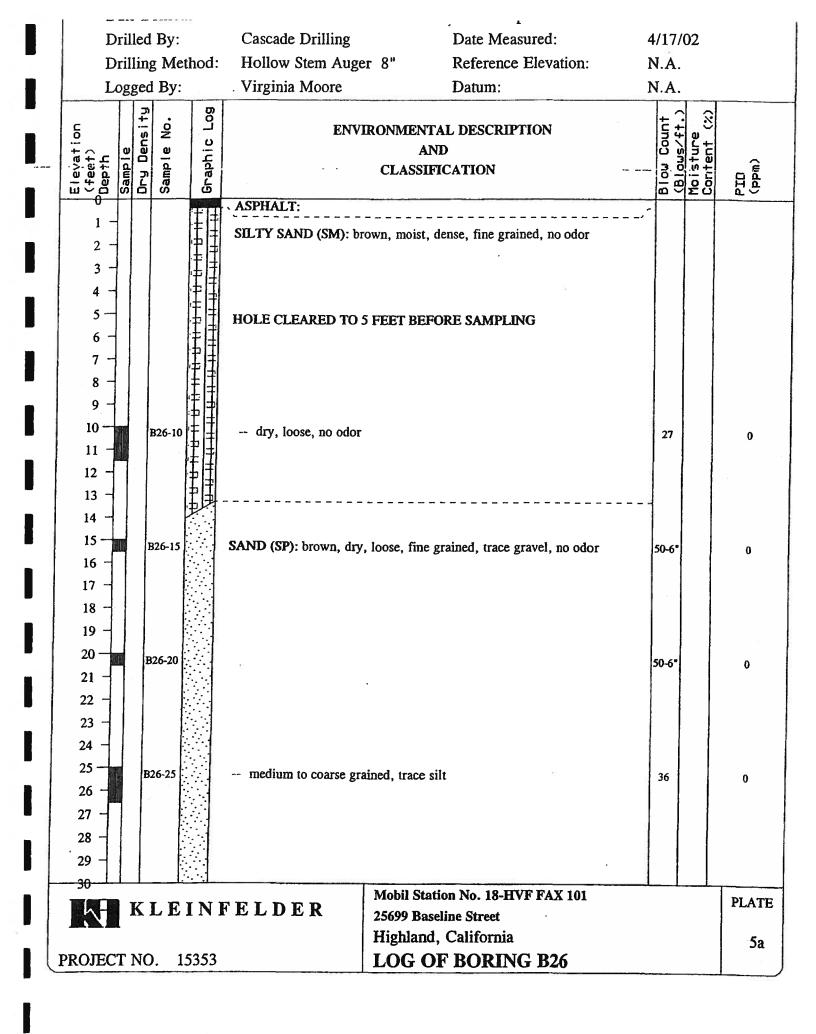
Elevation Depth Dry Densit Graphic Lo	ENVIRONMENTAL DESCRIPTION     Imposite a structure       AND     CLASSIFICATION       (Continued From Previous Page)     Imposite a structure	PID (ppm)
30 31 - 32 - 33 - 34 -	trace gravel 20 50-6"	596
35	SILTY SAND (SM): dark brown, dense, fine to medium grained, odor present	702
39 - 40 - B23-40	SAND (SP): brown, dense, fine to medium grained, odor present       50-6"         Boring terminated at 40 feet.       50-6"         Groundwater not encountered.       Boring backfilled with bentonite grout slurry.	60
	·	
	Mobil Station No. 18-HVF FAX 101	PLATE
PROJECT NO. 1535	FELDER25699 Baseline StreetHighland, CaliforniaLOG OF BORING B23	2b

Drilled By:	Cascade Drilling	Date Measured:	4/17/02	
Drilling Method:		Reference Elevation:	N.A.	
Logged By:	Virginia Moore	Datum:	N.A.	
Elevation (feet) Depth Sample Drij Density Sample No.		NTAL DESCRIPTION AND SIFICATION	Blow Count ( <u>Bjows/ft.)</u> Moësture Content (2)	PIU (mud)
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	SILTY SAND (SM): brown, mois odor HOLE CLEARED TO 5 FEET BI	t, dense, fine to medium grained, no		
$\begin{array}{c} 9 \\ -10 \\ 11 \\ 11 \\ -12 \\ 13 \\ -14 \end{array}$	SAND (SW): light brown, moist, lodor	loose, medium to coarse grained, no	. 35	0
$ \begin{array}{c} 15 \\ 16 \\ 17 \\ 18 \\ 19 \\ 19 \\ \end{array} $ B24-15		х	25 50-6*	
20 - B24-20 21 - 22 - 23 - 23 - 23 - 20 - 20 - 20 - 20			28 50-6*	0
24 – 25 – 26 – 27 – 28 – 29 –	brown, dense, fine to medium	grained, no odor	30 50-6"	0
	מתתקת	Station No. 18-HVF FAX 101		PLAT
PROJECT NO. 15353	Highla	Baseline Street nd, California OF BORING B24		3a

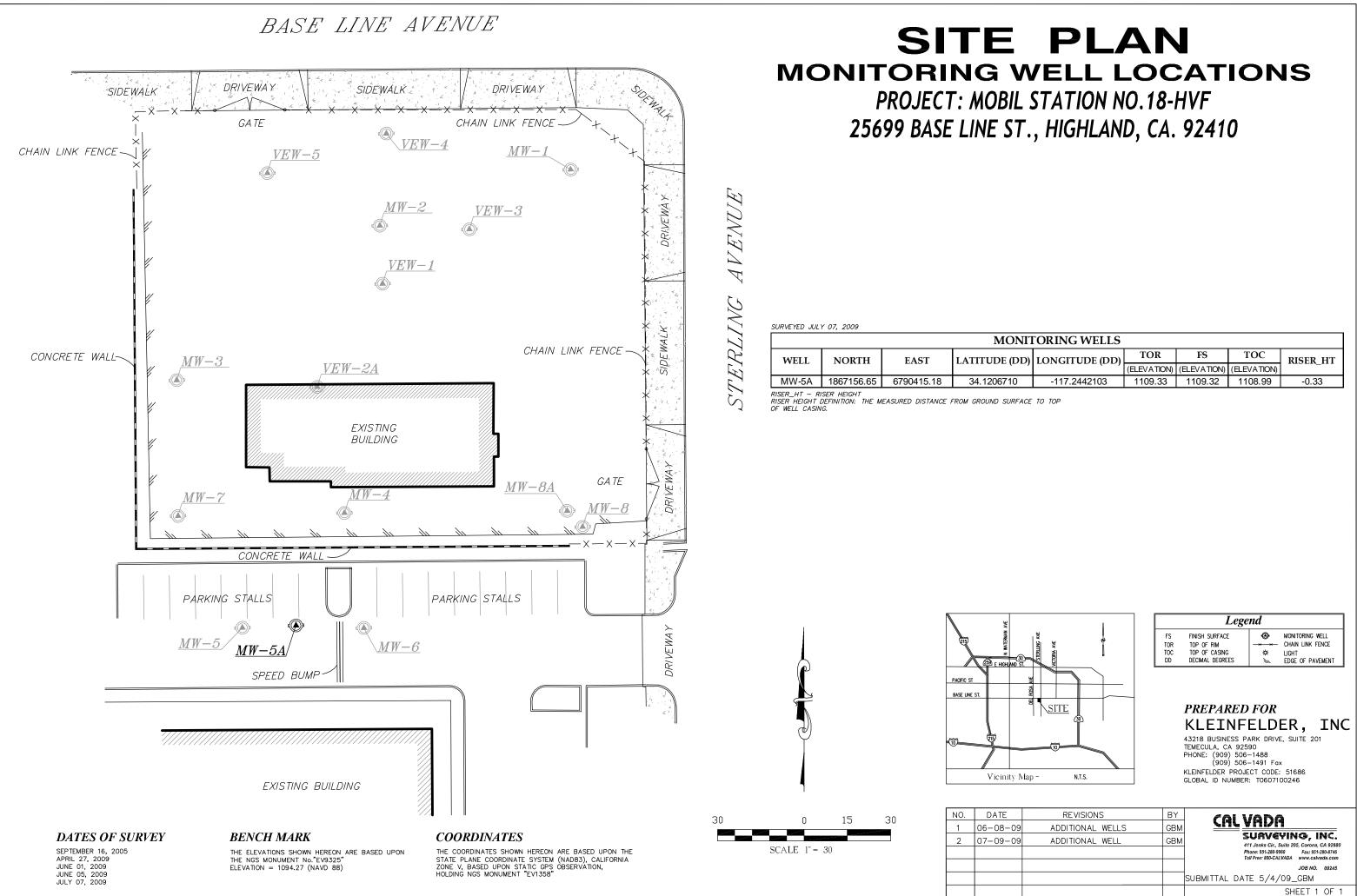
Elevation Gfeet) Depth Sample	Dry Densi Sample No. Graphic L	1	NVIRONMENTAL DESCRIPTION AND CLASSIFICATION ntinued From Previous Page)	Blow Count (Blows/ft, Moisture Content (3	PID (mqq)
31 - 32 - 33 -	B24-30	;	ense, some fine grained sand, odor present		1694
34 - 35 - 36 - 37 -	B24-35	SAND (SW): brown	, moist, dense, fine to medium grained	90	1260
38 - 39 - 40 -	B24-40	Boring terminated at	40 fact	38	1146
		Groundwater not enc	ountered. h bentonite slurry grout.		
1 1					

Drilled By:	Cascade Drilling	Date Measured:	4/17/02	
Drilling Method			N.A.	
Logged By:	Virginia Moore	Datum:	N.A.	
Elevation (feet) Depth Sample Dry Density Sample No.		RONMENTAL DESCRIPTION AND CLASSIFICATION	Blow Count ( <u>Blows/ft.)</u> Moisture Content (%)	PID (ppm)
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	grained, no odor	own, moist, medium dense, fine to coarse		
9 - 10 - 11 - 12 - 13 -	medium to coarse gr	ained, trace gravel, no odor	32	0
14 - 15 - 16 - 17 - 18 -	SAND (SW): brown, mo increasing percentage of a	ist, dense, medium to coarse grained, gravel, no odor	25 50-6*	0
19 - 20 - 21 - 22 - 23 -			25 50-6*	4
24 - 25 - 26 - 27 - 28 - 29 -	trace gravel, strong o	dor	15 50-6"	3343
-30		Mobil Station No. 18-HVF FAX 101		
KLEIN	FELDER	25699 Baseline Street		PLAT
		Highland, California		4a
ROJECT NO. 1535	3	LOG OF BORING B25		





Elevation % (feet) Depth Sample	Dry D	Sample No	Graphic L	(Contin	RONMENTAL DESCRIPTION AND CLASSIFICATION wed From Previous Page)	Blow Coun (Blows/ft Moisture	Content (; PID (ppm)
31 - 32 - 33 -		B26-30		no silt or gravel		27 50-6"	0
34 - 35 - 36 - 37 -		B26-35		SAND (SW): brown, dry percentage of gravel	y, medium to coarse grained, increasing	19 50-6°	0
38 - 39 - 40 -		B26-40		increasing percentag Boring terminated at 40 f Groundwater not encount	feet.	21 50-6"	0
					tered. entonite grout slurry and capped with asphalt		
	K			FELDER	Mobil Station No. 18-HVF FAX 101 25699 Baseline Street		PLATE
PROJECT	NC	D. 15	5353		Highland, California LOG OF BORING B26		5Ъ

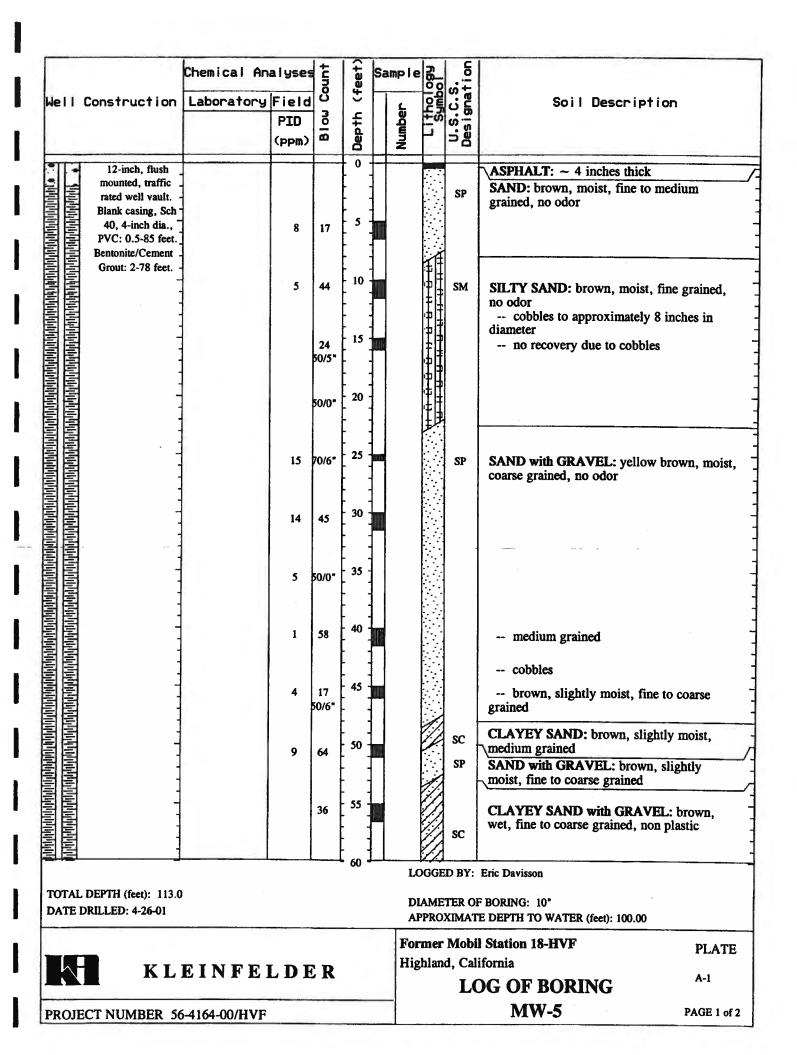


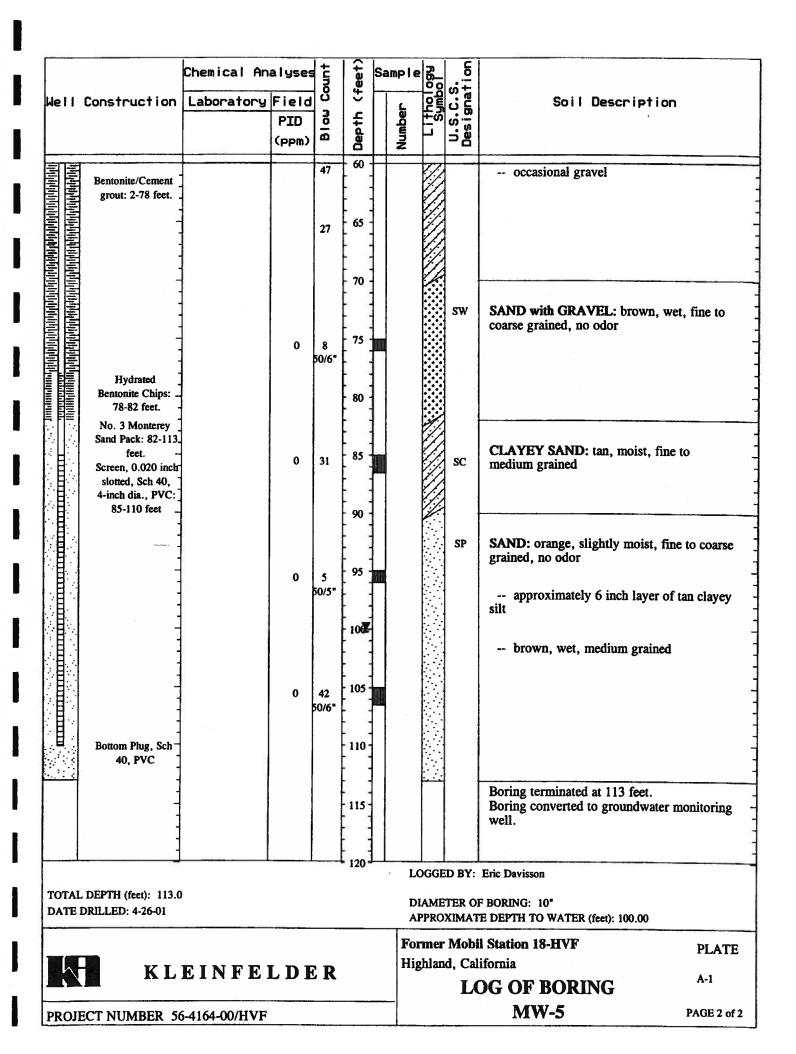
MONIT	<b>CORING WELLS</b>				
UDE (DD)	LONGITUDE (DD)	TOR	FS	TOC	RISER HT
<i>CDL</i> ( <i>DD</i> )	Longin CDE (DD)		(ELEVATION)	(ELEVATION)	huozh_m
206710	-117.2442103	1109.33	1109.32	1108.99	-0.33

	5 CHE	SAPEA	ke dri'	ELD VE, SUIT	E 101	DRI Ci BOI	BUKING LL RIG: ME-75 RING DIAMETER:	DATE DRILLED: 11-19-98 BORING NUMBER:	LOGGED BY: KELLY WINTERS
SAMPLE	BLOW	DEPTH (feet)	WELL CONST	OVA (ppm)	SOIL TYP	1( E	DESCRIPTION AND	REMARKS	
256	COUNT 6-10-11 25-45-49 16-50/5" 27-37-30 16-26-32 32-50/3" 26-40-45 18-38-50 16-40-50 45-50-50 50/6" 21-22-34 19-29-35 35-25-35 20-60-50 MER M 29 BAS					4' FF BI #( BI C(	SAND: as above, medium GRAVEL: with medium so GRAVELLY SAND: fine to GRAVEL: with fine to coord GRAVELLY SAND. fine to SAND: fine to medium, g SAND: as above SAND: as above SAND: as above SAND: fine, tan, very mo SILTY SAND: fine to media SILTY SAND: fine to coard MPLETION NOTES: DIAMETER SCH ROM 41 TO 51 ANK CASING FI 3 MONTEREY SA D/60 SAND FRO ENTONITE FROM DNCRETE AND T	Ind, brown, damp, no odor coarse, reddish broan, damp, very brse sand, brown, damp, very dense coarse, brown, damp to moist, ver gray to brown, very moist, very den ist, very dense, slight odor um, gray, wet, very dense, slight o se, brown, damp, very dense, no od se, brown, damp, very dense, no od se, brown, damp, very dense, no od FEET BGS. 4" DIAMET ROM 41 FEET BGS TO ND FROM 39 TO 52 DM 38 TO 39 FEET BIO 2 TO 38 AND 52 TO TRAFFIC RATED WELL	dense, no odor a, no odor ry dense, no odor se, moderate odor dor dor dor DTTED CASING ER SCH 40 PVC SURFACE. FEET BGS. GS. HYDRATED ) 57 FEET BGS.
ROJECT	NO: 4294—	01	PA	AGE 1 OI	- 1	1- 0	LI 003 10 0K	OUND SURFACE.	

KLEINFELDER       DRILL RIC: CME-75       DATE DRILLED: I1-20-98       LOGGED BY: KELLY         9555 CHESAPEAKE DRIVE, SUITE 101 SAN DIEGO, CALIFORNIA 92123       DRING DIAMETER: BORING DIAMETER: 10"       BORING NUMBER: WW3       KELLY WINTERS         SAMPLE       DEFTH (feet)       WELL (CDIST.       OVA (ppm)       SOIL TYPE       DESCRIPTION AND REMARKS         Image: Sample Soil Image: Soil
9555 CHESAPEAKE DRIVE, SUITE 101 SAN DIEGO, CALIFORNIA 92123       BORING DIAMETER: 10"       BORING NUMBER: MW3       KELLY WW1TERS         SAMPLE       DEPTH (feet)       WELL CONST.       OVA (ppm)       Soil TYPE       DESCRIPTION AND REMARKS         The       BORING DIAMETER: 10"       DEPTH MW3       WELL (feet)       OVA (ppm)       Soil TYPE       DESCRIPTION AND REMARKS         The       BORING DIAMETER: 10"       DEPTH (feet)       WELL (feet)       OVA (ppm)       Soil TYPE       DESCRIPTION AND REMARKS         The       BORING DIAMETER: 10"       O       SP       SAND:medium, brown, damp, no odor         10       SM SILTY SAND Ine to medium, brown, damp, no odor       SP       SAND:medium, ton to brown, damp, no odor         11-19-30       20       O       SP       SAND:fine, light brown, damp, dense, no odor         11-19-30       20       O       SM SILTY SAND:fine to medium, brown, damp, no odor         10-14-30       0       SM SILTY SAND:fine to medium, brown, damp to moist, no odor         10-14-30       40       SM SILTY SAND:fine to coarse, red brown, moist, medium dense, no odor         10-14-30       40       SM SILTY SAND:fine to coarse, red brown, moist, medium dense, no odor         10-14-30       SN SILTY SAND:fine to coarse, red brown, moist, medium dense, no odor       SP SAND:fine to coarse, red brown
SAN DIEGO, CALIFORNIA 92123       BORING DIAMETER:       BORING NUMBER:       WINTERS         SAMPLE       DEPTH       WELL       OVA       SOIL TYPE       DESCRIPTION AND REMARKS         Image: Budwin
But with the construction         (feet)         CONST.         (ppm)           Impe         But the construction         Impe
II = -5 $II = -5$ $II = -5$ $II = -20$ $I$
F-60         SITE:         FORMER MOBIL STATION         25699 BASELINE STREET         HIGHLAND, CALIFORNIA         HIGHLAND, CALIFORNIA         COMPLETION NOTES:         4" DIAMETER SCH 40 PVC 0.020" SLOTTED CASING         FROM 40 TO 55 FEET BGS. 4" DIAMETER SCH 40 PV         BLANK CASING FROM 40 FEET BGS TO SURFACE.         #3 MONTEREY SAND FROM 38 TO 55 FEET BGS.         #0/60 SAND FROM 37 TO 38 FEET BGS. HYDRATED         BENTONITE FROM 2 TO 37 FEET BGS. CONCRETE AND         TRAFFIC RATED WELL BOX FROM 2 FEET BGS TO         SURFACE.
51-4294-01 PAGE 1 OF 1

					t	BURING LO	JG .						
	LE	INF	ELD	DER	DRI	DRILL RIG: DATE DRILLED: LOGGEI							
9555 CHESAPI		עוסח ב	E CIII	TE 101	CI	CME-75 11-20-98 KELLY							
SAN DIEGO,					1	RING DIAMETER.	BORING NUMBER:	WINTERS					
					10	)"	MW4						
SAMPLE DEP (fee		WELL CONST.	OVA (ppm)	SOIL TYP	E DESCRIPTION AND REMARKS								
TYPE COUNT		CONST.											
			С		SP	SAND: fine, light brown, damp	, no odor						
T E	Ř	3 🕅											
	' Å	3 🕅											
						SILTY SAND: fine to medium,	brown, damp, no odor						
	Ř	8 🕅	2	<u>k                                      </u>	SP	SP SAND: medium, brown, damp, no odor							
	5 8	8 🕅											
	Š	8 🕅			GW	SANDY GRAVEL:brown, damp,	no odor						
13-26-40 - 2	0 8	8	о		SP	SAND: modium tag to be							
	Ŭ Å	8 🕅				SAND: medium, ton to brown,	aamp, very aense, no odor						
	5 X	8 🕅											
	Ř	8 🕅											
24-32-36	24-32-36 30 0						SAND.as above						
						SAND. as above							
=3	5 🕅												
	X												
26-45-50/5" - 4	0		0		SAND: as obove, moist								
			Ŭ										
	5												
11-20-20 5			₽ 100			SAND: coarse, gray, wet, dens							
	11-20-20 50 100						ML						
22-45-50 - 5.							moist, very stiff, slight odor						
	-		,5										
	-												
			·····										
SITE:					COMPLETION NOTES:								
FORMER MOBIL STATION 25699 BASELINE STREET						4" DIAMETER SCH 40 PVC 0.020" SLOTTED CASING							
HIGHLAND, CAL					FROM 40 TO 55 FEET BGS. 4" DIAMETER SCH 40 PVC BLANK CASING FROM 40 FEET BGS TO SURFACE. #3 MONTEREY SAND FROM 38 TO 55 FEET BGS. #0/60 SAND FROM 37 TO 38 FEET BGS. HYDRATED								
							TO 37 FEET BGS. (						
					TF	RAFFIC RATED WELL	BOX FROM 2 FEE						
PROJECT NO:		DAC	GE 1 C		Sl	JRFACE.							
51-4294-01			<u>, I</u>	/1									





		Ana	llyses		ber	ot		lodm		SOIL DESCRIPTION
	Well Construction	Lab. Field (ppmv)		Depth ( feet)	Sample Number	Blows per Foot	Sample Type	Lithology Symbol	U.S.C.S. Designation	AND CLASSIFICATION
	12" Traffic-rated well box set in a 3'x3' concrete well pad		+						2 2 2	6" asphalt at surface.
	Backfilled with volclay grout (1-99.5 feet bgs)		3.6	-     	MW -5A -10	-		<u>ה נו נו נו נו נו</u>	SM	Boring air knifed to 8 feet below ground surface (bgs). Silty Sand: Light brown, slightly moist, fine-grained sand, micaceous.
	4-inch sch-40 PVC blank casing (0.5-108 feet bgs)		1.6		MW -5A -15	ŝ.			SP	Same as above, fine- to coarse-grained sand. Poorly-Graded Sand: Pale brown, dry to slightly moist, fine- to medium-grained sand, with trace fine gravel. Poorly-Graded Sand: Pale brown, dry to slightly moist, fine- to medium-grained sand, with some coarse-grained sand, trace fine gravel, some silt. Same as above, decreasing silt, increasing coarse-grained sand. Same as above, trace cobbles up to 4".
SUI			0.8		-5A -20 MW -5A -25				SM SP	Same as above, trace coarse gravel, no cobbles. Silty Sand: Olive brown, slightly moist, fine-grained sand, weakly cemented. Poorly-Graded Sand: Yellowish brown, moist, medium-grained sand, with trace coarse sand.
TO DA DR	RFACE ELEVA TAL DEPTH ( fe TE DRILLED: 6 ILLING COMPA	et): 146 -29-09	I	feet (1,108	3.99 T	D D	IAN EPT	IETI H T	ER OF O STA	D. Hasham BORING (inches): 8 ATIC WATER (feet): 136.76 (7/13/09) THOD: Sonic w/ Continuous Core
PR	OJECT NO. 9	DER ight Solutions.				]		25 High	Jobil Service Station 18HVFPLATE6699 Baseline Street1land, California 924101aIa	

	alyses		ber	ot		mbol		SOIL DESCRIPTION		
Well Construction	Depth (feet)	Sample Number	Blows per Foot Sample Type Lithology Symbol U.S.C.S. Designation				AND CLASSIFICATION (Continued From Previous Page)			
-			-	н.				SP	Poorly-Graded Sand: Yellowish brown moist, medium-grained sand, with trace sand. (continued)	7
-		2.6		MW -5A					Same as above, olive-gray, some silt.	
			- -	-30				- - - - - - - -	Poorly-Graded Sand: Yellowish brown moist, medium- to coarse-grained sand, some silt.	
-		4.9		MW -5A			200 S.		Same as above, medium-grained sand.	
-				-35					Same as above, trace cobbles to 4". Poorly-Graded Sand: Yellowish brown moist, medium-grained sand.	,
			-				and the second		Occasional weakly-cemented silty nodu	les.
		4.1	40—	MW -5A -40					Same as above, some coarse-grained same	nd.
		13.2	- - 45	MW -5A -45			articles and articles		Two large cobbles stuck in drill bit. Recovered sample for 43 to 44 feet bgs consists mostly of rock flour and some of gravel.	coarse
			_						Same as above, trace gravel.	
-		5.2	 50— -	MW -5A -50			a situation and prime a		Poorly-Graded Sand: Yellowish brown moist, medium-gained sand, occasional weakly-cemented silty layers.	, thin
SURFACE ELEVA TOTAL DEPTH (fe DATE DRILLED: 6 DRILLING COMPA	et): 146 -29-09	j	eet (1,108	3.99 T(	D D	IAM EPI	IETI H T	ER OF O STA	D. Hasham BORING (inches): 8 ATIC WATER (feet): 136.76 (7/13/09) THOD: Sonic w/ Continuous Core	
(KLE					25				obil Service Station 18HVF treet, Highland, California 92410	PLAT
PROJECT NO. 9	tight Solutions.							OF BORING MW-5A	1b	

Note: The boundaries between soil types shown on the logs are approximate as the transition between different soil layers may be gradual.

	Ала	lyses		ber	fot		mbol		SOIL DESCRIPTION	
Well Construction	Lab.	Field PID/OVA (ppmv)	Depth (feet)	Sample Number	Blows per Foot	Sample Type	Lithology Symbol	U.S.C.S. Designation	AND CLASSIFICATION (Continued From Previous Page	e)
		5.0	- - 55— -	MW -5A -55			<u>u u u u u u u u u u u u u u u u u u u </u>	SP SM	Silty Sand: Light brownish gray, very mo fine- to coarse-grained sand. Same as above, occasional gray fine sand	
		2.9	- - 60	MW -5A -60				SP	layers. Poorly-Graded Sand: Yellowish brown, moist, fine- to medium-grained sand, with trace coarse grained sand and cobbles, occasional stringers of dark gray micaceor sand. Same as above, with silt as layers, occasic peat-like nodules. Gravelly Sand: Light grayish brown, moi	us onal
		13.7	- - 65 -	MW -5A -65				SM SW	Silty Sand: Brown, moist, dense, fine-gra sand, with some coarse-grained sand. Gravelly Sand: Brown, moist, fine- to coarse-grained sand, with some fine and coarse gravel, some silt.	vel.
		8.3	 - 70— -	MW -5A -70					Same as above, trace cobbles. Same as above, increasing fine-grained sa and silt, decreasing fine and coarse gravel	nd
		16.8	- - 75— -	MW -5A -75				SM	Same as above, increasing fine and coarse gravel. Same as above, increasing silt and fine- to medium-grained sand, trace fine and coarse gravel. Silty Sand: Brown, slightly moist, fine- to medium-grained, with some fine gravel, weakly-cemented fragments. Silty Sand: Olive brown, moist, fine-grain with trace coarse sand.	5e — — — —
SURFACE ELEVA TOTAL DEPTH ( fc DATE DRILLED: 6 DRILLING COMPA	eet): 146 -29-09		eet (1,10	8.99 T(	D D	IAN EPJ	1ETI TH T	ER OF O STA	D. Hasham BORING (inches): 8 ATIC WATER (feet): 136.76 (7/13/09) THOD: Sonic w/ Continuous Core	
KLE	Bright People. R				25				Iobil Service Station 18HVFPlStreet, Highland, California 92410	LATI
PROJECT NO. 9	99863						L	OG (	OF BORING MW-5A	lc

	Ana	alyses		per	ot		mbol		SOIL DESCRIPTION	
Well Construction	Lab.	Field PID/OVA (ppmv)	Depth (feet)	Sample Number	Blows per Foot	Sample Type	Lithology Symbol	U.S.C.S. Designation	AND CLASSIFICATION (Continued From Previous Page	age)
		11.6	 80 	MW -5A -80			<u></u>	SM	Silty Sand: Brown, slightly moist, fine medium-grained, with some fine grave weakly-cemented fragments. (continue Same as above, some medium- to coarse-grained sand, moderately cemen	e- to l, d)
		11.4	  85	MW -5A -85				SP SM	Poorly-Graded Sand: Olive brown, mo fine- to medium-grained sand, with sor Silty Sand: Olive brown, moist, weakl moderately cemented, fine- to medium	me silt. y to -grained
		8.1	- - 90	MW -5A -90				SC	sand, with trace coarse-grained sand, tr coarse gravel. Clayey Sand: Yellowish brown, moist to coarse-grained sand, dense.	race
		3.9	 - 95	MW -5A -95				SM SW	Silty Sand: Yellowish brown, moist, fi coarse-grained sand, dense. Well-Graded Sand: Yellowish brown, fine- to coarse-grained sand, with some some iron oxide staining.	moist,
Hydrated bentonite chips (99.5-105.5 feet		29.5	  100	MW -5A -100				SM	Silty Sand: Mottled yellowish brown a olive gray, moist, fine- to medium-grai sand, with trace coarse-grained sand, w cemented, dense.	ned veakly
bgs) -			-					sw	Well-Graded Sand: Yellowish brown, fine- to coarse-grained sand, with some	
SURFACE ELEVA TOTAL DEPTH (f DATE DRILLED: ( DRILLING COMP.	eet): 146 5-29-09	5	eet (1,10	8.99 T(	D D	IAN EPI	AET TH T	ER OF O STA	D. Hasham BORING (inches): 8 ATIC WATER (feet): 136.76 (7/13/09) THOD: Sonic w/ Continuous Core	
KL	EINFE A				25				obil Service Station 18HVF Street, Highland, California 92410	PLAT
PROJECT NO.							L	OG	OF BORING MW-5A	1d

	Ana	alyses		ber	ot		lodm		SOIL DESCRIPTION
Well Construction	Lab.	Field PID/OVA (ppmv)	Depth ( feet)	Sample Number	Blows per Foot	Sample Type	Lithology Symbol	U.S.C.S. Designation	AND CLASSIFICATION (Continued From Previous Page)
#3 Monterey sand filter pack (105.5-144.5 feet bgs)		23.3	105— - -	MW -5A -105			<u>יייין ה ה ה ה</u> ע ע ע ע ע	SW SM	Well-Graded Sand: Yellowish brown, moist, fine- to coarse-grained sand, with some silt. (continued) Silty Sand: Yellowish brown, slightly moist, fine- to coarse-grained sand, weakly cemented fragments.
4-inch sch-40 PVC 0.02" well screen (108-143 feet bgs)		17.3	- 110 -	MW -5A -110				ML SW	Sandy Silt: Brown, moist, hard, with fine- to medium-grained sand, trace coarse-grained sand. Well-Graded Sand: Yellowish brown, moist, fine- to coarse-grained sand, with some silt.
		9.3		MW -5A -115				SP ML SW	Gravelly Sand: Yellowish brown, moist, medium- to coarse-grained sand, with coarse gravel, some silt. Poorly-Graded Sand: Yellowish brown, moist, medium-grained sand, with occasional approximately 1" hard silt lenses. Sandy Silt: Brown, moist, fine-grained, with some medium-grained sand, trace
		0.7	- - 120 -	MW 5A -120				ML SP	coarse-grained sand. Well-Graded Sand: Yellowish brown, moist, fine- to coarse-grained sand, with trace silt, some coarse gravel. Sandy Silt: Brown, moist, hard, fine-grained, with some medium- and coarse-grained sand. Poorly-Graded Sand: Yellowish brown, slightly moist, medium-grained sand, with some coarse-grained sand, some silt.
		24.3	- 125 - -	MW -5A -125				SM ML SM	Same as above, occasional thin (>1") weakly-cemented silty/clayey layers. Same as above, increasing moisture, bottom of sample contains many hard, weakly-cemented, \dry, silty fragments. \Silty Sand: Brown, moist, fine-grained sand, with some medium- and coarse-grained sand. Sandy Silt: Brown, moist, hard, with fine-grained sand, some medium- and
		15.6	8 	MW					coarse-grained sand.
SURFACE ELEVA' TOTAL DEPTH ( fe DATE DRILLED: 6 DRILLING COMPA	et): 146 -29-09		eet (1,10	8.99 T	D D	IAN EP1	AETI FH T	ER OF O STA	D. Hasham F BORING (inches): 8 ATIC WATER (feet): 136.76 (7/13/09) THOD: Sonic w/ Continuous Core
KLE	SINFE A				25				Iobil Service Station 18HVFPLATEStreet, Highland, California 92410
PROJECT NO. 9	99863						L	OG	OF BORING MW-5A

		Ana	lyses		ber	oot		Iodm		SOIL DESCRIPTION	
	Well Construction	Lab.	Field PID/OVA (ppmv)	Depth (feet)	Sample Number	Blows per Foot	Sample Type	Lithology Symbol	U.S.C.S. Designation	AND CLASSIFICATION (Continued From Previous Page)	)
			-	Ţ _	-5A -130				SM	Silty Sand: Yellowish brown, slightly mois fine- to medium-grained sand, with some coarse-grained sand. (continued)	
			26.7	- 135— -	MW -5A -135				SP	Poorly-Graded Sand: Yellowish brown, moist, medium-grained sand, with trace coa gravel. Same as above, some silt, very moist. Same as above, with silt, very fine-grained.	-
			3.0	  140 	MW -5A -140				SM ML	Silty Sand: Yellowish brown, wet, very fin grained. Silt: Yellowish brown, very moist, stiff, wi some fine-grained sand.	
	Bentonite pellets (144.5-146 feet bgs)		6.1	 145	MW -5A -145						-
										Boring terminated at 146 feet bgs.	- - -
0LND.GDT 8/28/09		TION 1	100.22.5								-
TO PHYEICEN KY	JRFACE ELEVA' DTAL DEPTH ( fe ATE DRILLED: 6 RILLING COMPA	et): 146 -29-09		eet (1,108	3.99 T(	D D	IAN EPI	AETI TH T	ER OF O STA	D. Hasham BORING (inches): 8 ATIC WATER (feet): 136.76 (7/13/09) THOD: Sonic w/ Continuous Core	
ENTAL MOBIL	KLE	INFEL Bright People. Ri				250				Cobil Service Station 18HVFPLAStreet, Highland, California 92410	ATE
ENVIRONMENTAL	ROJECT NO. 9	99863						L	OG	OF BORING MW-5A	lf

Well Construction	Analyses Field Lab. PID/OVA (ppmv)	Depth ( feet)	Sample Number	Blows per Foot	Sample Type	Lithology Symbol	U.S.C.S. Designation	SOIL DESCRIPTION AND CLASSIFICATION	
12" Traffic-rated well box set in a 3'x3' concrete well pad -								6" asphalt at surface. Boring air knifed to 8 feet below grou surface (bgs).	
Backfilled with volclay grout (1-99.5 feet bgs)	3.6	  	MW -5A -10	-		<u></u> 	SM	Silty Sand: Light brown, slightly moi fine-grained sand, micaceous.	- st, - -
4-inch sch-40 PVC blank casing (0.5-108 feet bgs)	1.6	- - 15 -	MW -5A -15				SP	Same as above, fine- to coarse-grained Poorly-Graded Sand: Pale brown, dry slightly moist, fine- to medium-graine with trace fine gravel. Poorly-Graded Sand: Pale brown, dry slightly moist, fine- to medium-graine	to d sand, to d sand, d sand,
	2.5	_ 20— _ _	MW -5A -20				SIM	with some coarse-grained sand, trace f gravel, some silt. Same as above, decreasing silt, increas coarse-grained sand. Same as above, trace cobbles up to 4". Same as above, trace coarse gravel, no cobbles.	- sing - -
5082/8 LG9 OWTO SURFACE ELEVA	0.8	- - 25-	MW -5A -25				SM SP	Silty Sand: Olive brown, slightly moi fine-grained sand, weakly cemented. Poorly-Graded Sand: Yellowish brow moist, medium-grained sand, with trac sand.	//
SURFACE ELEVA TOTAL DEPTH ( f DATE DRILLED: 6 DRILLING COMPA	eet): 146 5-29-09	feet (1,108	3.99 T(	D D	IAN EPT	1ETI TH T	ER OF O EN	D. Hasham F BORING (inches): 8 COUNTERED WATER (feet): 136.76 THOD: Sonic w/ Continuous Core	(7/13/09)
TOTAL DEPTH ( fr DATE DRILLED: 6 DRILLING COMPA	EINFELDER Bright People. Right Solutions. 99863		*	250		Base	eline S	lobil Service Station 18HVF Street, Highland, California 92410 OF BORING MW-5A	PLATE 1a

	Well Construction	Ana Lab.	alyses Field PID/OVA (ppmv)	Depth (feet)	Sample Number	Blows per Foot	Sample Type	Lithology Symbol	U.S.C.S. Designation	SOIL DESCRIPTION AND CLASSIFICATION (Continued From Previous P	age)
			2.6		MW -5A -30				SP	Poorly-Graded Sand: Yellowish brow moist, medium-grained sand, with trac sand. <i>(continued)</i> Same as above, olive-gray, some silt. Poorly-Graded Sand: Yellowish brow	n, ce coarse - - - n,
			4.9	- - 35—	MW -5A -35					moist, medium- to coarse-grained sand some silt. Same as above, medium-grained sand. Same as above, trace cobbles to 4". Poorly-Graded Sand: Yellowish brow	-
			4.1		MW -5A -40					moist, medium-grained sand. Occasional weakly-cemented silty noc Same as above, some coarse-grained s	-
			13.2		MW -5A -45					Two large cobbles stuck in drill bit. Recovered sample for 43 to 44 feet bg consists mostly of rock flour and some gravel. Same as above, trace gravel.	
D.GDT 8/28/09			5.2	  50 	MW -5A -50					Poorly-Graded Sand: Yellowish brow moist, medium-gained sand, occasiona weakly-cemented silty layers.	
J KA	SURFACE ELEVA TOTAL DEPTH ( f DATE DRILLED: 6 DRILLING COMP/	eet): 146 5-29-09	5	feet (1,10	8.99 T	D	IAM EPT	IETI H T	ER OF O EN	D. Hasham F BORING (inches): 8 COUNTERED WATER (feet): 136.76 THOD: Sonic w/ Continuous Core	(7/13/09)
ENVIRONMENTAL MOBIL	PROJECT NO.	Bright People. R	LDER light Solutions.			250		Base	eline S	lobil Service Station 18HVF Street, Highland, California 92410 OF BORING MW-5A	PLATE 1b

	Well Construction	Ana	llyses Field		Sample Number	Blows per Foot	<b>Fype</b>	Lithology Symbol	tion	SOIL DESCRIPTION AND	
		Lab.	PID/OVA (ppmv)	Depth (feet)	ample )	lows pe	Sample Type	itholog	U.S.C.S. Designation	CLASSIFICATION	(a.c.a.)
K// A	-		(ppmv)	<u> </u>	Ň	B	Š	T	SP	(Continued From Previous P	age)
	-		5.0		MW -5A				SM	Silty Sand: Light brownish gray, very fine- to coarse-grained sand.	moist, _
					-55					Same as above, occasional gray fine satisfy layers.	and -
			2.9	  60	MW -5A -60				SP	Poorly-Graded Sand: Yellowish brow moist, fine- to medium-grained sand, trace coarse grained sand and cobbles, occasional stringers of dark gray mica sand. Same as above, with silt as layers, occ peat-like nodules.	with - ceous -
				_						Gravelly Sand: Light grayish brown, fine- to coarse-grained sand, with fine	moist, _ gravel
			13.7	65—	MW -5A -65				SM SW	Silty Sand: Brown, moist, dense, fine sand, with some coarse-grained sand. Gravelly Sand: Brown, moist, fine- to coarse-grained sand, with some fine and	
			8.3		MW -5A -70					coarse gravel, some silt. Same as above, trace cobbles. Same as above, increasing fine-graine and silt, decreasing fine and coarse gra	- d sand _ avel.
.GDT 8/28/09			16.8	 75 -	MW -5A -75				SM	Same as above, increasing fine and co gravel. Same as above, increasing silt and fine medium-grained sand, trace fine and c gravel. Silty Sand: Brown, slightly moist, fin medium-grained, with some fine grave weakly-cemented fragments. Silty Sand: Olive brown, moist, fine-s with trace coarse sand.	e- to oarse -  e- to el,
F.GPJ KA	SURFACE ELEVA TOTAL DEPTH ( fc DATE DRILLED: 6 DRILLING COMPA	eet): 146 5-29-09	)	feet (1,10	8.99 T	D D	ІАМ ЕРТ	1ETH TH T	ER OF O EN	D. Hasham 7 BORING (inches): 8 COUNTERED WATER (feet): 136.76 THOD: Sonic w/ Continuous Core	(7/13/09)
ENTAL MOB	KLE	EINFE A Bright People. R				250				lobil Service Station 18HVF Street, Highland, California 92410	PLATE
ENVIRONM	PROJECT NO. 9	99863						L	OG	OF BORING MW-5A	1c

		Ana	lyses		L.			bol			
Well Construc	ction		Field		Sample Number	Blows per Foot	ype	y Symbol	ion	SOIL DESCRIPTION AND	
, on construc		Lab.	PID/OVA	ų ()	ple N	vs pe	Sample Type	Lithology	U.S.C.S. Designation	AND CLASSIFICATION	
			(ppmv)	Depth ( feet)	Samj	Blov	Sam	Lithe	U.S. Desi	(Continued From Previous P	age)
			11.6	80	MW -5A -80				SM	Silty Sand: Brown, slightly moist, fin medium-grained, with some fine grave weakly-cemented fragments. <i>(continue</i> Same as above, some medium- to coarse-grained sand, moderately ceme	e- to el, <i>ed)</i>
	1		11.4	  85	MW -5A				SP	Poorly-Graded Sand: Olive brown, m fine- to medium-grained sand, with so	me silt.
	<u> </u>				-85				SM	Silty Sand: Olive brown, moist, weak moderately cemented, fine- to medium sand, with trace coarse-grained sand, t coarse gravel.	n-grained
			8.1	90— _	MW -5A -90				SC	Clayey Sand: Yellowish brown, mois to coarse-grained sand, dense.	-
	-								SM	Silty Sand: Yellowish brown, moist, t coarse-grained sand, dense.	fine- to
			3.9	 95 -	MW -5A -95				sw	Well-Graded Sand: Yellowish brown fine- to coarse-grained sand, with som some iron oxide staining.	
Hydrated	1 1		29.5		MW	*			SM	Silty Sand: Mottled yellowish brown olive gray, moist, fine- to medium-gra sand, with trace coarse-grained sand, w cemented, dense.	ined .
Hydrated bentonite ch (99.5-105.5 bgs)				100— — —	-5A -100				sw	Well-Graded Sand: Yellowish brown fine- to coarse-grained sand, with som	, moist, e silt.
SURFACE ELI TOTAL DEPT DATE DRILLI DRILLING CO	H ( fe ED: 6	et): 146 -29-09		eet (1,10	8.99 T	D D	IAN EP	ЛЕТ. ГН Т	ER OF O ENG	D. Hasham F BORING (inches): 8 COUNTERED WATER (feet): 136.76 THOD: Sonic w/ Continuous Core	(7/13/09)
(	KLE	Bright People. R				250				obil Service Station 18HVF Street, Highland, California 92410	PLATE
PROJECT N	) 0. 9	99863						L	OG	OF BORING MW-5A	1d

ENVIRONMENTAL MOBIL 18-HVF.GPJ KA\_RDLND.GDT 8/28/09

Well Construction	Analyses Field Lab. PID/OVA (ppmv)	Depth ( feet)	Sample Number	Blows per Foot	Sample Type	Lithology Symbol	U.S.C.S. Designation	SOIL DESCRIPTION AND CLASSIFICATION (Continued From Previous P	age)
#3 Monterey sand filter pack (105.5-144.5 feet bgs)	23.3	105— — —	MW -5A -105				SW SM	Well-Graded Sand: Yellowish brown, fine- to coarse-grained sand, with som (continued) Silty Sand: Yellowish brown, slightly fine- to coarse-grained sand, weakly co fragments.	, moist, e silt  moist,
4-inch sch-40 PVC 0.02" well screen (108-143 feet bgs)	17.3	 110  	MW -5A -110				ML SW	Sandy Silt: Brown, moist, hard, with medium-grained sand, trace coarse-gra sand. Well-Graded Sand: Yellowish brown, fine- to coarse-grained sand, with som	mined
	9.3	- 115 -	MW -5A -115				SP ML SW	Gravelly Sand: Yellowish brown, mo medium- to coarse-grained sand, with gravel, some silt. Poorly-Graded Sand: Yellowish brow moist, medium-grained sand, with occ approximately 1" hard silt lenses. Sandy Silt: Brown, moist, fine-graine some medium-grained sand, trace	coarse $\begin{array}{c} & & \\ \hline n, & - & - \end{array}$ asional $\begin{array}{c} & \\ & \\ & \\ & - & - \end{array}$
	0.7	- - 120 - -	MW -5A -120				ML SP	coarse-grained sand. Well-Graded Sand: Yellowish brown, fine- to coarse-grained sand, with trace some coarse gravel. Sandy Silt: Brown, moist, hard, fine-g with some medium- and coarse-graine Poorly-Graded Sand: Yellowish brow slightly moist, medium-grained sand,	e silt, grained, d sand.
B/28/08 	24.3	125-	MW -5A -125				SM ML SM	some coarse-grained sand, some silt. Same as above, occasional thin (>1") weakly-cemented silty/clayey layers. Same as above, increasing moisture, b sample contains many hard, weakly-ce \dry, silty fragments. \Silty Sand: Brown, moist, fine-graine with some medium- and coarse-graine Sandy Silt: Brown, moist, hard, with fine-grained sand, some medium- and coarse-grained sand.	ottom of emented, / _ / _ d sand, _ / _
	15.6		MW						
SURFACE ELEVA TOTAL DEPTH ( f DATE DRILLED: 0 DRILLING COMP	eet): 146 6-29-09	feet (1,108	.99 T	D D	IAN EPT	1ET) TH T	ER OF O EN	D. Hasham F BORING (inches): 8 COUNTERED WATER (feet): 136.76 THOD: Sonic w/ Continuous Core	(7/13/09)
	EINFELDER Bright People, Right Solutions,			256				lobil Service Station 18HVF Street, Highland, California 92410	PLATE
PROJECT NO.	<i></i>					L	OG	OF BORING MW-5A	1e

Well Construction		lyses Field PID/OVA (ppmv)	Depth ( feet)	Sample Number	Blows per Foot	Sample Type	Lithology Symbol	U.S.C.S. Designation	SOIL DESCRIPTION AND CLASSIFICATION (Continued From Previous P	Page)
			- Ţ -	-5A -130				SM	Silty Sand: Yellowish brown, slightly fine- to medium-grained sand, with so coarse-grained sand. <i>(continued)</i>	y moist,
		26.7	 135 -	MW -5A -135				SP	Poorly-Graded Sand: Yellowish brow moist, medium-grained sand, with trac gravel. Same as above, some silt, very moist. Same as above, with silt, very fine-gra	ce coarse
		3.0		MW -5A -140				SM ML	Silty Sand: Yellowish brown, wet, ve grained. Silt: Yellowish brown, very moist, sti some fine-grained sand.	
Bentonite pellets (144.5-146 feet bgs)		6.1	 145 	MW -5A -145					Boring terminated at 146 feet bgs.	
SURFACE ELEVA TOTAL DEPTH ( fc DATE DRILLED: 6 DRILLING COMPA	eet): 146 -29-09		èet (1,10	8.99 T	D	IAN EP1	ЛЕТ ГН Т	ER OF O EN	D. Hasham BORING (inches): 8 COUNTERED WATER (feet): 136.76 THOD: Sonic w/ Continuous Core	(7/13/09)
KLE	EINFE I Bright People. R				25				obil Service Station 18HVF Street, Highland, California 92410	PLATE
PROJECT NO. 9							L	OG	OF BORING MW-5A	1f

	Chemical Ar	n <b>a i yse</b> s	t	(feet)	Sa	ample	560	ion		
ell Constructio	on Laboratory	Field PID (ppm)	Blow Co	Depth (f		Number	Lithold Symbo	U.S.C.S Designat	Soil Description	
+ 12-inch, flus			-	0	T				$ASPHALT: \sim 4$ inches thick	
mounted, traff rated well van Blank casing, S 40, 4-inch dis PVC: 0-45 fee Bentonite/Ceme Grout: 2-40 fee	t. ] ch ] ., ] t. ] nt ]		26	- 5				SP	SAND: dark brown, slightly moist, find medium grained, trace silt, no odor	e to
			29	- 10					light brown, moist, loose, fine grai trace coarse sand, no odor SAND: tan, dry, fine to coarse grained	
	]				-			SP	sub angular gravel to 1/2 inch, no odor	, trace
			66	- 15					slightly moist to moist, medium to coarse grained, trace fine gravel, no od cobbles at approximately 17 feet	
			54	- 20					slightly moist, trace sub rounded g to approximately 1/2 inch, no odor	ravel
			42	- 25				SP	SAND: red-brown, moist, fine grained, slightly micaceous, no odor	,
	-		48	30	-				fine to medium grained, some coar \sand, loose, no odor	se
			62	- 35				SP	SAND: gray-brown, slightly moist to n fine to medium grained, some coarse sa loose, trace silt, no odor red-brown coarse granitic gravel, fairly weath tan, slightly moist, fine to coarse g sand	nd, ered
Bentonite Chips 40-43 feet.	- - - -		38	- 40				SM	approximately 4 inch thick seam of red-orange, fine grained, highly oxidize sand	f ed
No. 3 Montere Sand Pack: 43-				F				sw	SILTY SAND: brown, fine grained, mi	uch
feet. Screen, 0.020 in slotted, Sch 40 4-inch dia., PV 45-55 feet.	.]			45					SAND: brown, slightly moist, loose, fi coarse grained, with well rounded grave inches, iron staining	ne to el to 3
OTAL DEPTH (feet): 9	1.0			* 50					D. Grossman	
OTAL DEFTH (ICCI). 9 ATE DRILLED: 5-8-01									F BORING: 10" E DEPTH TO WATER (feet): 53.00	
										PLATE
K	LEINFE	LDE	E R			Hig	,hlan		ifornia DG OF BORING	-2
DOIECT NI IMPED	56-4164-00/HVF					-			<b>MW-6</b> PA	GE 1 of 2

	Chemical f Laborator	Ana lyses	nt n	(feet)	Sam	ple		S. ation	
ell Construction	Laborator	Y Field PID (ppm)		Depth (f		Number i tha	S S L M	U.S.C.S Designat	Soil Description
Backfilled with sold from cuttings: 75-90 feet.			47 70 17 50/4" 18 50/4" 27 26	50 55 55 60 60 65 70 70 75 80 80 85 80 90 90 90 90				SM SW SM SC ML SW SM	<ul> <li>SILTY SAND: dark brown, slightly moist to moist, fine to medium grained, non plastic, with clay</li> <li>SAND: brown, loose, fine to coarse grained, with fine gravel</li> <li>SILTY SAND: gray, wet, fine to coarse grained, with iron staining <ul> <li>groundwater encountered at approximately 53 feet</li> </ul> </li> <li>SILT: olive, wet, micaceous</li> <li>CLAYEY SAND: brown, fine to medium grained</li> <li>CLAYEY SAND: brown, fine to medium grained</li> <li>CLAYEY SILT: brown, damp, plastic, approximately 18 inch aquiclude layer</li> <li>SAND: brown, fine to coarse grained</li> <li>SILTY SAND: gray-brown, wet, fine to medium grained, no odor, fines matrix supporting coarser grains</li> <li>SAND: light brown, moist, medium grained, no odor</li> <li>brown, medium to coarse grained, with rounded to sub rounded gravel to 1 inch, no odor</li> <li>red-brown, trace granitic cobble</li> </ul> <li>CLAY: light brown, slightly moist, hard, low plasticity, no odor</li> <li>SILTY SAND: red-brown, moist, dense, fine grained, no odor</li> <li>Boring terminated at 91 feet. Groundwater encountered at approximately 53 feet.</li> <li>Borehole converted to groundwater monitoring well.</li>
OTAL DEPTH (feet): 91.0 ATE DRILLED: 5-8-01		<u> </u>		100 -		DIAN	MEI	TER OI	D. Grossman F BORING: 10' E DEPTH TO WATER (feet): 53.00
KL	EINFE	LDE	C R			Form	er ]	Mobi I, Cal	I Station 18-HVF PLATI ifornia A-2
ROJECT NUMBER 56									MW-6 PAGE 2 of

	Chemical A	nalyses			lber	oot	/mbol		SOIL DESCRIPTION	
Well Construction		Field		Type	Nun	per F(	gy Sy	S. ation	AND	
	Lab.	PID (ppm)	Depth (feet)	Sample Type	Sample	Blows per Foot	Lithology Symbol	U.S.C.S. Designation	CLASSIFICATION	
Morrison-Dubuque traffic rated well box - set in 3X3' concrete pad									3-inches asphalt at grade Air knifed and cleared borehole to 8-fe	et bgs
2 to 29 feet bgs: Annulus backfilled with bentonite grout slurry			5					SP		
		2.6		THEFT	rw7 -10	12 15 20			<b>Poorly-Graded Sand with Gravel:</b> M light gray and orange-brown, moist, fin subangular sand, some fine subangular trace medium sand, dense, no odor	e
		18			fW7 -15	14 21 27	-14	SP	Poorly-Graded Sand: Mottled light g orange-brown, moist, fine to medium s dense, no odor	and,
			-					SM	Silty Sand: Brown, moist, micaceous f sand, some silt, dense, no odor	
0.5 to 35 feet bgs: 4-inch dia. Schedule 40 PVC casing		1.4	20— — — —		rw7 ·20	14 21 30		SP	Poorly-Graded Sand: Same as above, dense	very
TOP OF CASING ELE TOTAL DEPTH (feet): DATE DRILLED: 9-1-0	66.5	feet): 1	MSL				D	IAME	D BY: V. Prast TER OF BORING (inches): 10 TO STATIC WATER (feet): N/A bgs	
KLEI	NFEI	DE	E R			il 18-1 9 Bas			et, Highland, California 92410	PLAT
PROJECT NO. 516	86			T	0	GO	FF	sUb	ING MW-7	2a

Well Construction	Lab.	alyses ield PD H pm) Q		Sample Number	Blows per Foot	Lithology Symbol	U.S.C.S. Designation	SOIL DESCRIPTION AND CLASSIFICATION (Continued From Previous Pag	re)
2 to 29 feet bgs: Annulus backfilled with bentonite grout slurry 0.5 to 35 feet bgs:		).4		1W7 -25	12 17 23	1-11 11 11	SP	Alternating layers of fine to coarse graine sand <b>Poorly-Graded Sand:</b> Mottled olive-gray orange-brown, moist, dense, no odor	
4-inch dia. Schedule 40 PVC casing 29 to 32 feet bgs: Hydrated bentonite chip seal		2.8	11111	1W7 -30	17 19 32	<u></u>	SM	Silty Sand: Olive-gray, moist, fine to mee sand, some silt, very dense, no odor	dium
32 to 56 feet bgs: Montercy #3 filter sand	6	.4		IW7 -35	24 30 34		SP	<b>Poorly-Graded Sand:</b> Olive-brown, mois fine to medium micaceous angular to subangular sand, trace gravel, very dense, odor	•
35 to 55 feet bgs: 4-inch dia. Schedule 40 PVC well screen with 0.02-inch slots	7	.2		W7 40	14 24 28		ML SP	Sandy Silt: Mottled olive-gray and orange-brown, moist, silt, some fine micaceous sand, hard, no odor Poorly-Graded Sand: Mottled orange-br fine to coarse sand, very dense, no odor	own,
	9.	2	NIIII	W7 45	30 50		SP	Poorly-Graded Sand with Gravel: Mott light gray and orange-brown, moist, mediu to coarse sand, some fine sand, some angu to subrounded gravel, very dense, no odor	ım lar
TOP OF CASING ELEV TOTAL DEPTH (feet): DATE DRILLED: 9-1-0	66.5	vt): MSL				DL	AMET	D BY: V. Prast TER OF BORING (inches): 10 TO STATIC WATER (feet): N/A bgs	
KLEII	NFEL:	DER			18-F Base		Street	, Highland, California 92410	ATE,
PROJECT NO. 5168	6			DO	<b>G</b> [ <b>O</b> ]	FB	ORI	NG MW-7	2Ъ

		Chemica	Analyses		ber	ot	lodin		SOIL DESCRIPTION	
Well	Construction		Field		Sample Type Sample Number	Blows per Foot	Lithology Symbol	U.S.C.S. Designation	AND	
		Lab.	PID	Depth (feet)	nple	1 SAVC	holo	S.C.S signe	CLASSIFICATION	
<del></del>		<u> </u>	(ppm)	<u>Å</u>	Sal Sal	Ā		эğ	(Continued From Previous Pa	ige)
	32 to 56 feet bgs: Monterey #3 filter sand		2.0		50	7 24 33 42		SP- SM	Initial perched water at 50-feet bgs <b>Poorly-Graded Sand with Silt</b> : Gray, fine to medium sand, very dense, no od	wet, lor
	35 to 55 feet bgs: 4-inch dia. Schedule 40 PVC well screen with 0.02-inch slots									
	Threaded bottom cap		3.7	.55 -	-55	10		MI.	Sandy Silt: Mottled orange-brown and olive-gray, moist, micaceous silt, some	fine to
	56 to 60 feet bgs: Borehole backfilled with hydrated bentonite chips					14			medium sand, little to some clay, very s odor	stiff, no
	60 to 66.5 feet bgs: Borehole caved, no backfill		3.7	- 60  -				GM	Silty Gravel with Sand: Olive-gray, m schist gravel, some fine to coarse sand, silt, very dense, no odor	noist, some
in de la dela de la dela de la dela de la dela de	- - - - - - - - 		0.0			7 33 35 50		GM	Silty Gravel with Sand: Mottled orange-brown and olive-gray, moist, metamorphic gravel, some fine to coars some silt, very dense, no odor	e sand
	-									
	OF CASING ELEY		(feet): 1	MSL					DBY: V. Prast	
	AL DEPTH (feet): E DRILLED: 9-1-0				1				TER OF BORING (inches): 10 TO STATIC WATER (feet): N/A bgs	
	KLEI	NFE	LDE	R		oil 18- 9 Bas		Stree	et, Highland, California 92410	PLAT
PRO	JECT NO. 516	86			LO	GC	)F F	OR	ING MW-7	2c

	Chemical	Analyses		i	of In	nbol		SOIL DESCRIPTION	
Well Construction		Field		Sample Type Sample Number	Blows per Foot	Lithology Symbol	i. ation	AND	
	Lab.	PID	Deptlı (feet)	unple	ovys r	tholo	U.S.C.S. Designation	CLASSIFICATION	
Morrison-Dubuque		(ppm)	<u> </u>	ີ ທີ່		E .	эă	3-inches asphalt at grade	<u> </u>
traffic rated well box set in 3X3' concrete	-		-			• E		Air knifed and cleared borehole to 8-feet bg	s
pad	-		-			0			
	-		-			• [			
	-		-			0.			
2 to 30 feet bgs:	4		5-			0 o	SP		
Annulus backfilled with bentonite grout			_			20	ər		
slurry						• O			
			-			° C			
			_			00			
			10-			0			
		38.8	10	-1(	12	0 0	SP	Poorly-Graded Sand with Gravel: Olive- fine to medium sand, some gravel, medium	gга
				Ш	15	.0		dense, no odor	
						0			
						0			
		8.9	15—	-15			SP	Poorly-Graded Sand: Mottled olive-gray a orange-brown, moist, fine to medium sand,	nd
				Ш	26	1.2		dense, no odor	
	1						ŀ		
	1		_			° q			
						0.0			
0.5 to 35 feet bgs: 4-inch dia. Schedule		14.1	20-	//W		• C	SP	Poorly-Graded Sand with Gravel: Mottlee light gray and orange-brown, fine to medium	ł
40 PVC casing			-	Щ	26	0		sand, some fine gravel, dense	:
			-			in	ļ		
							,		
			-					Sandy Silt: Olive-gray, moist, silt, some fine	
OP OF CASING ELE	VATION	(feet) M	ISL.				)GGF	DBY: V. Prast	
DTAL DEPTH (feet):								FER OF BORING (inches): 10	
ATE DRILLED: 9-1-0	)5					DH	EPTH	TO STATIC WATER (feet): N/A bgs	
	י גד קד זא		<b>n</b>	Mol	bil 18	HVF		PLA	TF
KLEI	IN IL IE I	սոռ	ĸ	2569	99 Ba	seline	Stree	t, Highland, California 92410	
								3a	
ROJECT NO. 516	86			LO	) <b>G</b> (	)F B	OR	ING MW-8	

Well Construction	Chemical /	Analyses Field		rype Vumber	er Foot	Lithology Symbol	ion	SOIL DESCRIPTION AND	
	Lab.	PID (ppm)	Depth (feet)	Sample Type	더 Blows per Foot		U.S.C.S. Designation	CLASSIFICATION (Continued From Previous Pag	e)
2 to 30 feet bgs: Annulus backfilled with bentonite grout slurry				-25	20 25		ML SP	sand, very stiff, no odor Poorly-Graded Sand: Same as above	
0.5 to 35 feet bgs: 4-inch dia. Schedule 40 PVC casing 30 to 32 feet bgs: Hydrated bentonite chip seal		11.7			3 16 26 31		SP	<b>Poorly-Graded Sand:</b> Mottled light gray orange-brown, moist, fine to medium sand trace gravel, very dense, no odor	' and d,
32 to 56 feet bgs: Monterey #3 filter sand		24.7	-   	- - 	15 26 30		SP	<b>Poorly-Graded Sand</b> : Mottled light gray orange-brown, fine to medium sand, little coarse sand, trace gravel, very dense, no o	
35 to 55 feet bgs: 4-inch dia. Schedule 40 PVC well screen with 0.02-inch slots		7.7	 40  	-40	17 23 34		SP	<b>Poorly-Graded Sand with Gravel:</b> Mott light gray and orange-brown, fine to coars grained, predominantly fine to medium, so gravel, very dense, no odor	e
		9.2	45  	-45	15 23 30		SP	. Poorly-Graded Sand with Gravel: Same above	e as
日: IOP OF CASING ELEV IOTAL DEPTH (feet): : DATE DRILLED: 9-1-04	56.5	feet): MS	_⊽ SL			DL	AMET	O BY: V. Prast ER OF BORING (inches): 10 IO STATIC WATER (feet): N/A bgs	
KLEIN	NFEI	DE]	R	Mobi 25699			Street	, Highland, California 92410	ATE
PROJECT NO. 5168	6			LO	20	FR	UDI	NG MW-8	3b

			Chemical Analyses		TT			Ī			
	Wel	Il Construction	Lab. PID (ppm)	Depth (feet)	Sample Type	Sample Number	Blows per Foot	Lithology Symbol	U.S.C.S. Designation	SOIL DESCRIPTION AND CLASSIFICATION	
RDLND.GDT 9/19/05		32 to 56 feet bgs: Monterey #3 filter sand 35 to 55 feet bgs: 4-inch dia. Schedule 40 PVC well screen with 0.02-inch slots Threaded bottom cap	8.1	55		1778 1778 1-50	6 9 13		SP SM ML	Initial wet zone at 50-feet bgs <b>Poorly-Graded Sand with Gravel</b> and orange-brown, wet, fine to coar predominantly medium grained, son dense Silty Sand: Brown, moist, fine şand medium sand, trace clay, dense, no of <b>Sandy Silt with Clay</b> : Olive-brown silt, some clay, fine to medium mica sand with iron oxde stained soil frac stiff, no odor	: Light gray se sand, ne gravel, , trace odor
18-HVF.GPJ KA RDLN	TOTA	L DEPTH (feet): 5 DRILLED: 9-1-05	6.5					DIA	AMEI	DBY: V. Prast ER OF BORING (inches): 10 FO STATIC WATER (feet): N/A bgs	
ENVIRONMENTAL 18-1	h	KLEINFELDER					18-H Base		Street	, Highland, California 92410	PLATE
ENVIRO	PROJ	ECT NO. 5168		L	)G	OI	F BO	ORI	NG MW-8	3c	

	Well Construction		llyses Field PID/OVA (ppmv)	Depth ( feet)	Sample Number	Blows per Foot	Sample Type	Lithology Symbol	U.S.C.S. Designation	SOIL DESCRIPTION AND CLASSIFICATION	
	12" Traffic-rated well box set in a 3'x3' concrete well pad			  5						2" asphalt at surface. Borehole airknifed to 8 feet below gro surface (bgs).	
	Backfilled with volclay grout (1-95.5 feet bgs) 4-inch sch-40 PVC blank		1.6		MW -8A -10 MW -8A -15	1			SP SP/GP	Poorly-Graded Sand: Brown, moist, fine-grained sand, trace silt, trace grav 1/2", micaceous. Same as above, no silt. Poorly-Graded Sand with Gravel: Light-brown, moist, medium- to	- rel up to - - 
GDT 8/28/09	casing (0.5-105 - feet bgs) - - - - - - - - - - - - - - - - - - -		3.8	 20—  25—	MW -8A -20 MW				ML SP/GP	Poorly-Graded Sand and Gravel: Ligl slightly moist, fine- to medium-graine with coarse gravel up to 3". Sandy Silt: Olive gray, moist, stiff, w fine-grained sand. Poorly-Graded Sand and Gravel: Mot	$\frac{1}{2}$
KA_RDLND	GURFACE ELEVA TOTAL DEPTH ( fa DATE DRILLED: 4 DRILLING COMP	eet): 135 -16-09	i			D	IAN EPT	1ETI TH T	ER OF O EN	<ul> <li>light gray and orange, moist, fine- to</li> <li>V. Reynolds</li> <li>F BORING (inches): 8</li> <li>COUNTERED WATER (feet): 121.01</li> <li>THOD: Sonic w/ Continuous Core</li> </ul>	(6/16/09)
ENVIRONMENTAL MOBIL 18-HVF.GPJ	PROJECT NO.	Bright People. R 99863				250		Base	eline S	lobil Service Station 18HVF Street, Highland, California 92410 OF BORING MW-8A	PLATE 2a

	Well Construction	Ana Lab.	ilyses Field PID/OVA (ppmv)	Depth ( feet)	25-25	Blows per Foot	= Sample Type	Lithology Symbol	U.S.C.S. Designation	SOIL DESCRIPTION AND CLASSIFICATION (Continued From Previous P	age)
			0.0		-23 MW -8A -30				ML SP SM SP	<ul> <li>medium-grained sand, with some</li> <li>coarse-grained sand, gravel up to 2".</li> <li>Sandy Silt: Olive gray, moist, with</li> <li>fine-grained sand, micaceous, red iron</li> <li>and dark gray staining. (continued)</li> <li>Poorly-Graded Sand: Red-brown, mo</li> <li>to medium-grained sand.</li> <li>Silty Sand: Olive-gray, moist, with fi</li> <li>medium-grained sand.</li> <li>Poorly-Graded Sand: Brown, moist, f</li> <li>medium-grained sand, with some subicoarse gravel up to 1".</li> </ul>	ist, fine- $\frac{1}{1}$ ne- to
			1.9	 35—	MW -8A -35					Same as above, some weakly-cementer fine-grained sand. Poorly-Graded Sand: Light-brown, m fine- to medium-grained sand, with so gravel.	oist, me fine
			0.2		MW -8A -40					<ul><li>Poorly-Graded Sand: Light-brown, m to medium-grained sand, with trace sil cementation.</li><li>Same as above, trace fine gravel.</li><li>Same as above, red-brown, some silt.</li></ul>	
			0.0	 45 	MW -8A -45				SP/GF	moist, fine- to medium-grained sand, coarse subangular gravel up to 1.5".	with
D.GDT 8/28/09			0.7		MW -8A -50				SP	Poorly-Graded Sand: Red-brown, mo to medium-grained sand. Same as above, mottled light brown as orange, increasing medium-grained sa coarse gravel up to 1".	nd
IS HVF.GPJ KA_RDLN	URFACE ELEVA OTAL DEPTH ( f ATE DRILLED: 4 RILLING COMP/	eet): 135 -16-09	5	•		D D	IAN EPT	1ET) TH T	ER OI O EN	V. Reynolds F BORING (inches): 8 COUNTERED WATER (feet): 121.01 THOD: Sonic w/ Continuous Core	(6/16/09)
ENVIRONMENTAL MOBIL 18-HVF.GPJ KA_RDLND	ROJECT NO.	Bright People. F				250		Base	eline	Iobil Service Station 18HVF Street, Highland, California 92410 OF BORING MW-8A	PLATE 2b

	Well Construction	Ana Lab.	lyses Field PID/OVA (ppmv)	Depth (feet)	Sample Number	Blows per Foot	Sample Type	Lithology Symbol	U.S.C.S. Designation	SOIL DESCRIPTION AND CLASSIFICATION (Continued From Previous P	age)
	-			-				111	SP	Same as above, increasing gravel.	
	-		0.4	55— -	MW -8A -55				ML	Sandy Silt: Olive gray, moist, with fir medium-grained sand, with trace clay, red iron staining.	ne- to trace
	-									Sandy Silt: Olive gray, very moist, sti coarse gravel/cobbles up to 4".	ff, with -
			0.0	 60 	MW -8A -60				SP/GP	Poorly-Graded Sand and Gravel: Oliv moist, fine- to medium-grained sand, v coarse gravel up to 2", micaceous.	
	-			_				0 0 0		Same as above, red-brown. Same as above, decreasing gravel.	
			0.0	 65 	MW -8A -65				ML SP	Sandy Silt: Olive gray, moist, stiff, with to medium-grained sand, red iron stain Poorly-Graded Sand: Brown, moist, fi medium-grained sand, with coarse gra 1.5".	ing/_
	-									Same as above, decreasing gravel, weakly-cemented sand.	
			0.4	70	MW -8A -70				GP	Poorly-Graded Gravel: Light gray, sli moist, coarse subangular gravel up to 2 possibly broken cobbles, with fine- to medium-grained sand.	
60	-		0.7	 75 	MW -8A -75				SM	Silty Sand: Brown, moist, fine-graine with silt.	
D.GDT 8/28/05	-								SP	Poorly-Graded Sand: Brown, moist, f	ine- to
T KA	SURFACE ELEVA OTAL DEPTH ( fe DATE DRILLED: 4 DRILLING COMPA	eet): 135 16-09	i			D D	ІАМ ЕРТ	1ETI TH T	ER OF O EN	V. Reynolds 7 BORING (inches): 8 COUNTERED WATER (feet): 121.01 THOD: Sonic w/ Continuous Core	(6/16/09)
UTAL MOBIL	KLE					250				lobil Service Station 18HVF Street, Highland, California 92410	PLATE
ENVIRONMEL	PROJECT NO. 9	99863	iynt solutions,					L	OG	OF BORING MW-8A	2c

Note: The boundaries between soil types shown on the logs are approximate as the transition between different soil layers may be gradual.

	Well Construction	Ana Lab.	alyses Field PID/OVA (ppmv)	Depth (feet)	Sample Number	Blows per Foot	Sample Type	Lithology Symbol	U.S.C.S. Designation	SOIL DESCRIPTION AND CLASSIFICATION (Continued From Previous P	age)
			0.0		MW -8A -80				SP	medium-grained sand, micaceous.	
			0.3	 85 	MW -8A -85				CL SC	Sandy Clay: Olive-gray, wet, stiff, wi fine-grained sand, trace micas. Clayey Sand: Olive gray, moist, fine- medium-grained sand, with clay, trace	
			0.0	 90 	MW -8A -90				SP	Same as above, decreasing clay. Poorly-Graded Sand: Red-brown, mo to medium-grained sand, with some cl	- - ist, fine- lay, trace
	Hydrated betonite chips (95.5-101.5 feet bgs)		0.3		MW -8A -95				CL SP	fine gravel. Sandy Clay: 3" thick layer, olive gray very stiff, with fine- to medium-grained Poorly-Graded Sand: Light-brown, sl moist, fine- to medium-grained sand, some fine gravel, trace clay. Same as above, red-brown, moist, fine	, moist, ed sand. / ightly with
8/28/09			0.0	  100  	MW -8A -100					sand, some coarse gravel up to 1". Same as above, trace clay.	-
F.GPJ KA_RDLND.GDT	URFACE ELEVA OTAL DEPTH ( fo DATE DRILLED: 4 DRILLING COMPA	eet): 135 I-16-09	5			D D	IAN EPT	1ET TH T	ER OF O EN	Sandy Clay: 3" thick layer, olive gray V. Reynolds F BORING (inches): 8 COUNTERED WATER (feet): 121.01 THOD: Sonic w/ Continuous Core	
ENVIRONMENTAL MOBIL	ROJECT NO.	EINFE, Bright People, I 99863				250		Base	eline S	lobil Service Station 18HVF Street, Highland, California 92410 OF BORING MW-8A	PLATE 2d

Г				,			1		-					
	X	/ell Construction	Ana	lyses Field		Sample Number	Blows per Foot	ype	Lithology Symbol	uo	SOIL DESCRIPTION AND			
		en construction	Lab.	PID/OVA		le N	s pe	le T	logy	C.S.	CLASSIFICATION			
			Euo.	(ppmv)	Depth ( feet)	amp	Blow	Sample Type	itho	U.S.C.S. Designation		aaa)		
				(PP)		S	Щ	S	77		(Continued From Previous P stiff, with fine- to medium-grained sar	age)		
				0.4	105— — —	MW -8A -105				SC SP	Clayey Sand: Brown, slightly moist, f medium-grained sand, with clay, weak cemented pieces, some gravel up to 0. Poorly-Graded Sand: Light red-brown slightly moist, fine- to medium-graine with some fine gravel, trace clay.	fine- to $\frac{1}{5}$		
		4-inch sch-40 PVC 0.02" well screen (105-135 ft bgs)		0.0	 110 -	MW -8A -110				SC	Clayey Sand: Red-brown, moist, fine- medium-grained sand, with clay, weak cemented. Same as above, decreasing clay.			
		-			_					SP	Poorly-Graded Sand: Red-brown, mo weakly cemented, fine- to medium-gra sand, with some clay and gravel up to	ained		
		-		0.0	115	MW -8A -115					Same as above, no gravel, some coarse-grained sand.	-		
		-			⊊ _ 120—					SP-SC	Same as above, increase in coarse-grai	ned		
		-		0.0		MW -8A -120				CL	Interbedded layers of Clayey Sand and Poorly-Graded Sand: Clayey Sand is brown, very moist, fine- to medium-gr sand, with clay. Poorly-Graded sand i mottled orange and light brown, very fine- tomedium-grained sand. Sandy Clay: Olive brown, very moist.	olive rained s / moist, /		
				0.0		MW -8A -125				SP	Poorly-Graded Sand: Brown, very model to medium-grained sand, micaceous.			
GDT 8/28/09										SC	Clayey Sand: Red-brown, moist, fine- sand with clay, micaceous.	-grained		
DLND	CI ID I	FACE ELEVA	TION: 1	100 66 4	Faat (1 10	) 25 T		<u> </u>		DV-	V. Poynolds			
A R R		AL DEPTH ( fe				7.55 T					BORING (inches): 8			
GPJ		E DRILLED: 4	· ·								COUNTERED WATER (feet): 121.01	(6/16/09)		
H H H		LING COMPA		DC Expl	oration &	: Wells					THOD: Sonic w/ Continuous Core			
ENVIRONMENTAL MOBIL 18-HVF.GPJ KA_RDLND.GDT		$\bigcap$	EINFEI Bright People. R	LDER				]	Forn	ner M	lobil Service Station 18HVF Street, Highland, California 92410	PLATE		
ENVIRONME	PRO	PROJECT NO. 99863						LOG OF BORING MW-8A						

	Well Construction	Ana Lab.	alyses Field PID/OVA (ppmv)	Depth (feet)	Sample Number	Blows per Foot	Sample Type	Lithology Symbol	U.S.C.S. Designation	SOIL DESCRIPTION AND CLASSIFICATION (Continued From Previous P	age)
D.GDT 8/28/09				135-	MW -8A -130 MW -8A -135				SC	Sandy Clay: Brown, moist, with fine- medium-grained sand, less sand than a Boring terminated at 135 ft bgs.	- - - to
L 18-HVF.GPJ KA_RDLN	SURFACE ELEVA TOTAL DEPTH ( f DÀTE DRILLED: 4 DRILLING COMP.	èet): 135 4-16-09	5			D D	IAM EPT	1ETI TH T	ER OF O EN	V. Reynolds F BORING (inches): 8 COUNTERED WATER (feet): 121.01 THOD: Sonic w/ Continuous Core	(6/16/09)
ENVIRONMENTAL MOBIL 18-HVF.GPJ KA_RDLND.GDT	PROJECT NO.	EINFE Bright People. I 99863				256		Base	eline S	lobil Service Station 18HVF Street, Highland, California 92410 OF BORING MW-8A	PLATE 2f

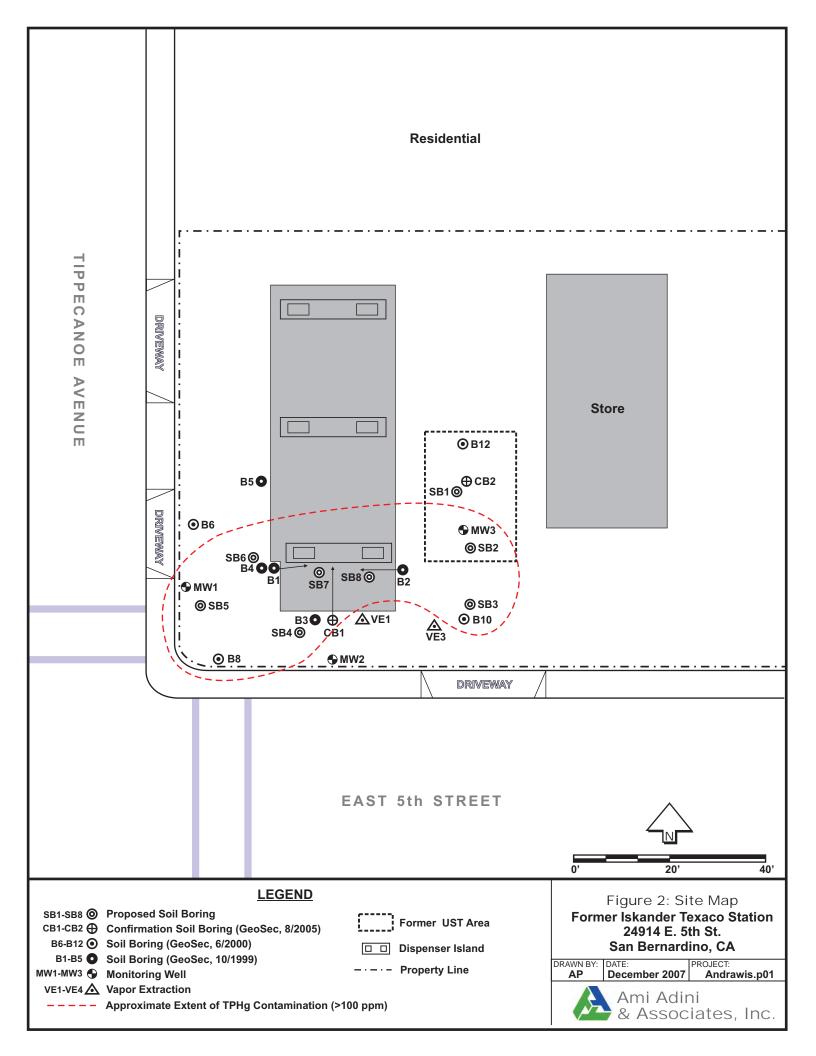
GEOSC	× /	2. WIN	1		١G	LC		LOGGED BY:	
Environmental Construction Maintenance			CME 75			1	DATE DRILLED: 4-11-96		
43218 Bu Temecula	BORING DIAMETER: 8–INCH			IR:	BORING NUMBER: VEW-2A	NJA			
	LOW OUNT	H WELL CONST.	OVA PPM	SOIL TYPE		DESCRIPTION AND REMARKS			
0	- - 5 		0		SP	FINE TO	MEDIUM SAND, brown, slightly domp, l	laase, na ador	
0	- 10 		0		SP	MEDIUM TO COARSE SAND with troce gravel, solt and pepper grey, slightly domp, no odor			
	- 15 		0	-					
D	20 		0		SM SW	SILTY SA	ND, brawn, slightly damp, na odor		
D	– 25 –		0			FINE TO COARSE SAND with trace gravel and cobbles, solt ond pepper grey, slightly damp, no odor			
0	- 30 -		0		ML SW	FINE TO	vn, dry, no odor COARSE SAND with some silt, dk. broi		
0	E 35		0	4	SW SW	MEDIUM TO COARSE SAND with trace gravel, solt and pepper grey, dry, na adar FINE TO COARSE SAND with trace gravel, solt and pepper grey, slightly domp, no adar			
0	- 40 -		10		SP SP	GRAVELLY	D with trace silt, block, damp, slight COARSE SAND, grey, slightly domp, i	······································	
0	- 45 -		20		SP GP SP SW	FINE SAND, grey, slightly damp, slight ador GRAVEL AND COBBLES, grey, slightly damp, slight ador FINE TO MEDIUM SAND with trace gravel, solt and pepper grey, slightly damp, slight od FINE SAND with trace coarse sand and silt, dk brown, slightly damp, slight odor			
0	- 50 -		20		SW	FINE TO (	COARSE SAND with trace gravel, sait a	and pepper grey ta brawn, wet, slight oda	
0	E - 55 -		20		-	SILT with trace sand and trace clay, brown, damp, slight plasticity, slight odor FINE TO COARSE SAND with trace grovel, cabbles and silt, salt ond pepper grey to brown, domp, slight adar			
0	60		20		SW				
0 - 30 30 - 45 0 - 2 F 2 - 28	TON NOTES: FT BGS SCH. FT BGS SCH. T BGS CONCRE FT BGS HYDRA FT BGS #3 S	40 4" DIA. 40 4" DIA ETE TRAFFIC TED BENTC	C RATED			CASING	SITE: MOBIL STATIO 25699 BASEL HIGHLAND, CA	INE ROAD	
46 - 84	5 FT BGS HY	DRATED BE	NTONITE				PROJECT NO. 29506.03	PAGE 1 OF 2	

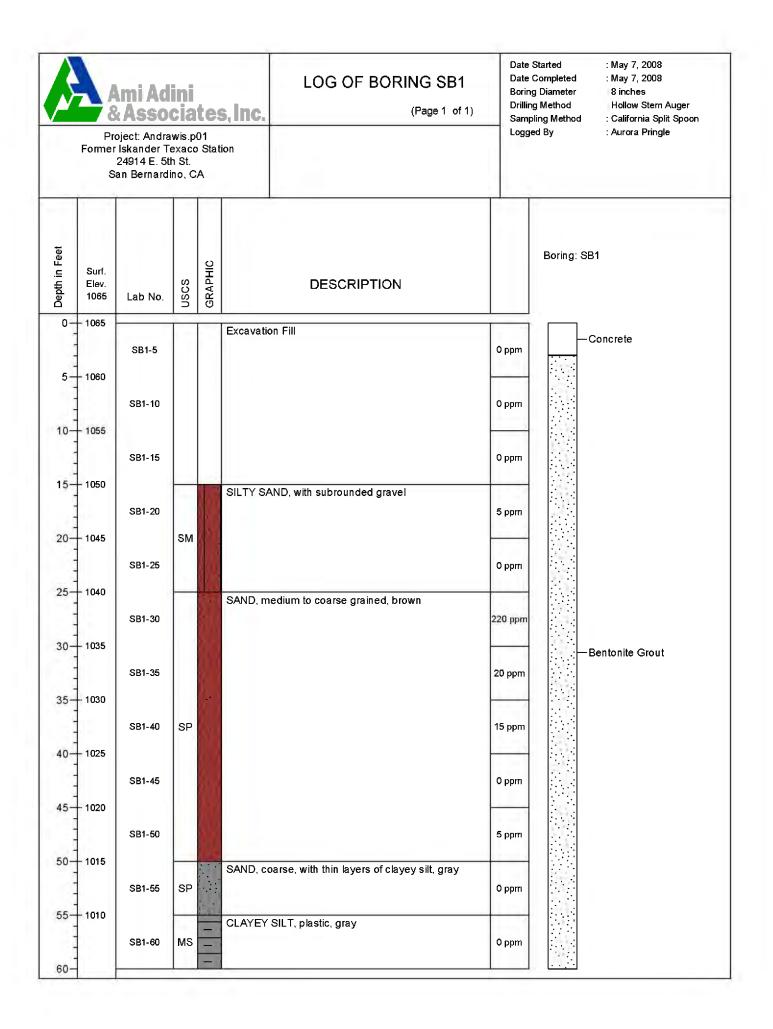
		IR	WIN	BO	RIN	١G	LC	)G			
	Environmental		DRILL RIG:				DATE DRILLED:	LOGGED BY:			
	Construction Maintenance			CME 75				4-11-96			
				BORING DIAMETER:				BORING NUMBER:	NJA		
	43218 Business Temecula, Calif	8-INCH				VEW-2A					
	SAMPLE	OVA AVA									
	SAME LL	MPLE DEPTH WELL FEET CONST.			PPM SOIL						
	BLOW TYPE COUNT	TYPE				DESCRIPTION AND REMARKS					
		-			4	SW					
		- - 65				-		COARSE SAND with trace gravel, cobb , damp, slight odar	es and silt, solt and pepper gre	y	
	0			0							
					. 4						
	0	- 70 -		0	1 <sup>4</sup> 4						
		-		-	à,						
	Û	- 75 E		0	4 .						
					4	-					
		80		0							
		E		12	۸. <sup>۸</sup>						
	0	- 85						<u></u>			
			:								
			:								
								5			
						x					
	~										
ł	COMPLETION N	NOTES:						SITE:		-	
	0 – 30 FT BGS	5 SCH. 4	0_4" DIA.	BLANK	, MOBIL STATION #18-HVF 25699 BASELINE ROAD						
	30 – 45 FT BG										
	0 - 2 FT BGS 2 - 28 FT BGS	5 HYDRAT	ED BENT		HIGHLAND, CA	LIFORNIA					
	28 - 46 FT BG 46 - 84.5 FT I	S #3 SA	ND								
		200 110			PROJECT NO.	PAGE 2 OF	2				
		<u></u>			29506.03	PAGE 2 OF	2				

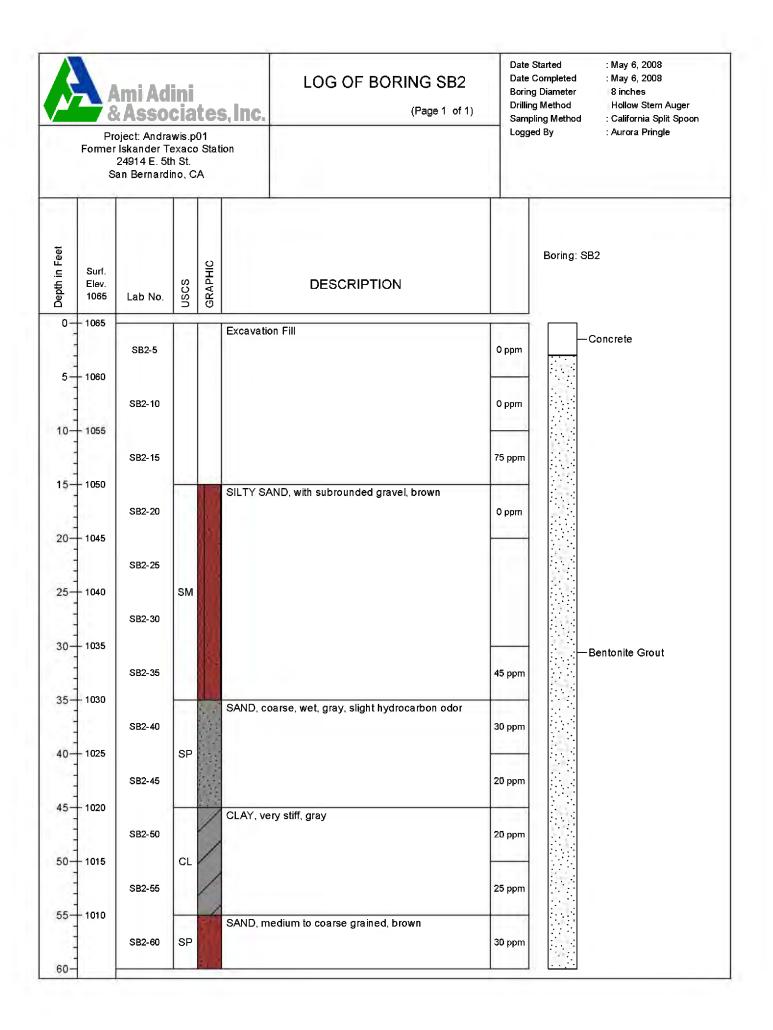


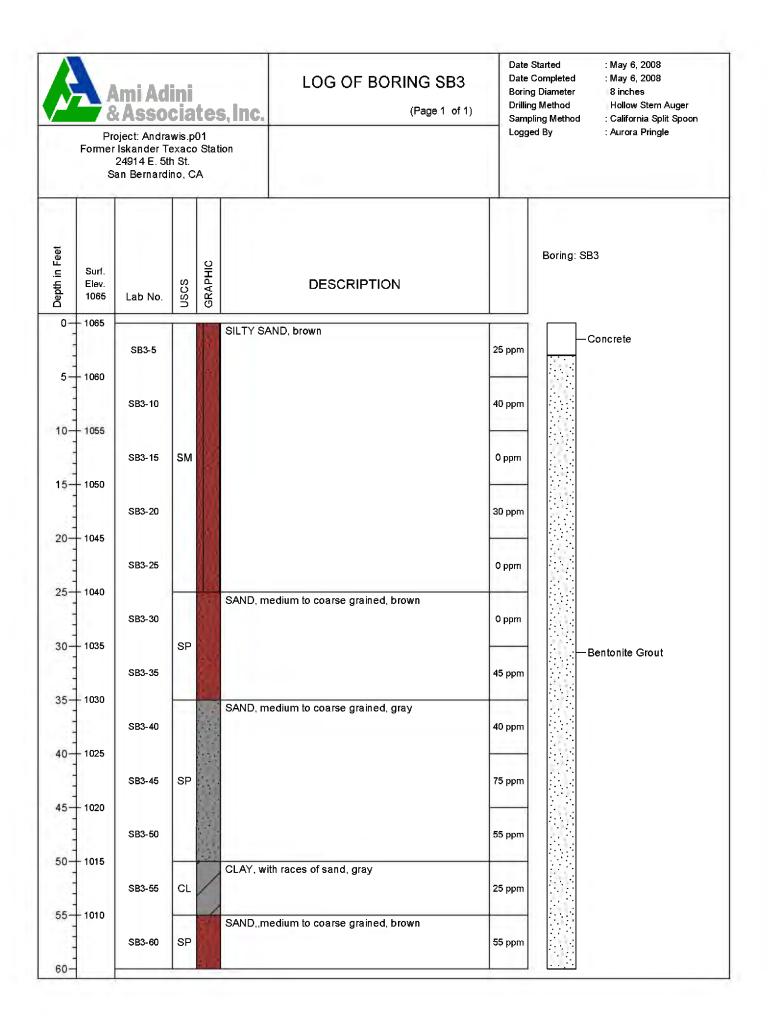
## **APPENDIX C2.4**

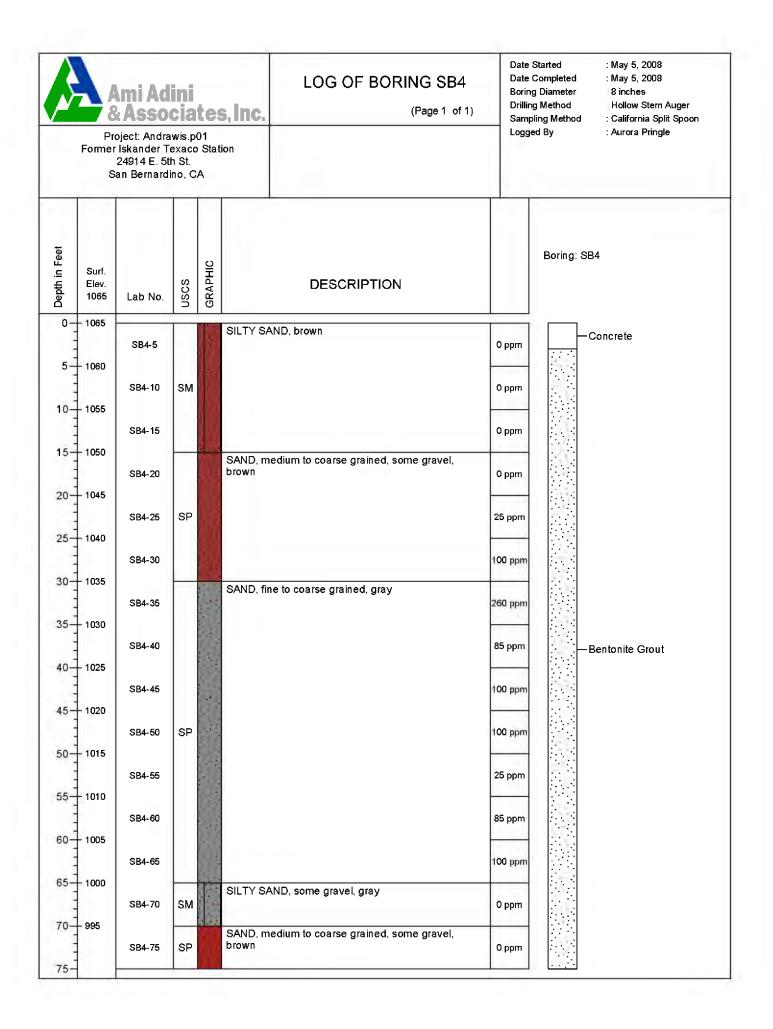
Iskandar Texaco GeoTracker Case I.D. T0607100550

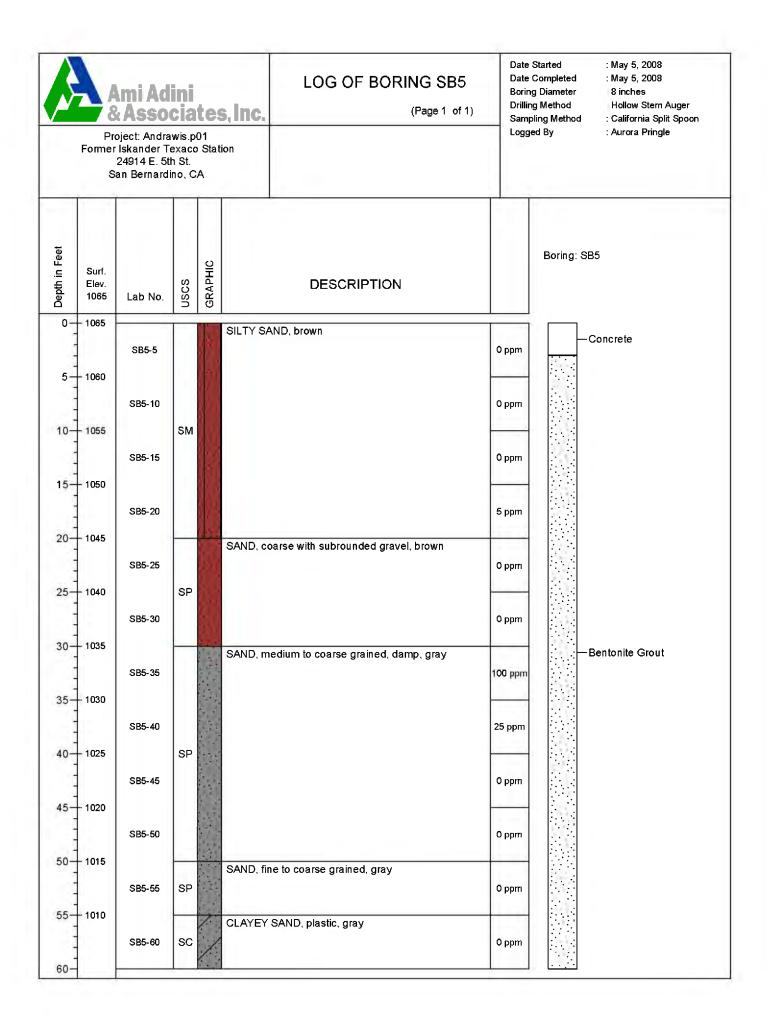


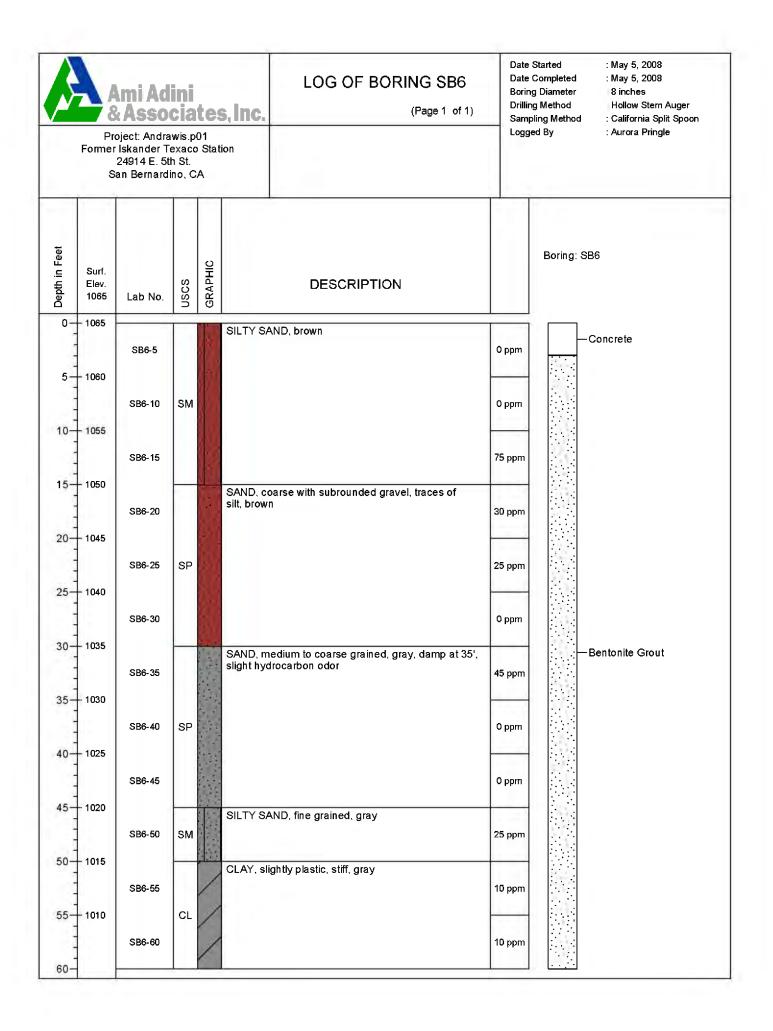


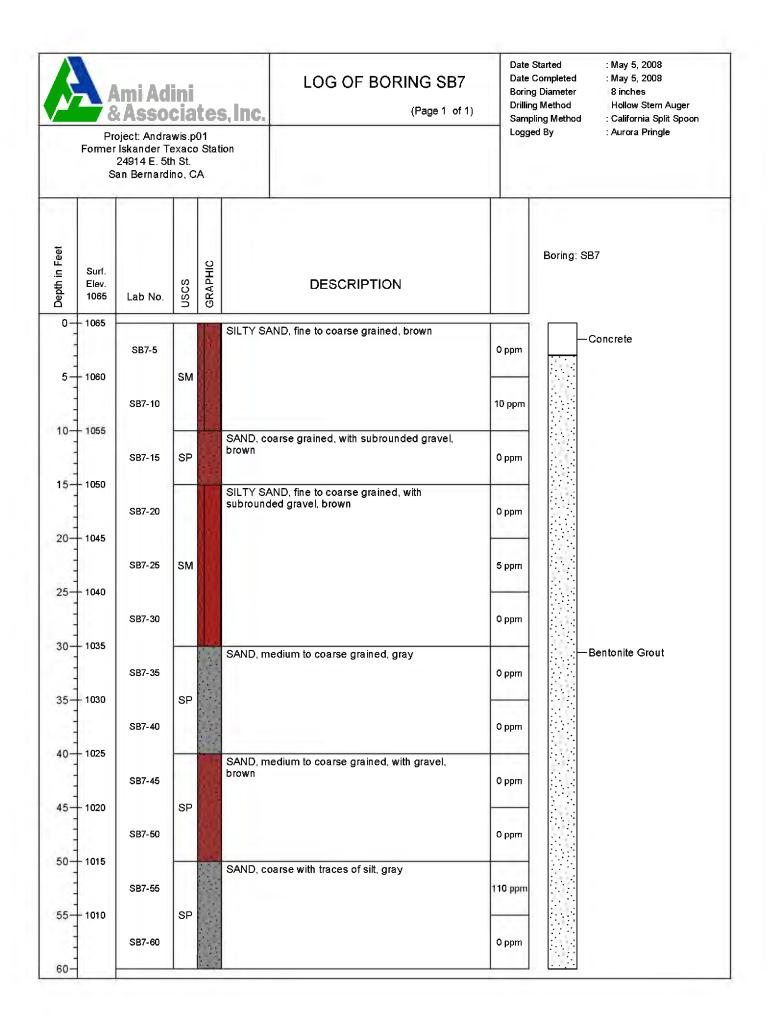


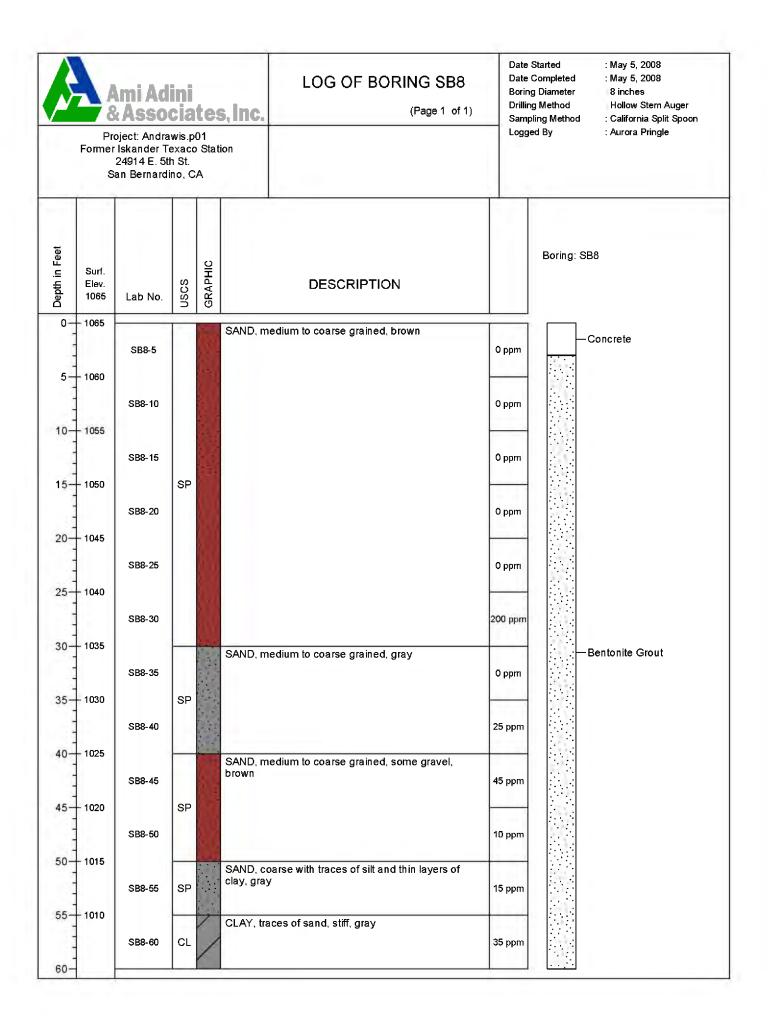








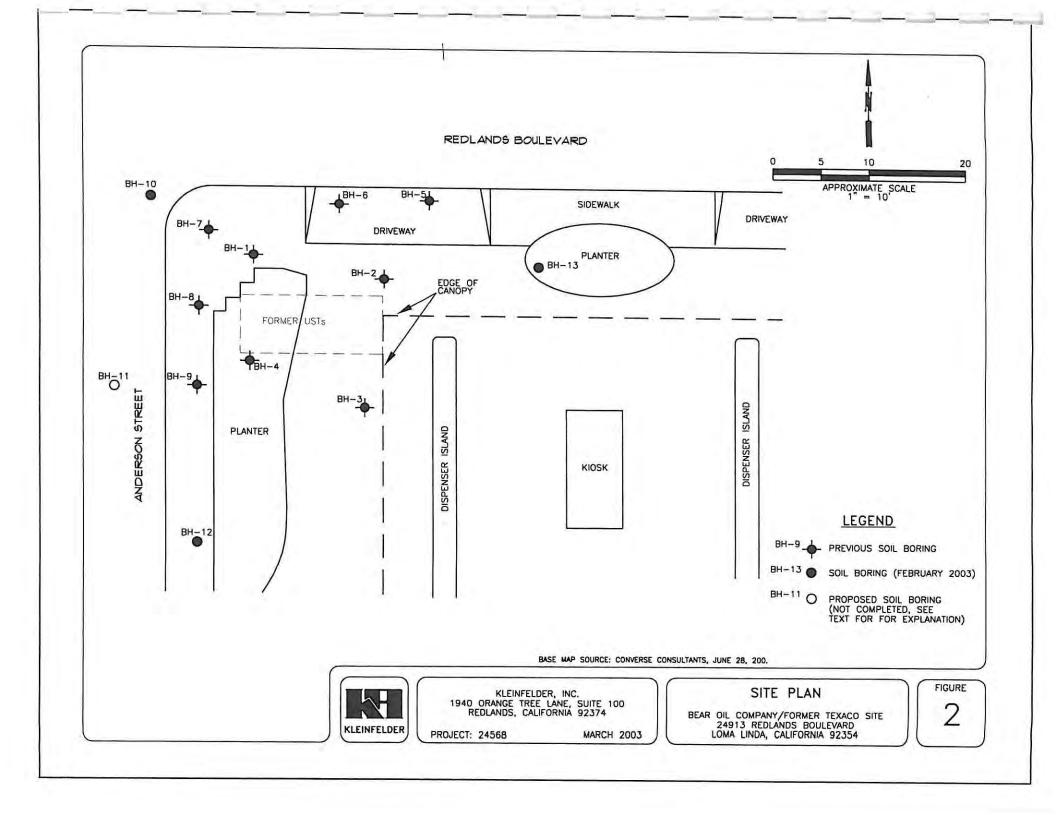






#### **APPENDIX C2.5**

Bear Oil Co./ Former Texaco GeoTracker Case I.D. T0607100598



			Logged by: <u>DM</u> Driving Weight and Drop:						1
		Geoprobe 5400						1	
ound	d Surface	e Elevation: N/A	Depth to Water: <u>not e</u>	ncou	nter	ed		-	
DEPTH (ft)	GRAPHIC LOG	This log is part of the report pr should be read together with th location of the boring and at th may differ at other locations an	UBSURFACE CONDITIONS repared by Converse for this project and re report. This summary applies only at the time of drilling. Subsurface conditions d may change at this location with the ented is a simplification of actual conditions	DRIVE	BULK	BLOWS	MOISTURE (%)	DRY UNIT WT. (psf)	(mqq) UIA
5 -		SANDY CLAY (CL); mo	oîst, grayish green						471 737
5 -			reen D (SP-SM); medium to coarse						274 46.7
5 -		SAND AND SILTT SATA sand, slightly moist, b CLAYEY SAND (SC); fi			•				2404
0		brown	no, gray, moist						2494 263
	<u></u>	End of boring at 34 feet No groundwater encounte Backfilled with Bentonite	red						
-		erse Consultants	Proj	ect No	). ).		Dı	awing N	lo.

Date Drilled: _	10/27/99	Logged by: DM	CH	necke	ed by:		MO	DC
Equipment:	Geoprobe 5400						_	
Fround Surfac	e Elevation: <u>N/A</u>	Depth to Water:	епсои	inter	ed		2	
DEPTH (f+) GRAPHIC LOG	This log is part of the report p should be read together with t location of the boring and at t may differ at other locations as	UBSURFACE CONDITIONS prepared by Converse for this project and the report. This summary applies only at the he time of drilling. Subsurface conditions nd may change at this location with the sented is a simplification of actual condition	<u>ه</u>	BULK	BLOWS	MOISTURE (%)	DRY UNIT WT. (psf)	PID (ppm)
5	green	ne to medium, slightly moist,						1080
10 -	CLAYEY SAND (SC); g	green						1228
15 -	greenish brown							50.2
20 -		lium to coarse sand with gravel,						6
25 -	moist, light brown							66.5
30 -	CLAY (CL); moist, brow	/n						750
	SILTY SAND (SM); mois End of boring at 36 feet		-					
	No groundwater encounter Backfilled with Bentonite	0	oject No					
Conve	rse Consultants		0 Ject No				wing N B-2	0,

Date Dri	illed: _	10/25/99	Logged by: <u>D</u>	M	Ch	ecke	d by:	_	мос	2
Equipme	ent:	Geoprobe 5400	Driving Weight and	Drop:	D	irec	t Pus	h	-	
Ground	Surfac	e Elevation: N/A	Depth to Water:	not en	соц	nter	ed			
DEPTH (ft)	GRAPHIC LOG	This log is part of the report pu should be read together with th location of the boring and at th may differ at other locations ar	JBSURFACE CONDITIC repared by Converse for this proju- te report. This summary applies the time of drilling. Subsurface co d may change at this location w ented is a simplification of actua	ect and only at the onditions ith the	DRIVE	BULK	BLOWS	MOISTURE (%)	DRY UNIT WT. (psf)	(mqq) (IIA
5 5		SAND/SILTY SAND (SP slightly moist, brown		v to		E-strate-strate				
10		CLAY (CL); slightly mo	st, green							
20 -		SILTY SAND (SM); with	gravel, slightly moist, b	rown						
25		CLAYEY SAND (SC); m	oist, greenish brown							
30										
35		SILTY SAND (SM); mois	t, brown							
		End of boring at 36 feet No groundwater encounter Backfilled with Bentonite	red							
-				Projec	t No.		-	Dr	awing No.	

1

99-41-277-01

B-3.

Date Drilled: _	10/27/99	Logged by: DM	Checked by: MOC
Equipment:	Geoprobe 5400	Driving Weight and Drop:	Direct Push
Ground Surface	Elevation: N/A	Depth to Water:	ot encountered

		SUMMARY OF SUBSURFACE CONDITIONS		PLES		(%)	1	1
DEPTH (ft)	GRAPHIC LOG	This log is part of the report prepared by Converse for this project an should be read together with the report. This summary applies only a location of the boring and at the time of drilling. Subsurface condition may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual condi- encountered.	t the ns ய	BULK	BLOWS	MOISTURE (3	DRY UNIT WT. (psf)	(mqq) UI9
5 -		CLAY (CL); slightly moist, brown						6.4
10		CLAYEY SAND (SC); brown						
15		CLAY (CL); moist, greenish						2022
20		SILTY SAND (SM); fine to medium. trace gravel, sligh moist, brown:	ntly					1.4
25		CLAY (CL); moist, brown						43.5
30		SANDY CLAY (CL); moist, brown						26.5
5		SILTY SAND (SM); fine, moist, brown						
		End of boring at 36 feet No groundwater encountered Backfilled with Bentonite						
2	Conve	rse Consultants	Project N			Dra	awing N	0.
2		99	-41-277	-01			B-4	

Date Drilled:	3/30/0	00	Logged by:	WR	_ Cł	necke	ed by:	-	JRZ/	RVH
Equipment:	Geopro	be	Driving Weigh	t and Drop:		N	/A			
iround Surfa	ce Elevation:	<u>N/A</u>	Depth to Wate	not	encou	nter	ed		÷	
DEPTH (f†) GRAPHIC LOG	This log is par should be read location of the may differ at o	t of the report pr l together with th boring and at th other locations an	JBSURFACE CON epared by Converse for t e report. This summary e time of drilling. Subsu d may change at this loca	his project and applies only at the rface conditions ation with the			RS	MOISTURE (%)	DRY UNIT WT. (psf)	(mqq)
LOG DEP	encountered.	e. Ine data prese	ented is a simplification o	l actual conditions	DRIVE	BULK	BLOWS	MOIS	DRY (ps4	PID
5	SANDY CL	AY (CL); sli	ghtly moist, dark gr	ay						0
10 -		D (SM); fine	to medium, slightly	moist, dark	-					0
15	CLAY (CL);	slightly mois	st, dark gray							0
20	SAND (SP);	fine to mediu	ım. slightly moist, g	gray						0
25 -										)
.0	CLAY (CL); End of boring	at 30 feet							¢	
	No groundwat Boring tackfil									
1				Duci	ect No.				wing No.	-

Loma Linda University

99-41-277-02

	3/30/00	Logged by:	WR	_ Ch	ecke	d by:	( <u></u>	JRZ/	RVH
Equipment:	Geoprobe	Driving Weight a	nd Drop:		N	/A			
Fround Surface	e Elevation: <u>N/A</u>	Depth to Water:	not e	ncou	nter	ed	_	-	
DEPTH (f+) GRAPHIC LOG	This log is part of the rep should be read together w location of the boring and may differ at other locatic	F SUBSURFACE CONDI- ort prepared by Converse for this ith the report. This summary app at the time of drilling. Subsurfac ns and may change at this locatio presented is a simplification of ac	project and lies only at the e conditions n with the	DRIVE	BULK	BLOWS	MOISTURE (%)	DRY UNIT WT. (psf)	(mqq) UI
5	CLAY (CL); moist,	dark gray	Ð						7.5
10 -	SANDY CLAY (CL);	slightly moist, dark gray							89.6
15	CLAY (CL); slightly	moist, dark gray							216
.0 -	SAND (SP); fine to m	iedium, slightly moist, gra	у						1.4
5 -	CLAY (CL); slightly ;	noist, reddish brown to gr	аў						8.8
	SILT (ML); slightly m End of boring at 30 fe No groundwater encou Boring backfilled on 3,	et ntered							18.8

Equipi	nent:	Geoprobe	Driving Weight and Drop:	SAMPLES R					
Ground	d Surfa	ce Elevation: <u>N/A</u>	Depth to Water: not e						
	1	SUMMARY OF S	UBSURFACE CONDITIONS	SAM	PLES			1.	T
DEPTH (ft)	GRAPHIC LOG	should be read together with t location of the boring and at the may differ at other locations an	repared by Converse for this project and he report. This summary applies only at the he time of drilling. Subsurface conditions nd may change at this location with the sented is a simplification of actual conditions	DRIVE	BULK	BLOWS	10ISTURE (2	DRY UNIT WT (psf)	
-	////	Hand auger to approxim	ately 3 feet	1			<u> </u>		
- 5 -		CLAY (CL); slightly mo	vist, dark gray						0
10 -		SANDY CLAY (CL); fir dark gray	ne to medium, slightly moist,						99.8
15		CLAY (CL); slightly moi	st. dark gray						2654
20		SAND (SP); fine to media	um. slightly moist, gray						4.8
25 -		CLAY (CL); slightly mois	t, dark brown-gray						15.1
30		SANDY CLAY (CL); sligh End of boring at 30 feet No groundwater encountere Boring backfilled on 3/30/0	ed					c	)
				No.					

Π

Geoprobe	Driving Weight and Drop:		N	I/A			
ce Elevation: <u>N/A</u>						20	
This log is part of the report pu should be read together with the location of the boring and at the may differ at other locations ar	repared by Converse for this project and ne report. This summary applies only at the ne time of drilling. Subsurface conditions nd may change at this location with the	DRIVE	PLES	BLOWS	MOISTURE (%)	DRY UNIT WT. (psf)	(mqq) []]
CLAY (CL); moist, dark	gray			1			8.4
SANDY CLAY (CL); mc	ist, dark gray	012.		12 -			843
CLAY (CL); moist, dark	gray						1729
SAND (SP); fine to mediu	ım, slightly moist, gray			и 4 4			6
CLAY (CL); slightly mois	t, gray			T T			3.3
End of boring at 30 feet No groundwater encountere	ed			1		c	)
	EXAMPLE Elevation: N/A SUMMARY OF SI This log is part of the report pur- should be read together with the location of the boring and at the may differ at other locations are passage of time. The data pressence encountered. CLAY (CL); moist, dark SANDY CLAY (CL); mc CLAY (CL); moist, dark SANDY (CL); moist, dark SANDY (SP); fine to medite SANDY SILT (ML); slight End of boring at 30 feet No groundwater encountered	re Elevation: <u>N/A</u> Depth to Water: <u>not e</u> SUMMARY OF SUBSURFACE CONDITIONS This log is part of the report prepared by Converse for this project and should be read together with the report. This summary applies only at the location of the boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered. CLAY (CL); moist, dark gray SANDY CLAY (CL); moist, dark gray CLAY (CL); moist, dark gray SAND (SP); fine to medium. slightly moist, gray CLAY (CL); slightly moist, gray SANDY SILT (ML); slightly moist, gray	e Elevation: N/A Depth to Water: not encour SUMMARY OF SUBSURFACE CONDITIONS This log is part of the report prepared by Converse for this project and should be read together with the report. This summary applies only at the location of the boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location of actual conditions encountered. CLAY (CL); moist, dark gray SANDY CLAY (CL); moist, dark gray CLAY (CL); moist, dark gray CLAY (CL); moist, dark gray CLAY (CL); slightly moist, gray SAND (SP); fine to medium. slightly moist, gray SANDY SILT (ML); slightly moist, gray SANDY SILT (ML); slightly moist, gray SANDY SILT (ML); slightly moist, gray	ee Elevation:       N/A       Depth to Water:       not encounter         SUMMARY OF SUBSURFACE CONDITIONS       SAMPLES         This log is part of the report prepared by Converse for this project and should be read together with the report. This summary applies only at the flocation of the boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered.       Image: CLAY (CL); moist, dark gray         CLAY (CL); moist, dark gray       Image: CLAY (CL); moist, dark gray       Image: CLAY (CL); moist, dark gray         SANDY CLAY (CL); moist, dark gray       Image: CLAY (CL); moist, dark gray       Image: CLAY (CL); moist, dark gray         CLAY (CL); slightly moist, gray       Image: CLAY (CL); slightly moist, gray       Image: CLAY (CL); slightly moist, gray         SANDY SILT (ML); slightly moist, gray       Image: CLAY (CL); slightly moist, gray       Image: CLAY (CL); slightly moist, gray	ee Elevation: N/A   Depth to Water: not encountered   SUMMARY OF SUBSURFACE CONDITIONS   This log is part of the report prepared by Converse for this project and should be read together with the report. This summary applies only at the level of the boring and at the time of drilling. Subsurface conditions   may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered.   CLAY (CL); moist, dark gray   SANDY CLAY (CL); moist, dark gray   CLAY (CL); moist, dark gray SAND (SP); fine to medium, slightly moist, gray CLAY (CL); slightly moist, gray End of boring at 30 feet No groundwater encountered	e Elevation: N/A   Depth to Water: not encountered   SUMMARY OF SUBSURFACE CONDITIONS   This log is part of the report prepared by Converse for this project and should be read together with the report. This summary applies only at the location of the boring and at the time of drilling. Subsurface conditions   max differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions   cLAY (CL); moist, dark gray   CLAY (CL); moist, dark gray CLAY (CL); moist, dark gray SANDY CLAY (CL); slightly moist, gray CLAY (CL); slightly moist, gray SANDY SILT (ML); slightly moist, gray End of boring at 30 feet No groundwater encountered	e Elevation: N/A   Depth to Water: not encountered   SUMMARY OF SUBSURFACE CONDITIONS   This log is part of the report prepared by Converse for this project and should be read together with the report. This summary applies only is the location of the borning and at the time of drilling. Suburface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions are may change at the location with the passage of time. The data presented is a simplification of actual conditions are may change at the total conditions and may change at the location with the passage of time. The data presented is a simplification of actual conditions are may change at the location with the passage of time. The data presented is a simplification of actual conditions are dependent on the data presented is a simplification of actual conditions are applied at the total presented is a simplification of actual conditions are applied at the total presented at a presented is a simplification of actual conditions are applied at the total presented at a presented a

		Geoprobe	Driving Weight and Drop:		N	N/A		_	
Ground	d Surfa	ce Elevation: <u>N/A</u>	Depth to Water:not	encoi	unter	red		-	
DEPTH (f†)	GRAPHIC LOG	This log is part of the report p should be read together with the location of the boring and at the may differ at other locations ar	UBSURFACE CONDITIONS repared by Converse for this project and he report. This summary applies only at the he time of drilling. Subsurface conditions ad may change at this location with the sented is a simplification of actual condition.		BULK	BLOWS	MOISTURE (%)	DRY UNIT MT. (psf)	PID (pam)
5		CLAY (CL); slightly mo	ist, dark gray						965
10		SILTY SAND (SM); fine	to medium, slightly moist, gray						89.6
15		CLAY (CL); moist, gray							1200
20 -		SAND (SP); fine to mediu	m, slightly moist, gray					C	)
5		SILT (ML); slightly moist,	gray		-			0	
		SANDY SILT (ML); fine, s End of boring at 30 feet No groundwater encountered Boring backfilled on 3/30/0	d					0	
		No groundwater encountered Boring backfilled on 3/30/0	d 0						

DRILLING COMPANY:       Strongarm Environmental       WAT         DRILLING EQUIPMENT:       Geoprobe       LOG         DIAMETER OF BORING:       1-1/2 inches       CHE         E       E       O       S         DESCRIPTION       S       DESCRIPTION       S         C       ASPHALT: ~ 6 inches thick       AGGREGATE BASE:       AGGREGATE BASE:         SILTY SAND: brown, slightly moist, medium dense, fine grained sand with gravel, some odor       BH         S	BH-10		RING N	Б	ar Oil 13 Redlands Blvd, Loma Linda, California		ECT: TION:	
ASPHALT: ~ 6 inches thick	DEPTH: 41.0 feet ENCOUNTERED: 38 ft bgs D BY: Carlos Campos ED BY:	ENCOUNTERE BY: Carlos	TER E	W L	NY: Strongarm Environmental	OMPA QUIPN	ing C Ing E	DRILI DRILI
ASPHALT: ~ 6 inches thick	STUDY SUDY	BLOW COUNTS PID (ppm)	SAMPLE No.	SAMPLE	DESCRIPTION	nscs	BRAPHIC LOG	DEPTH (ft)
groundwater encountered at approximately 38 feet, coarse grained sand	s 265 1789 s 2743 a 1783 a 27 c 27 c 27 1788 1	198 265 1789 1584 2743 1 1783 27	H-10-5 H-10-10 H-10-20 H-10-25 H-10-30 -10-35		AGGREGATE BASE:         SILTY SAND: brown, slightly moist, medium dense, fine grained sand with gravel, some odor         - trace gravel, some odor         - strong odor         - olive-green, slightly moist, loose, fine to medium grained sand, strong odor         - olive-green, wet, loose, fine grained sand, very strong odor, free product         SAND: light brown, slightly moist, loose, fine to medium grained, trace gravel, no odor         CLAY: olive-green to brown, slightly moist, stiff, strong odor         SILTY SAND: olive-green, slightly moist, loose, fine to medium grained, slight odor         SAND: brown, slightly moist, loose, fine to medium grained, slight odor	SP CL SM SP	<u>n n n n n n n n n n n n n n n n n n n </u>	5
40 - gray, saturated, medium dense, very fine grained grading into coarse grained sand with depth Boring terminated at 41 feet. Groundwater not encountered.					gray, saturated, medium dense, very fine grained grading into coarse grained sand with depth Boring terminated at 41 feet.			40

PROJECT:         Bear Oil           LOCATION:         24913 Redlands Blvd, Loma Linda, California           STARTED:         2/17/03						NO:	28 0 fe	r	BH-12				
DRILI DRILI	ling Ling Ietef	Compa Equipm R of Bo	NY: Strongarm Environmental		TOTAL DEPTH: 28.0 feet WATER ENCOUNTERED: 23 ft bgs LOGGED BY: Carlos Campos CHECKED BY:								
DEPTH (ft)	GRAPHIC LOG	USCS	DESCRIPTION	SAMPLE	SAMPLE No.	BLOW	(mqq)	(U)		BOREHOLE ABANDONMENT			
-		SP	SAND: brown, wet, loose, medium grained, with gravel							- Concrete cap.			
5	<u>ਹਿੰਧ ਹੈ ਹੋ ਹੋ ਹੋ ਹੋ ਹੋ</u> <u>ਦ ਪੁੱਚ ਹਾ ਸੂਜ ਸੂਜ</u>	SM	SILTY SAND: brown, slightly moist, dense, very fine grained - saturated, fine to medium grained sand						ներեները ներերությունը։ ԱՄԵՐԻՆԵՐԵՐԵՐԵՐԵՐԵՐԵՐԵՐԵՐԵՐԵՐԵՐԵՐԵՐԵՐԵՐԵՐԵՐ	- Borehole backfilled with hydrated bentonite chip			
-		ML	<ul> <li>slightly moist, very fine grained, no odor</li> <li>SILT: brown, slightly moist, dense, very fine grained</li> </ul>		BH-12-10		0.0		ուսիներեր Արելեներ Արելեներ				
- 15		SP	SAND: brown, slightly moist, dense, medium to coarse grained, no odor		BH-12-15		0.0		ներերերեր ԱՄԱՅԱՅԱՅԱՅ ԱՄԱՅԱՅԱՅԱՅ				
20 -		ML	SILT: brown, slightly moist, dense, very fine grained – no odor		BH-12-20		0.0		երեղերել է եներեր Արհերել է եներեր Արհերել է եներեր				
25-		SP	SAND: brown, saturated, loose, medium to coarse grained, groundwater encountered at approximately 23 feet						ાર્થના કે બેન્સ કે બે બેન્સ બેન્સ કે બેન્સ ક બેન્સ કે બેન્સ	-			
30-			Geoprobe terminated at 28 feet. Groundwater encountered at approximately 23 feet.										
35 - - - 40 -													
5													
1		Kle	einfelder						PRO	DJECT NO. 24568			

KA REDLANDS ENVIRON LOG (NO WELL) 24568 BEAR OIL GPJ LAEWNN04.GDT 3/27/03

Contract 1			4913 Redlands Blvd, Loma Linda, Californ	-				_		BH-13
DIAME	NG NG TEF	COM EQUI R OF E	17/03 COMPLETED: 2/17/03 PANY: Strongarm Environmental PMENT: Geoprobe 30RING: 1-1/2 inches		TOTAL D WATER E LOGGED CHECKE	ENCOL BY:	JNTERE			
0EPTH (ft)	GRAPHIC	USCS	DESCRIPTION	SAMPLE	1	BLOW	(mqq)	(tt) (tt)		BOREHOLE ABANDONMENT
-12	<u>u u u u u u u u u u u u u</u>	b b b b b b S M	SILTY SAND: brown, moist, loose, coarse grained, with gravel no odor black, very moist, fine to medium grained, trace gravel, very strong odor		BH-13-5 BH-13-10		0.0 576.0		ៅក្នុងទៅក្នុងទៅក្នុងទៅក្នុងទៅក្នុងទៅក្នុងទៅក្នុងទៅក្នុងទៀតទៀតក្នុងទៀត ស្នាល់ទៅទៅទៅទៅទៅទៅទៅទៅទៅទៅទៅទៀតទៀតក្នុងទៀតទៀតទៀត ទៅទៅទៅទៅទៅទៅទៅទៅទៅទៅទៅទៅទៅទៀតទៀតទៀតទៀតទៀត ទៅទៅទៅទៅទៅទៅទៅទៅទៅទៅទៅទៅទៀតទៀតទៀតទៀតទៀត	<ul> <li>Borehole backfilled with hydrated bentonite chips</li> </ul>
		ML	SILT: olive-green, moist, loose, very fine grained, slight odor		BH-13-15		21.0		ներին երերերիներ հերերներիներիներ հերերներիներիներ	
20		SP	SAND: light brown, slightly moist, loose, medium to coarse grained, trace gravel, slight odor		BH-13-20		28.7			
25-		ML	SILT: green, moist, loose, very fine grained, some odor		BH-13-25		35.7			
0	State and the	CL	CLAY: brown, moist, firm, trace very fine grained sand, slight odor		BH-13-30		8.3			
			SILTY SAND: brown, moist, loose, fine to medium grained, trace gravel, slight odor	E	3H-13-35		9.4	bet de tde tele tele		
		SM	<ul> <li>groundwater encountered at approximately 38 feet</li> <li>saturated, slight odor</li> </ul>				2.4	कोन-लेन-छोन-छोन-छोन् <u>न</u>		
			Geoprobe terminated at 43 feet. Groundwater encountered at approximately 38 feet.							
		194	einfelder 0 Orange Tree Lane lands, CA 92374						PRO	JECT NO. 24568

KA REDLANDS ENVIRON LOG (NO WELL) 24568 BEAR OIL.GPJ LAEWNN04.GDT 3/27/03

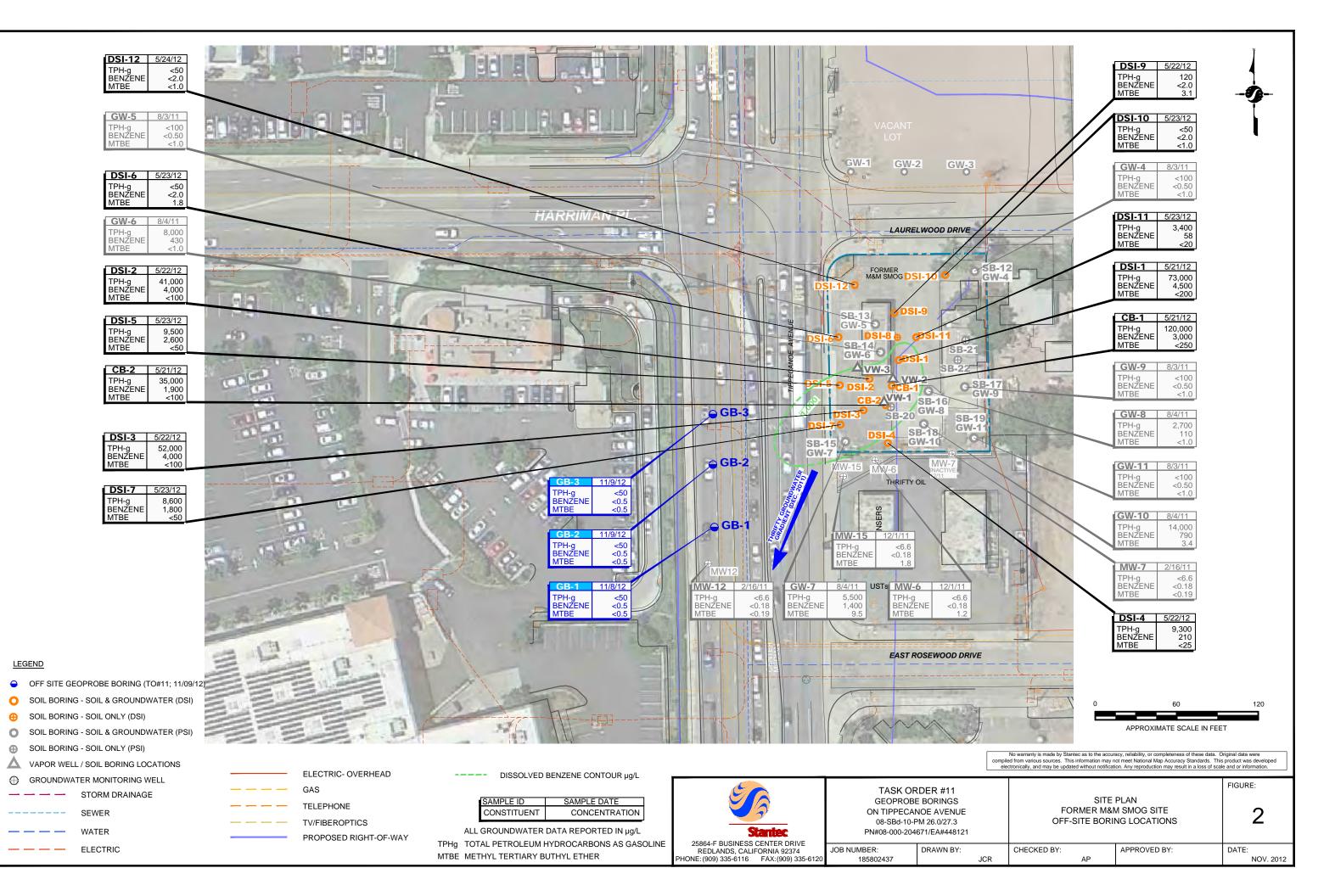
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#### **APPENDIX C2.6**

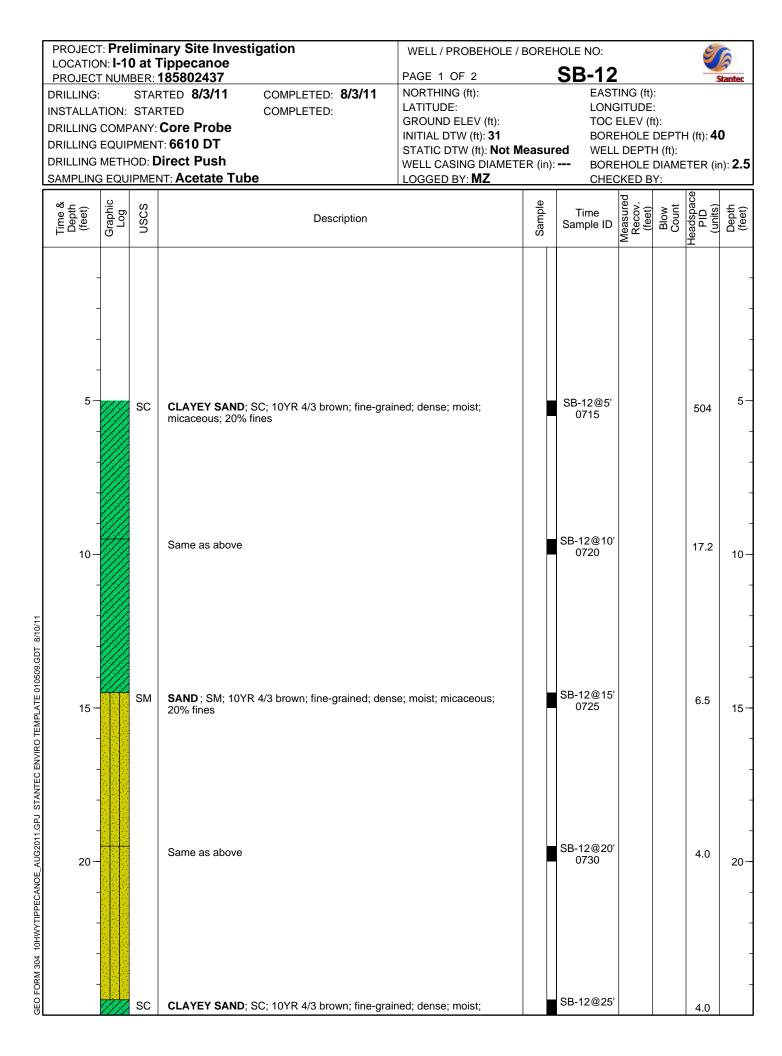
Former M&M Smog and Muffler GeoTracker Case I.D. T10000003588

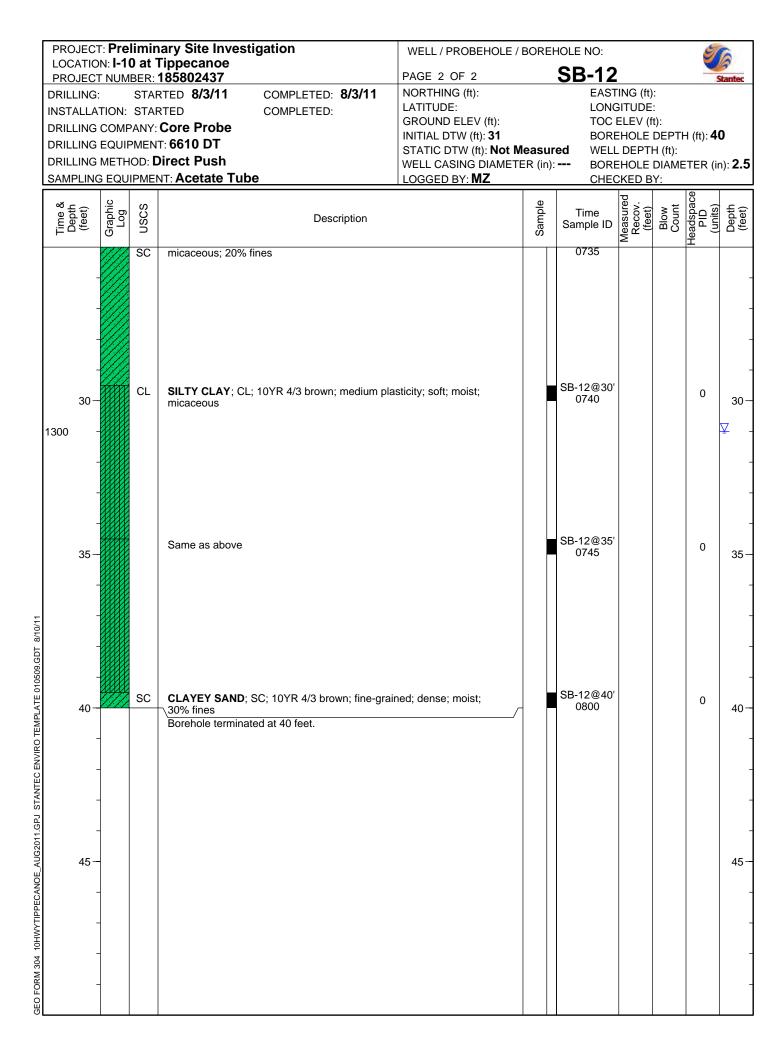


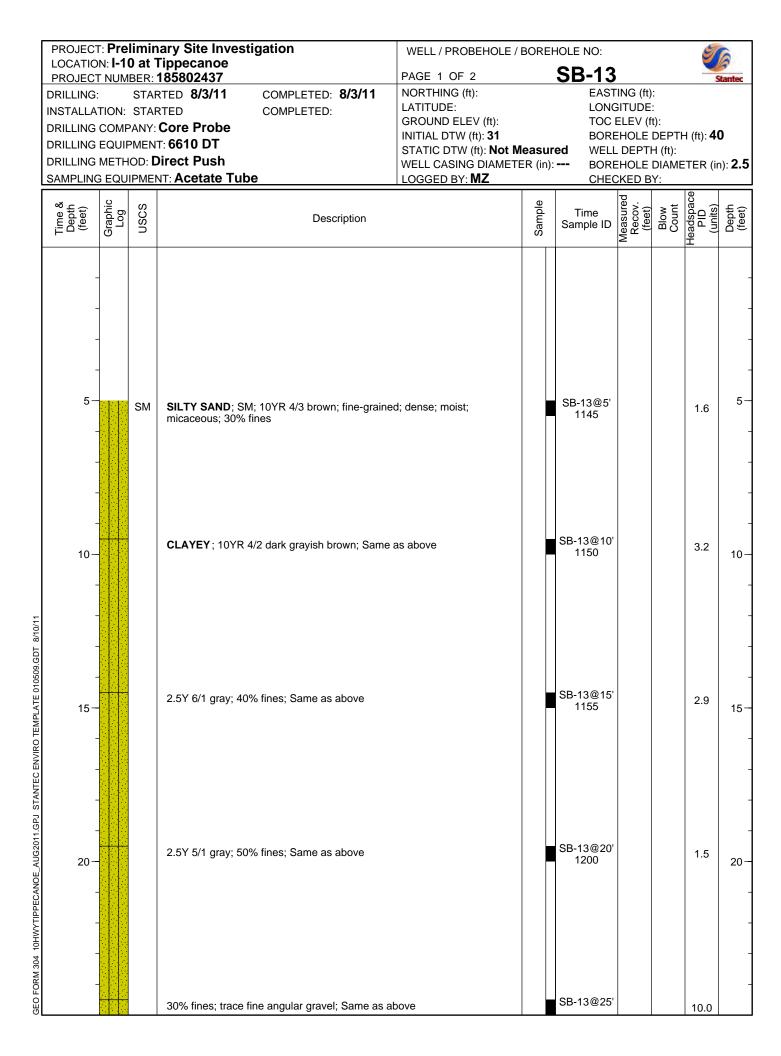
PROJECT: Former LOCATION: San Be PROJECT NUMBER:		WE			Stantec				
INSTALLATION: STA	Core Probe ⊤: 7822DT Track Rig Direct Push	GB-1       PAGE 1 OF 1         NORTHING (ft):       EASTING (ft):         LATITUDE:       LONGITUDE:         GROUND ELEV (ft):       TOC ELEV (ft):         INITIAL DTW (ft): 35       11/8/12         BOREHOLE DEPTH (ft):       NOT Encountered         WELL CASING DIAMETER (in): 1       BOREHOLE DIAMET         LOGGED BY:       MFB							t):
Time & Depth (feet) Graphic Log USCS	Description	Sample	Time Sample ID Method	Measured Recov. (feet)	Blow Count	Headspace PID (units)	Depth (feet)		Well Constructior
	8" Asphalt 8" Concrete/Lime Treated Base Hand Auger to -5' Below Ground Surface (bgs)					<u> </u>	-		<ul> <li>4" Concrete Cap</li> <li>(Blackened Granular Bentonite (Dry)</li> </ul>
5	Direct Push to 25' bgs						5		
10							10— - -		
- 15 - -							- 15— -		
20-							20-		← Bentonite/ Cement
25CL-	CLAY; CL; GLEY1 3/1 very dark greenish gray; medium to high plasticity; moist; no odor; no staining; ~10% silt, trace very fine		2309 GB-1@25 8260 BTEX OXYS TPHg	0.5		0.0	- - 25 -		Slurry
30- - - - -			2310 GB-1@30 8260 BTEX OXYS TPHg	0.5		0.0	- - 30- -		
35-	SAME AS ABOVE ; GLEY1 5/1 grayish gray; fine to coarse-grained; moist; no odor; no staining; trace silt		2320 GB-1@35 8260 BTEX OXYS TPHg	0.5		0.0	- ⊻35- -		
2326 40 <u>SP</u>	POORLY GRADED SAND ; SP; 10YR 4/1 dark gray; fine to medium-grained; moist; no odor; no staining; trace coarse grained sand; ~20% silt Borehole terminated at 40 feet.		2326 GB-1@40 8260 BTEX OXYS TPHg	0.5		0.0	40		

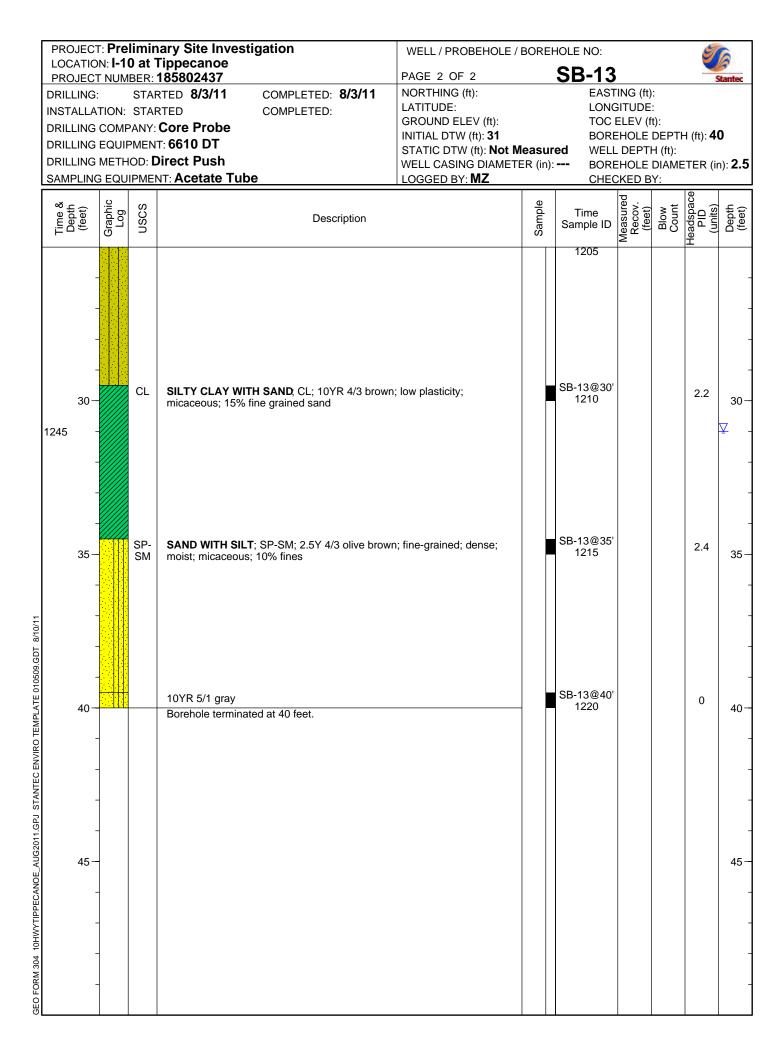
	er M&M Smog - Tippecanoe Bernardino, CA ∈R: 185802437	WEI			Stantec					
ORILLING: S NSTALLATION: S ORILLING COMPA	TARTED 11/8/12 COMPLETED: 11/9/12 TARTED COMPLETED: NY: Core Probe MENT: 7822DT Track Rig D: Direct Push	LATITUDE:LONGIGROUND ELEV (ft):TOC EIINITIAL DTW (ft): <b>34</b> 11/9/12STATIC DTW (ft): <b>Not Encountered</b> WELL CASING DIAMETER (in): <b>1</b> LOGGED BY: <b>MFB</b> CHECK						TING (ft): GITUDE: ELEV (ft): EHOLE DEPTH (ft): <b>40.0</b> L DEPTH (ft): EHOLE DIAMETER (in): <b>2</b> CKED BY:		
Time & Depth (feet) Graphic Log	Description	Sample	Time Sample ID Method	Measured Recov. (feet)	Blow Count	Headspace PID (units)	Depth (feet)	Well Constructior		
	8" Asphalt 8" Concrete/Lime Treated Base Hand Auger to -5' Below Ground Surface (bgs)							4" Concrete Cap (Blackened Granular Bentonite (Dry)		
5	Direct Push to 25' bgs						5			
10							10			
15 - - -							- 15- - - -			
20-							- 20- - - -	<ul> <li>Bentonite/ Cement Slurry</li> </ul>		
25	CL CLAY; CL; GLEY1 3/1 very dark grayish gray; medium to high plasticity; moist; no odor; no staining; ~10% silt; trace very fine		0019 GB-2@25 8260 BTEX OXYS TPHg	0.5		0.0	- 25— - -			
30-	POORLY GRADED SAND ; SP; 10YR 4/1 dark gray; fine to coarse-grained; slightly moist; no odor; no staining; ~10% fine gravel; trace silt		0020 GB-2@30 8260 BTEX OXYS TPHg	0.5		0.0	30			
35- - - - -	SP <b>POORLY GRADED SAND</b> ; SP; GLEY1 4/1 dark greenish gray; fine to medium-grained; moist; no odor; no staining; trace coarse grained sand; ~10% silt		0025 GB-2@35 8260 BTEX OXYS TPHg	0.5		0.0	⊻ _ 35− - - -			
40	POORLY GRADED SAND ; SP; 10YR 4/1 dark gray; fine-grained; wet; no odor; no staining; ~30% medium grained sand; trace coarse grained sand; trace silt Borehole terminated at 40 feet.		0029 GB-2@40 8260 BTEX OXYS TPHg	0.5		0.0	40			

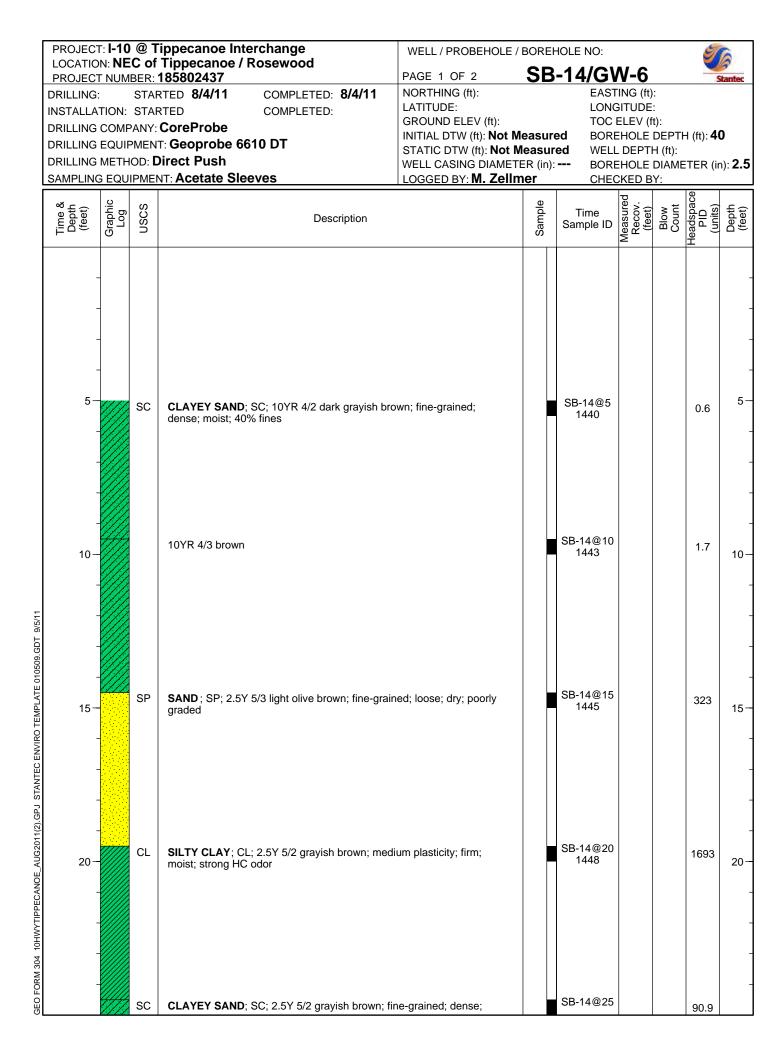
LOCATIO	N: <b>Sa</b>	n Bei	M&M Smog - Tippecanoe mardino, CA 185802437	WE	ILL / PROBEF		Stantec				
DRILLING: INSTALLA DRILLING DRILLING	TION: COMF EQUIF METH	STAF STAF PANY: PMEN OD: <b>D</b>	RTED       11/9/12       COMPLETED:       11/9/12         RTED       COMPLETED:       COMPLETED:         Core Probe       T: 7822DT Track Rig         Direct Push       Direct Push	LATI GRC INIT STA WEL	RTHING (ft): ITUDE: DUND ELEV ( IAL DTW (ft):	TING (ft): IGITUDE: ELEV (ft): EHOLE DEPTH (ft): <b>40.0</b> LL DEPTH (ft): REHOLE DIAMETER (in): <b>2</b> ECKED BY:					
Time & Depth (feet)	Graphic Log	nscs	Description	Sample	Time Sample ID Method	Measured Recov. (feet)	Blow Count	Headspace PID (units)	Depth (feet)		Well Construction
- - - 5-			8" Asphalt 8" Concrete/Lime Treated Base Hand Auger to -5' Below Ground Surface (bgs)					<u> </u>			Cap Cap (Blackened Granular Bentonite (Dry)
- - - 10-	-		Direct Push to 25' bgs						- - - - 10-		
- - - 15	-								- - - 15-		
- - 20 -	-								20-		■ Bentonite/ Cement
- - 25 - -		-cl-	CLAY; CL; 10YR 4/2 dark grayish brown; medium plasticity; slightly moist to moist; no odor; no staining; ~10% silt; trace very fine sand		0142 GB-3@25 8260 BTEX OXYS TPHg	0.5		0.0	25-		Slurry
- - 30 - -		SP	<b>POORLY GRADED SAND</b> ; SP; 10YR 5/3 brown; fine to medium-grained; slightly moist; no odor; no staining; trace coarse grained sand; trace silt		0143 GB-3@30 8260 BTEX OXYS TPHg	0.5		0.0	30-		
- 35- -		SP	<b>POORLY GRADED SAND</b> ; SP; 10YR 4/2 dark grayish brown; fine to medium-grained; wet; no odor; no staining; ~15% silt		0151 GB-3@35 8260 BTEX OXYS TPHg	0.5		0.0	⊻ _ 35− -		
- - 40 -		SP	<b>POORLY GRADED SAND WITH SILT</b> ; SP; 10YR 4/1 dark gray; fine to medium-grained; moist to wet; no odor; no staining; ~20% silt Borehole terminated at 40 feet.		0155 GB-3@40 8260 BTEX OXYS TPHg	0.5		0.0	40-		

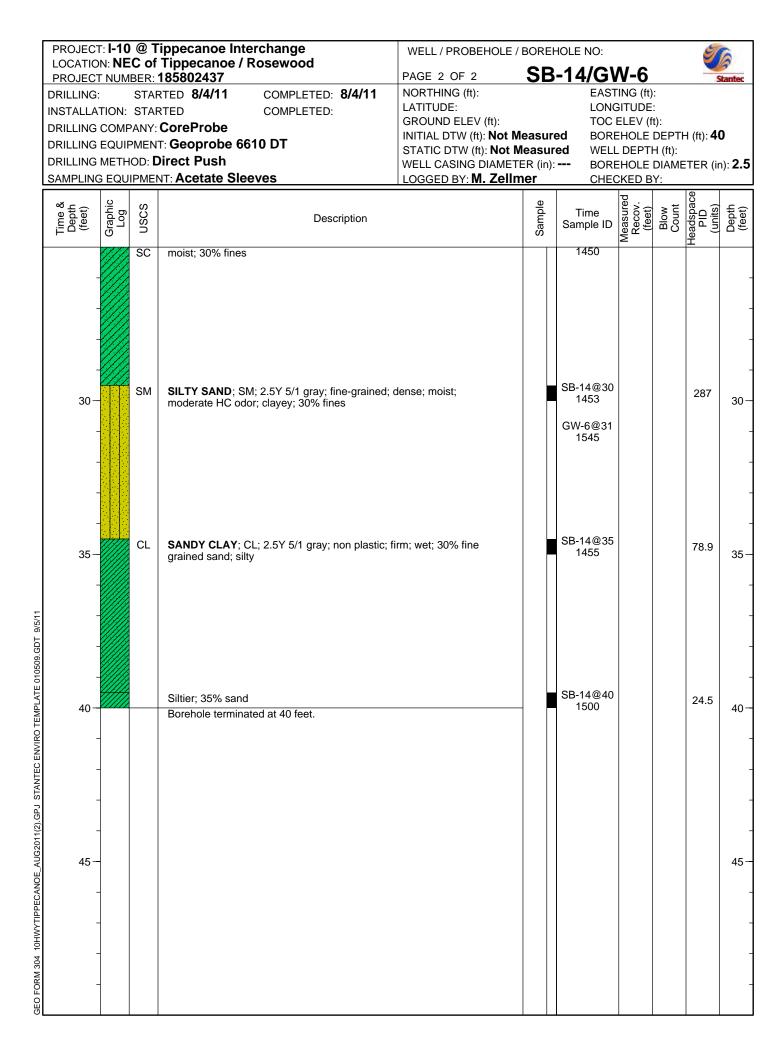


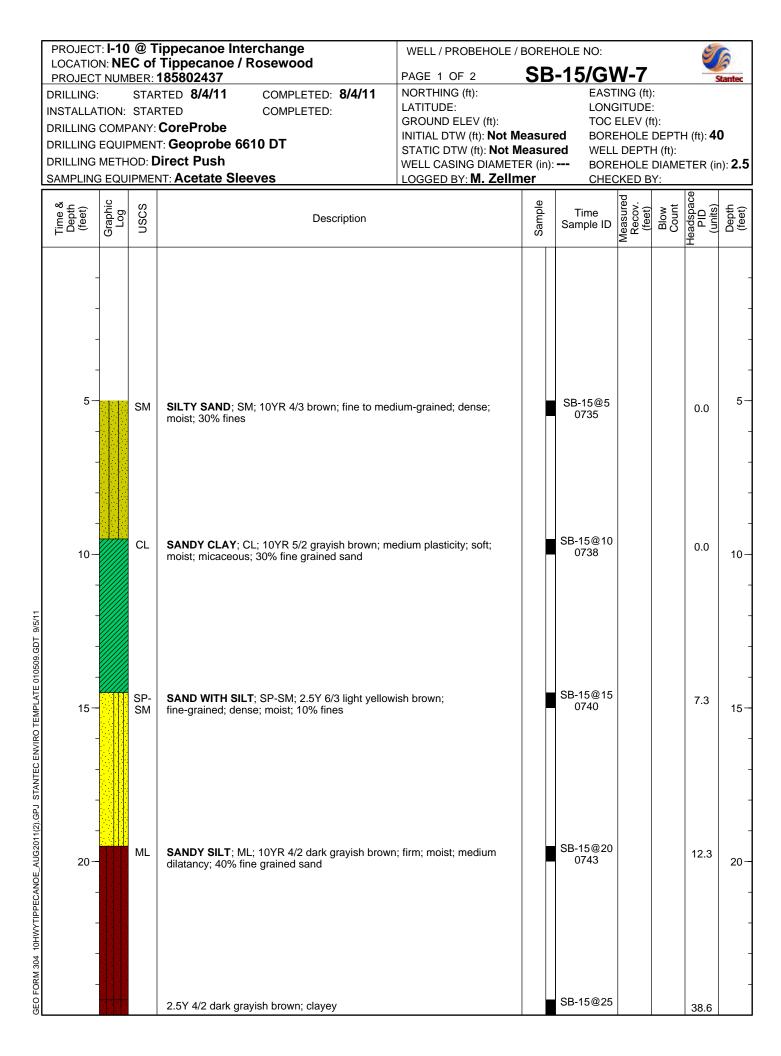


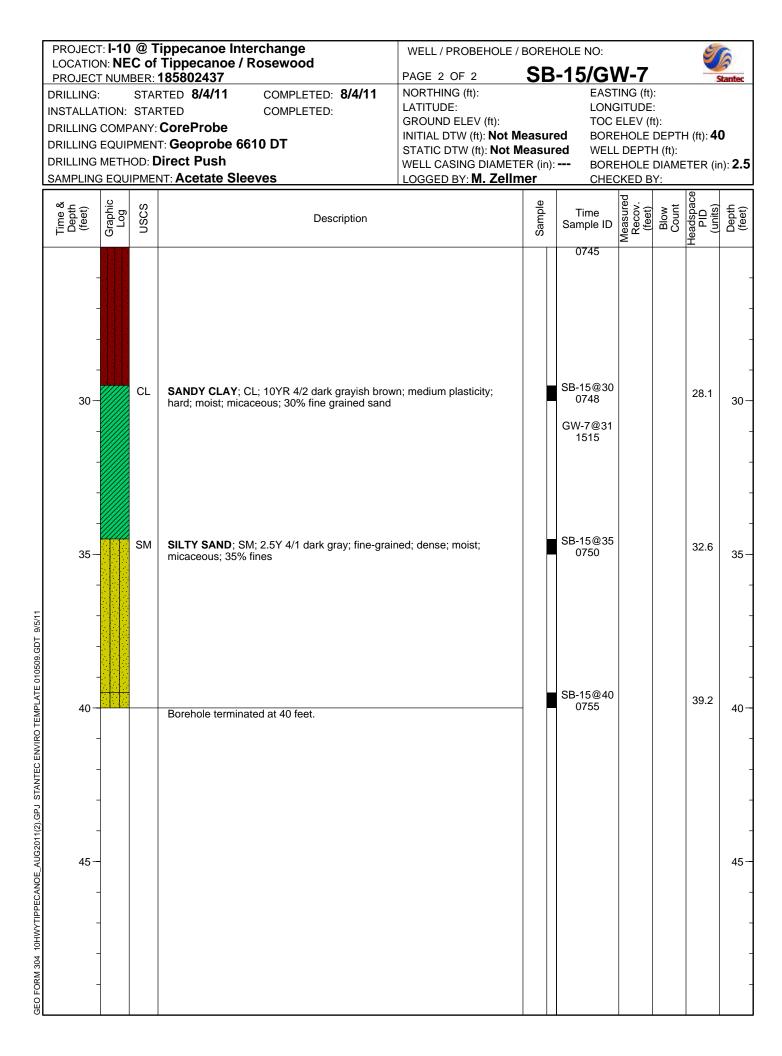


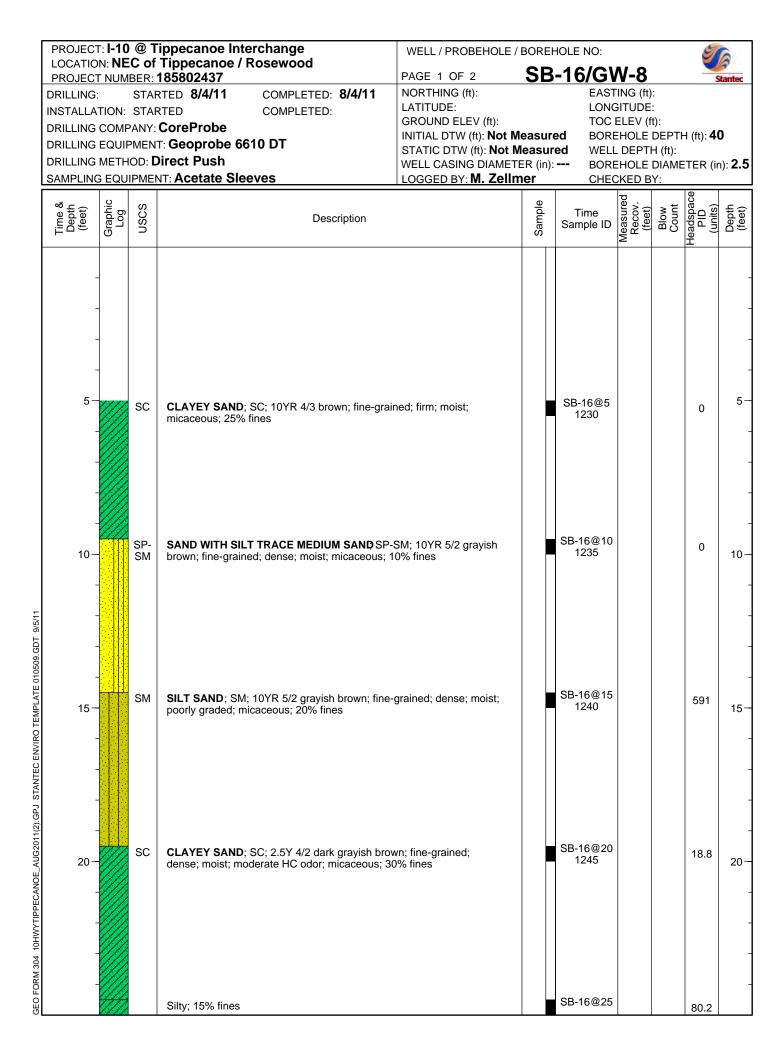


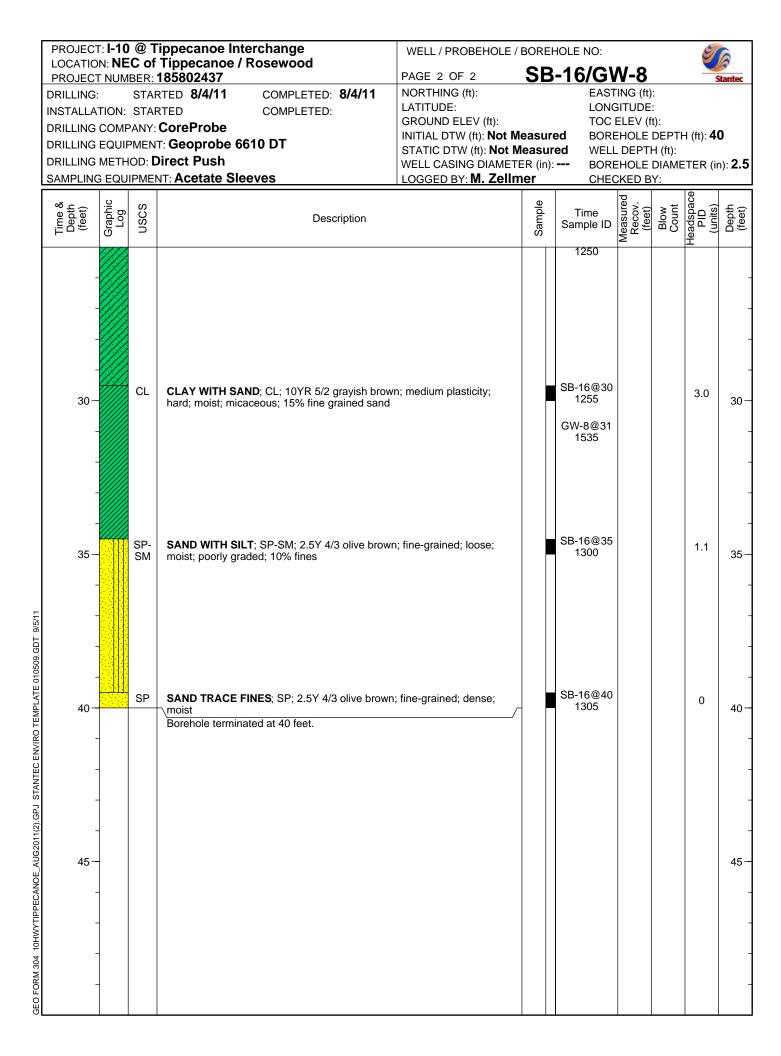


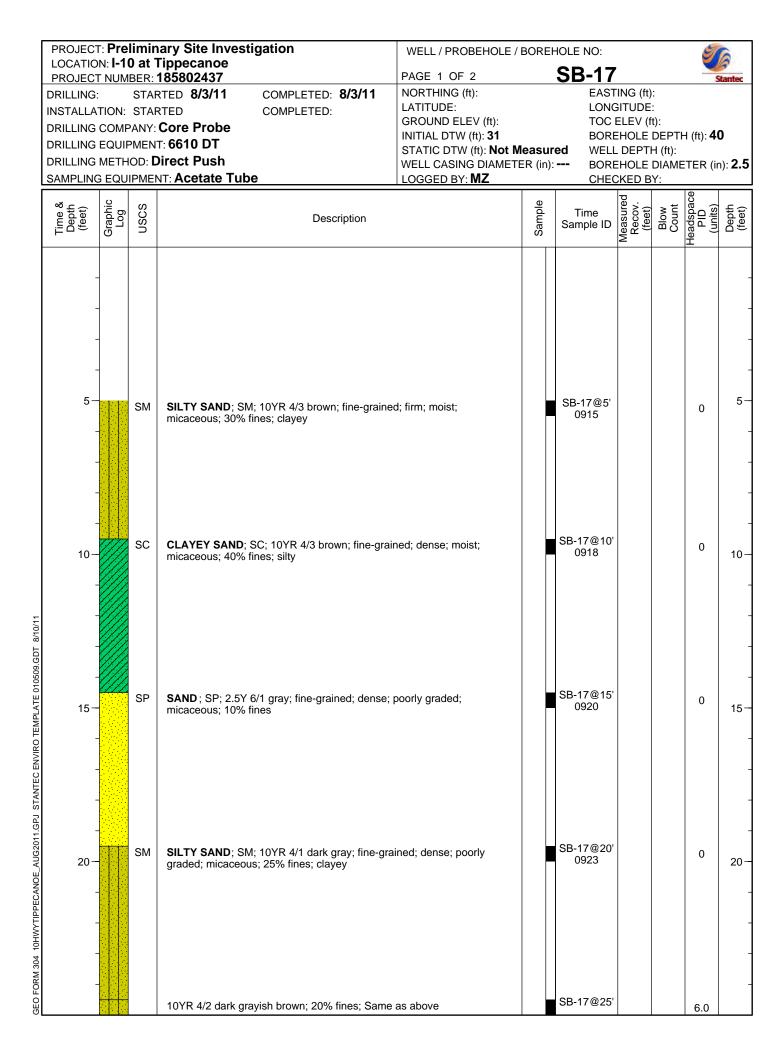


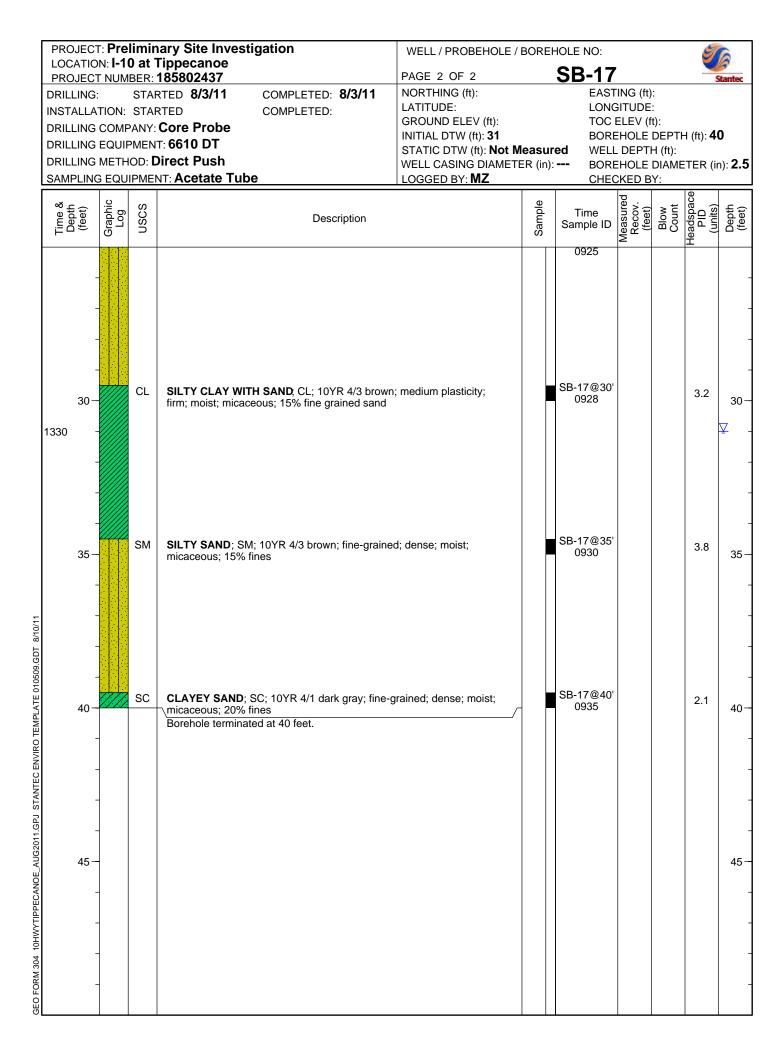




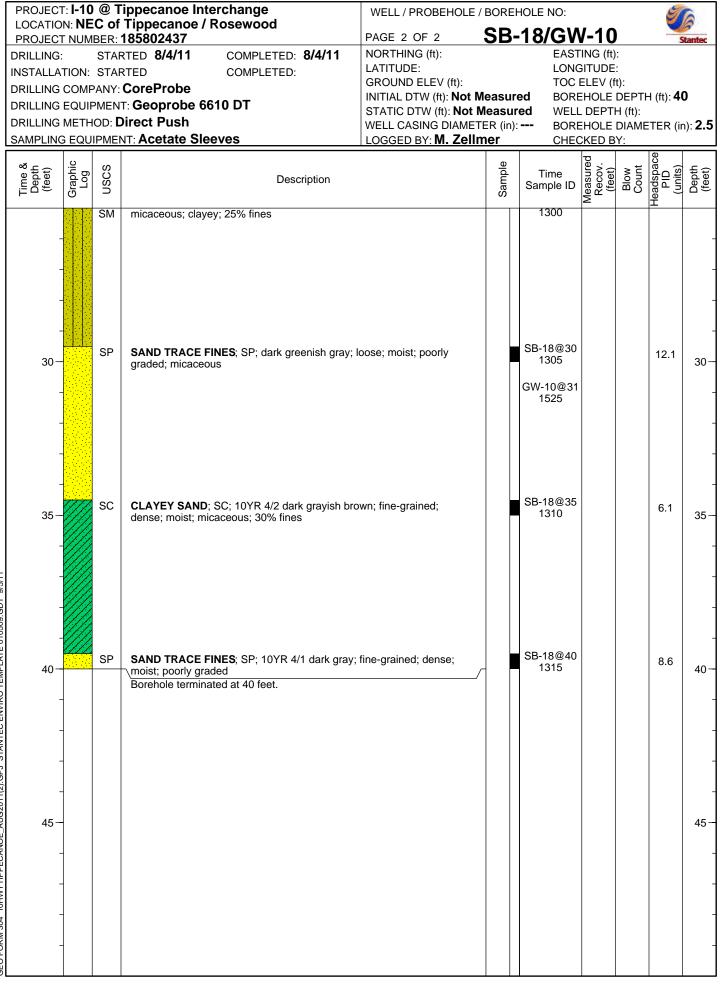




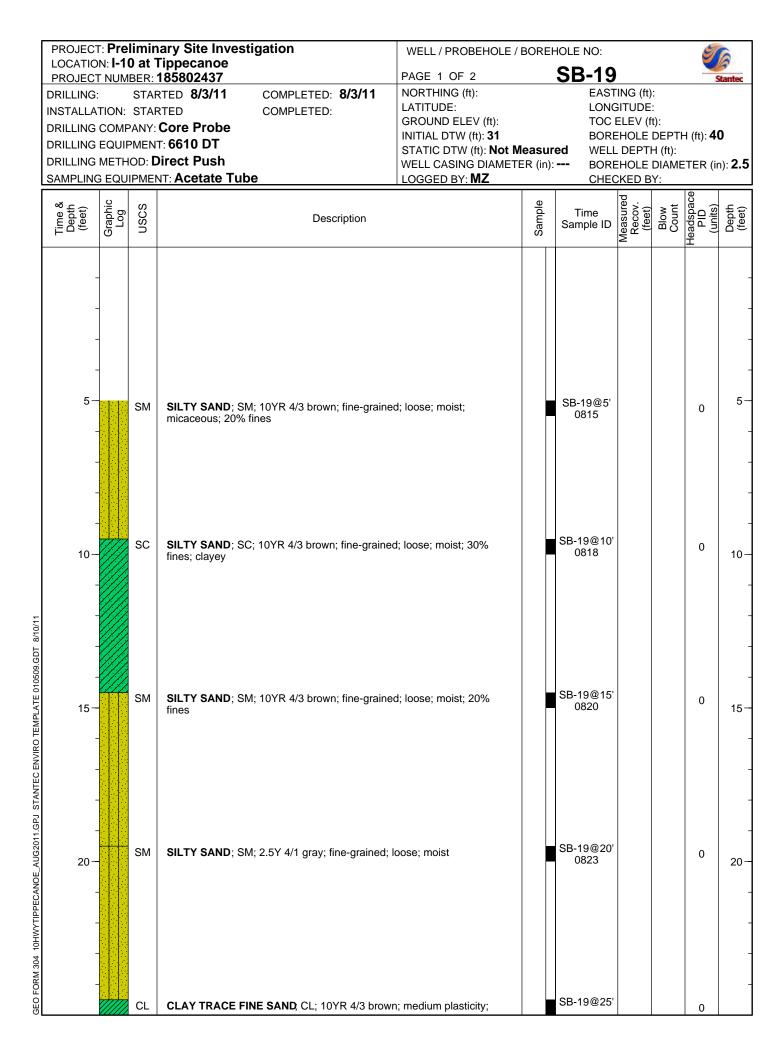


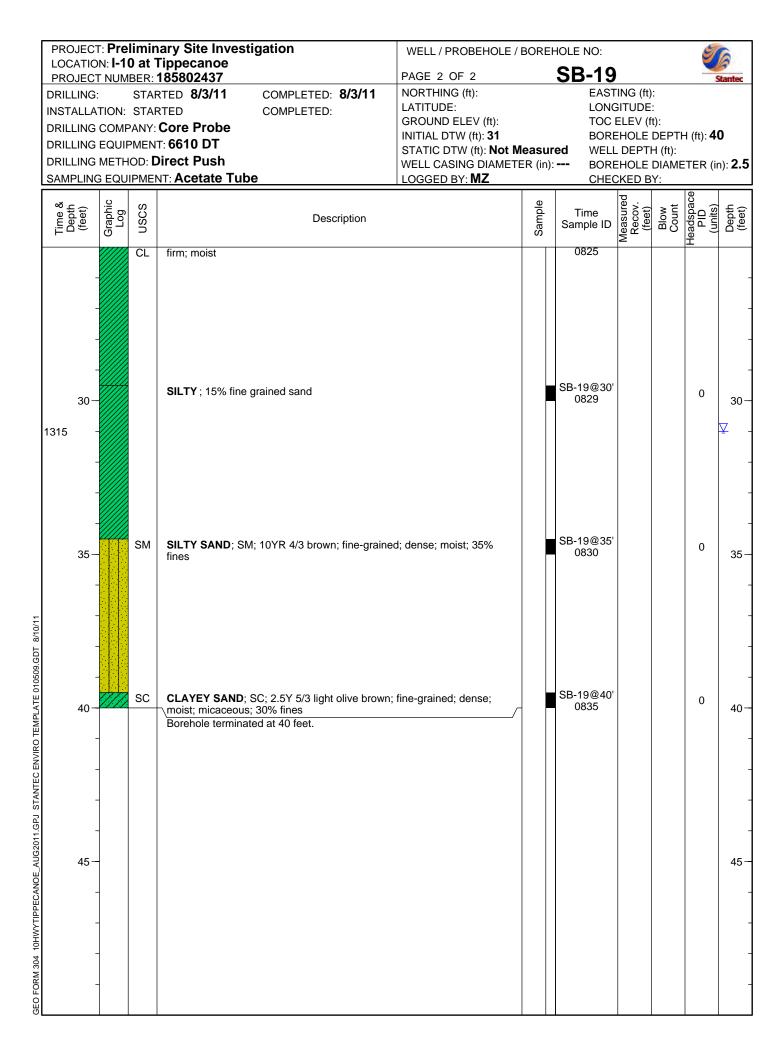


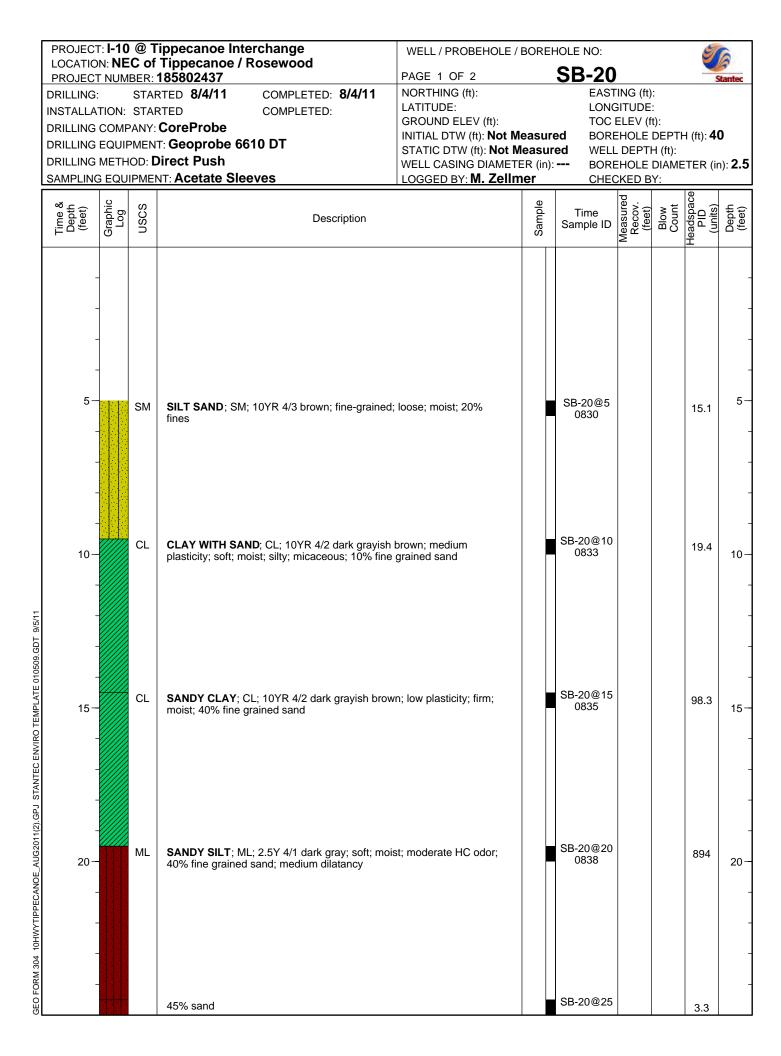
LOCATION: PROJECT N DRILLING: INSTALLATIC DRILLING CC DRILLING EC DRILLING ME SAMPLING E	NEC of UMBER: STA DN: STA DMPANY: QUIPMEN THOD: I QUIPME	Tippecanoe Interchange         Tippecanoe / Rosewood         185802437         RTED       8/4/11         COMPLETED:       8/4/11         RTED       COMPLETED:         • CoreProbe         T: Geoprobe 6610 DT         Direct Push         NT: Acetate Sleeves	WELL / PROBEHOLE / PAGE 1 OF 2 NORTHING (ft): LATITUDE: GROUND ELEV (ft): INITIAL DTW (ft): <b>Not M</b> STATIC DTW (ft): <b>Not M</b> WELL CASING DIAMET LOGGED BY: <b>M. Zellr</b>	DEPT HOLE	G (ft): UDE: EV (ft): DLE DEPTH (ft): <b>40</b> EPTH (ft): DLE DIAMETER (in): <b>2.</b> : ED BY:				
Time & Depth (feet) Graphic	Log USCS	Description		Sample	Time Sample ID	Measured Recov. (feet)	Blow Count	Headspace PID (units)	Depth (feet)
	SC SW SW	CLAYEY SAND; SC; 10YR 4/3 brown SAND ; SW; white; hard; dry; angular; well grade SAND ; SW; 10YR 3/2 very dark grayish brown; well graded			SB-18@5 1240 SB-18@10 1245			<u>т</u> 17.9 10.8	
TANTEC ENVIRO TEMPLATE 010509.GDT	SP	<b>SAND TRACE FINES</b> ; SP; 10YR 4/1 dark gray; poorly graded; micaceous	fine-grained; dense;		SB-18@15 1250			3.3	- - 15 -
GEO FORM 304 10HWYTIPPECANOE_AUG2011(2).GPJ STANTEC ENVIRO TEMPLATE 010509.GDT	SP- SM	SAND WITH SILT; SP-SM; 2.5Y 5/1 gray; fine-g poorly graded; 10% fines	grained; firm; moist;		SB-18@20 1255			6.6	- 20 - -
GEO FORM 30	SM	SILTY SAND; SM; 2.5Y 5/2 grayish brown; fine-	grained; dense;		SB-18@25			32.4	-

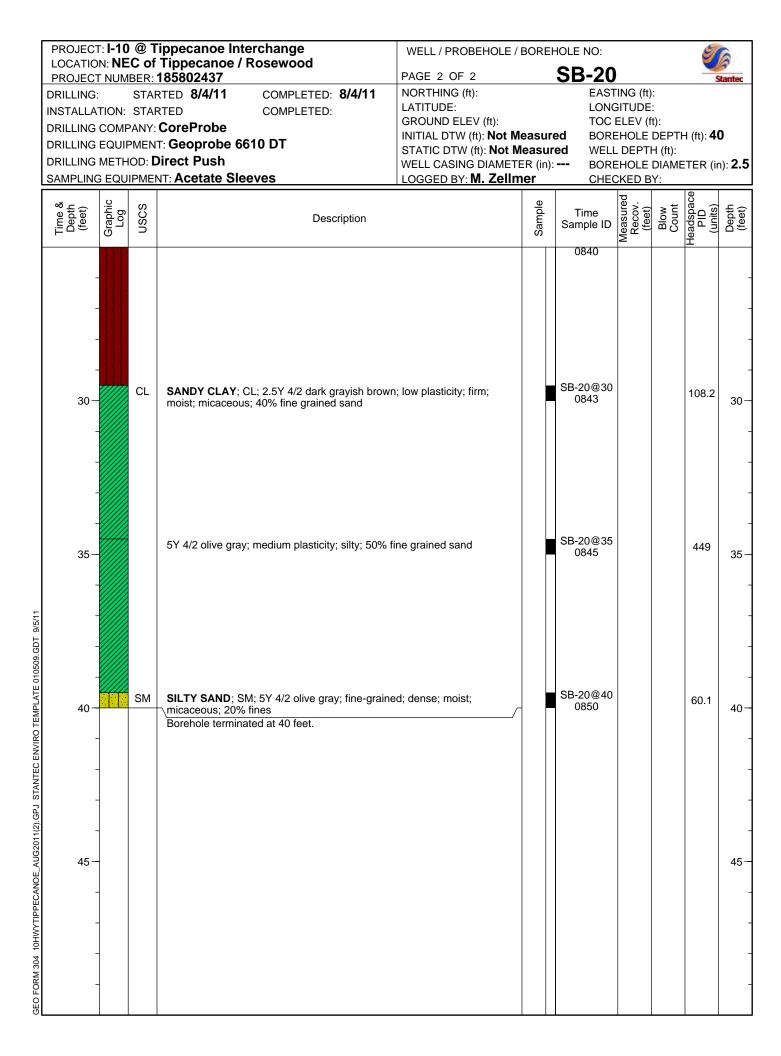


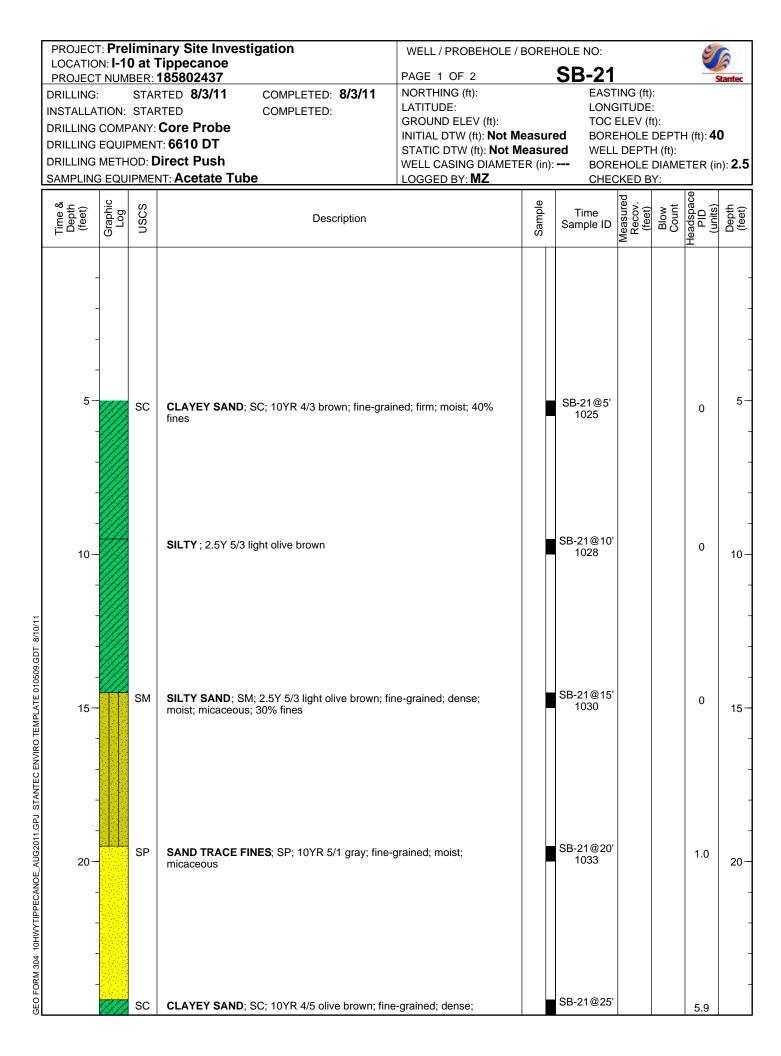
GEO FORM 304 10HWYTIPPECANOE\_AUG2011(2).GPJ STANTEC ENVIRO TEMPLATE 010509.GDT 9/5/11

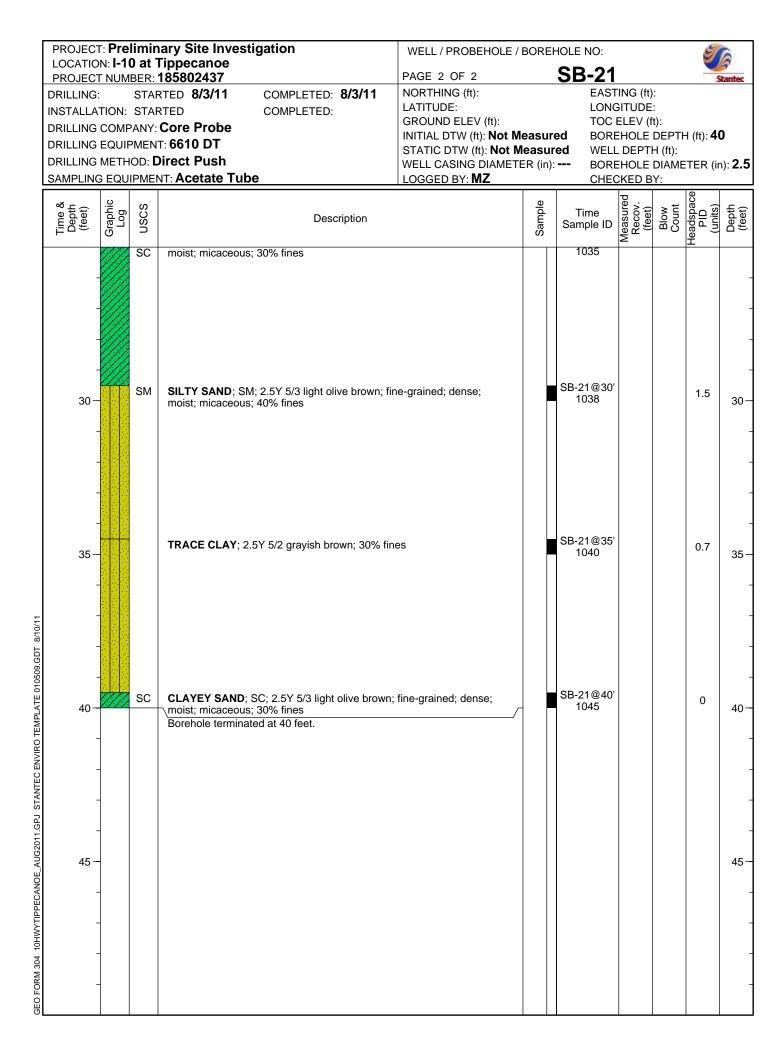


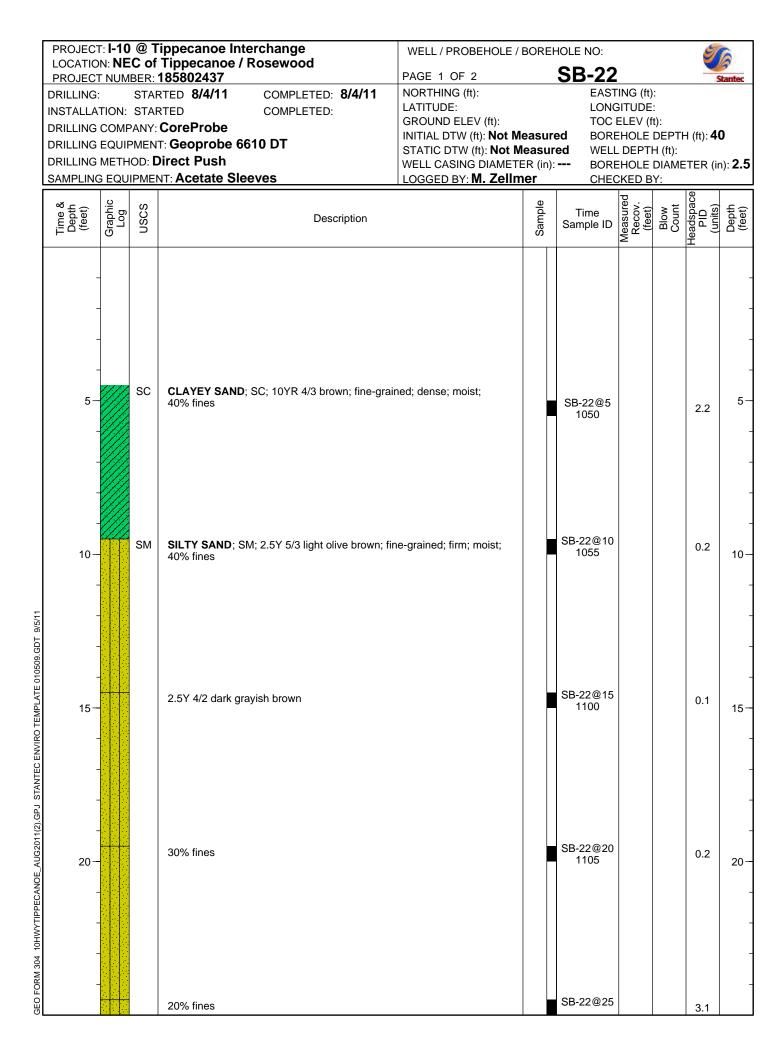


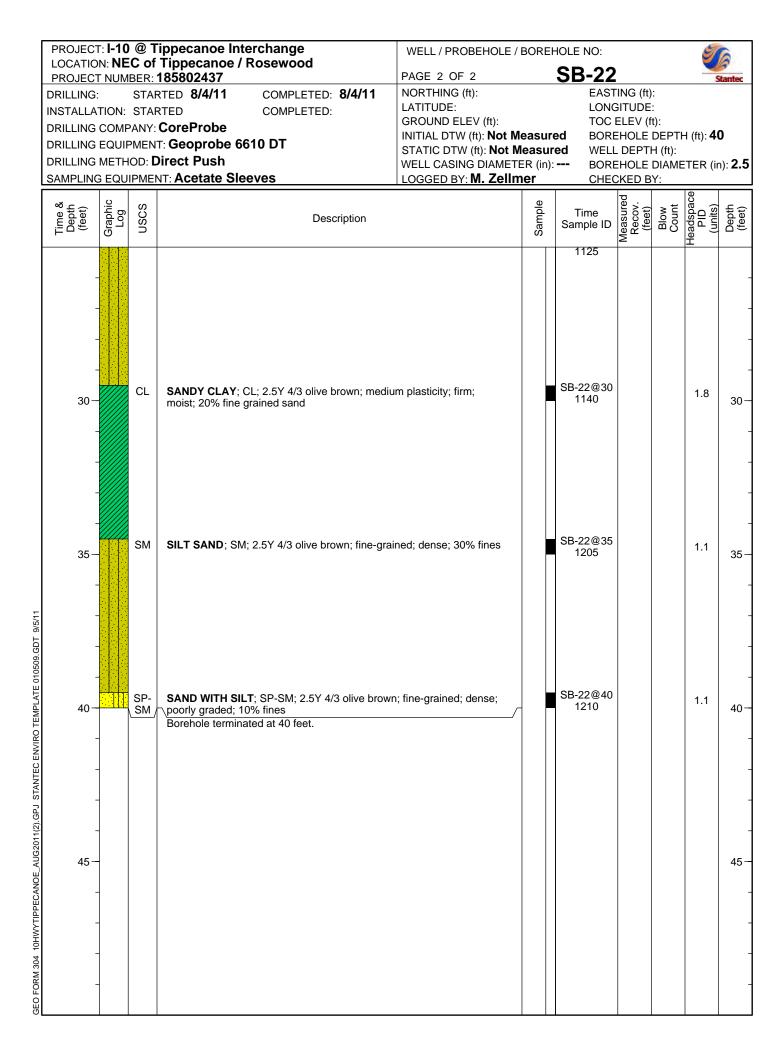












	LOCATIO PROJECT DRILLING	n: <b>M8</b> <u>f num</u> 3:	M Sn BER: STAR	Decanoe           nog           TED: 12/28/11           COMPLETED: 12/28/11           TED: 12/28/11           COMPLETED: 12/28/11	WELL / PROBEHOLE / BOREHOLE IDENTIFICATION: PAGE 1 OF 2 NORTHING (ft): LAT: LAT: LONG:									
	DRILLING DRILLING DRILLING SAMPLIN	B EQUI B METH	PMEN HOD: <b>H</b>	⊤: <b>B-61</b>	GROUND ELEV (ft):       TOC ELEV (ft):         INITIAL DTW (ft): Not Encountered       WELL DEPTH (ft): 30         STATIC DTW (ft): Not Encountered       BOREHOLE DEPTH (ft): 10         WELL CASING DIA. (in): 4       BOREHOLE DIA. (in): 10         LOGGED BY: ME       CHECKED BY:									H (ft): <b>35.5</b>
	Time & Depth (feet)	Graphic Log	NSCS	Description	Sample		Time Sample ID	Measured Recov. (feet) Blow		Headspace PID (units)	Depth (feet)	Well Construction		
GEO FORM 304_10HWYTIPPECANOE_DEC2011.GPJ_STANTEC ENVIRO TEMPLATE 010509.GDT_1/4/12	- - - - - - - - - - - - - - - - - - -		SM	SILT SAND ; SM; 10YR 4/3 brown; fine-grained; loose; moist; 20% fines CLAY WITH SAND ; CL; 10YR 4/2 dark grayish brown; medium plasticity; soft; moist; silty; micaceous; 10% fine grained sand SANDY CLAY ; CL; 10YR 4/2 dark grayish brown; low plasticity; firm; moist; 40% fine grained sand							5			Grout
STANTEC ENVIRO TE	-												-	5/8 Hydrated Bentonite Chips
PECANOE_DEC2011.GPJ	- 20— -		ML	<b>SANDY SILT</b> ; ML; 2.5Y 4/1 dark gray; soft; moist; moderate HC odor; 40% fine grained sand; medium dilatancy							20-			#3 Sand
EO FORM 304 10HWYTIP	-			45% sand							- - - -			4" Sch 40 PVC 0.020" Slotted Screen

LOCATIO PROJEC DRILLIN	ON: <b>M&amp;</b> CT NUM G:	<b>&amp;M Sn</b> I <u>BER:</u> STAR	ppecanoe           nog           TED: 12/28/11           COMPLETED: 12/28/11           COMPLETED: 12/28/11	WELL / PROBEHOLE / BOREHOLE IDENTIFICATION: SVE-1 PAGE 2 OF 2 NORTHING (ft): EASTING (ft): LAT: LONG:								
DRILLIN DRILLIN DRILLIN	G CON G EQU G MET	IPANY: IPMEN HOD: <b> </b>	CalPac ⊤: B-61	GROUND ELEV (ft):       TOC ELEV (ft):         INITIAL DTW (ft): Not Encountered       WELL DEPTH (ft): 3         STATIC DTW (ft): Not Encountered       BOREHOLE DEPTH         WELL CASING DIA. (in): 4       BOREHOLE DIA. (in): CHECKED BY:								
Time & Depth (feet)	Graphic Log	nscs	Description	Sample	Time Sample ID	Measured Recov. (feet)	Blow Count	Headspace PID (units)	Depth (feet)	Co	Well nstruction	
30-	-	CL	<b>CLAY</b> ; CL; 10YR 5/3 brown; medium plasticity; firm; moist; silty		0940 SVE-1@ 30-30.5'	1.5	6 8 20	25.9	- - - 30		-4" Sch 40 PVC 0.020" Slotted Screen	
35-	-		SILT ; ML; 2.5Y 6/2 brownish gray; low plasticity; firm; moist; clayey		0950 SVE-1@ 35-35.5'	1.5	5 5 8	6.8	- - 35-		−#3 Sand	
30 TEMPLATE 010509.GDT 1/4/12 - 06	-								- - - 40-			
GEO FORM 304_10HWYTIPPECANOE_DEC2011.GPJ_STANTEC ENVIRO TEMPLATE 010509.GDT_14/ 6 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0	-								- - 45			
GEO FORM 304 10HWYTI	-								-   -			

LOCATION PROJECT DRILLING INSTALLA DRILLING DRILLING	N: <b>M8</b> NUM : TION COM EQUI	AM Sm BER: STAR STAR STAR PANY: PMEN	TED: <b>12/28/11</b> COMPLETED: <b>12/28/11</b> TED: <b>12/28/11</b> COMPLETED: <b>12/28/11</b> <b>CalPac</b> T: <b>B-61</b>	WELL / PROBEHOLE / BOREHOLE IDENTIFICATION:       SVE-2         PAGE 1 OF 2       SVE-1         NORTHING (ft):       LONG:         LAT:       LONG:         GROUND ELEV (ft):       TOC ELEV (ft):         INITIAL DTW (ft): Not Encountered       WELL DEPTH (ft): 35         STATIC DTW (ft): Not Encountered       BOREHOLE DEPTH (ft):								
DRILLING SAMPLING			ISA NT: Split Spoon	WELL CASING DIA. (in): 4 BOREHOLE DIA. (in): 1 LOGGED BY: ME CHECKED BY:								
Time & Depth (feet)	Graphic Log	nscs	Description	ອ ຊີ່ Time ຮັ້ Sample ID		Measured Recov. (feet) Blow Count Headspace		Headspace PID (units)	Depth (feet)	С	Well onstruction	
5		SC	<b>CLAYEY SAND</b> ; SC; 2.5Y 4/2 dark grayish brown; fine to medium-grained; moist; 15% fines; trace subrounded coarse grained sand Same as above		1110 SVE-2@ 5-5.5' 1115 SVE-2@ 10-10.5'	1.5	7 6 7 10	0	- - - 5- - - - - - - - - - - - - - - -		- Grout	
GEO FORM 304 10HWYTIPPECANOE_DEC2011.GPJ STANTEC ENVIRO TEMPLATE 010509.GDT 1/4/1/		SM	<b>SILTY SAND</b> ; SM; 5Y 4/2 olive brown; fine-grained; moist; 40% fines; micaceous		1120 SVE-2@ 15-15.5'	1.5	6 8 10	390	- - 15 -		<ul> <li>- 3/8 Hydrate Bentonite Chips</li> </ul>	
- ECZ011.GPJ STA			Same as above		1125 SVE-2@ 20-20.5'	1.5	7 8 8	1190	- 20- -		<ul> <li>₩3 Sand</li> <li>₩3 Sand</li> </ul>	
3EO FORM 304 10					1130	1.5	12 18		-		PVC 0.020 Slotted Screen	

LOCATIC PROJEC	)n: <b>M8</b> t num	AM Sn BER:	-	IDE	ELL / PROBEH ENTIFICATION GE 2 OF 2		BOREH	IOLE S	/E-2	2 Stantec
DRILLING DRILLING DRILLING	ATION G COM G EQUI G METH	: STAR PANY: PMEN HOD: <b>F</b>		LA GR INI ST WE	OUND ELEV ( TIAL DTW (ft):	FING (ft): G: ELEV (ft): L DEPTH (ft): <b>35</b> EHOLE DEPTH (ft): <b>35.5</b> EHOLE DIA. (in): <b>10</b> CKED BY:				
Time & Depth (feet)	Graphic Log	NSCS	Description	Sample	· Time Sample ID	Measured Recov. (feet)	Blow Count	Headspace PID (units)	Depth (feet)	Well Construction
		SM	SILTY SAND ; SM; 5Y 4/2 olive brown; fine to medium-grained; moist; 20% fines; micaceous	$\times$	SVE-2@ 25-25.5'		22	996	-	
30-	-	ML	<b>SILT WITH SAND</b> ; ML; 2.5Y 4/3 olive brown; low plasticity; moist; 10% fine grained sand; slow dilation; micaceous	X	1135 SVE-2@ 30-30.5'	1.5	5 8 22	651	30-	4" Sch 40 PVC 0.020" Slotted Screen
		- CL	CLAY WITH SAND ; CL; 2.5Y 4/1 dark gray; low plasticity; moist; 15% silty fine grained sand; micaceous Borehole terminated at 35.5 feet.		1140 SVE-2@ 35-35.5'	1.5	4 5 6	248	- 35-	#-#3 Sand
IRO TEMPLATE 010509.GDT 1/4/12	-								- - - 40	
GEO FORM 304 10HWYTIPPECANOE_DEC2011.GPJ STANTEC ENVIRO TEMPLATE 010509.GDT 1/4/1	-								- - 45 -	
GEO FORM 304 1	-								-	-

LOCATION PROJECT DRILLING: INSTALLA DRILLING DRILLING SAMPLING	A: <b>M&amp;</b> NUME TION: COMF EQUII METH & EQU	M Sr BER: STAR STAR STAR PANY: PANY: PMEN IOD: H IPMEN	TED: 12/28/11 COMPLETED: 12/28/11 TED: 12/28/11 COMPLETED: 12/28/11 CalPac T: B-61	IDEN PAG NOR LAT: GRC INITI STA WEL LOG	UND ELEV (ft): AL DTW (ft): <b>Not E</b> TIC DTW (ft): <b>Not I</b> L CASING DIA. (in GED BY: <b>ME</b>	BOREHOLE BOREHOLE CHECKED	STING (ft): NG: C ELEV (ft): LL DEPTH (ft): <b>35</b> REHOLE DEPTH (ft): <b>35.5</b> REHOLE DIA. (in): <b>10</b> ECKED BY:		
Time & Depth (feet)	Graphic Log	nscs	Description	Sample	Time Sample ID Sample ID	Blow Count	Headspace PID (units)	Depth (feet)	Well Construction
EMPLATE 010509.GDT 1/4/12		SC	CLAYEY SAND ; SC; 10YR 4/2 dark grayish brown; fine-grained; dense; moist; 40% fines 10YR 4/3 brown SAND ; SP; 2.5Y 5/3 light olive brown; fine-grained; loose; dry; poorly graded						Grout
STANTEC ENVIRO T									← Hydrated Bentonite Chips
GEO FORM 304 10HWYTIPPECANOE_DEC2011.GPJ STANTEC ENVIRO TEMPLATE 010509.GDT 1/4/1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		CL	<b>SILTY CLAY</b> ; CL; 2.5Y 5/2 grayish brown; medium plasticity; firm; moist; strong HC odor					20	4" Sch 40 PVC 0.020 Slotted Screen

INSTALLATION: DRILLING COMI DRILLING EQUI DRILLING METH	M Sn BER: STAR STAR STAR PANY: PMEN IOD: H	TED: 12/28/11 COMPLETED: 12/28/11 TED: 12/28/11 COMPLETED: 12/28/11 CalPac T: B-61	WELL / PROBEHOLE / BOREHOLE IDENTIFICATION: PAGE 2 OF 2       SVE-3         NORTHING (ft): LAT: GROUND ELEV (ft):       EASTING (ft): LONG: TOC ELEV (ft): INITIAL DTW (ft): Not Encountered STATIC DTW (ft): Not Encountered WELL CASING DIA. (in): 4 LOGGED BY: ME       BOREHOLE DEPTH (ft): 35 BOREHOLE DEPTH (ft): 35 BOREHOLE DEPTH (ft): 10 CHECKED BY:								
Time & Depth (feet) Graphic Log	NSCS	Description	Sample	Time Sample	D Measured Recov.	(feet) Blow Count	Headspace PID (units)	Well Construction			
GEO FORM 304 10HWYTIPPECANOE_DECZ011.0PJ STANTEC ENVIRO TEMPLATE 010509.GDT 1/4/12	SM	brown; fine-grained; dense; moist; 30% fines SILTY SAND ; SM; 2.5Y 4/1 dark gray; fine to medium-grained; firm; moist; 40% fines; strong old HC odor CLAY ; CL; 10YR 5/2 grayish brown; high plasticity; firm; moist; trace silt Borehole terminated at 35.5 feet.		0810 SVE-3( 30-30.9 0820 SVE-3( 35-35.9	0 5 1.5 0 1.5	9 15 30	<u>₹</u> 1350 5.0		4" Sch 40 PVC 0.020" Slotted Screen #3 Sand		

L F D IN	PROJEC RILLING ISTALL/	DN: <b>Tif</b> <u>CT NUM</u> B: ATION:	D <b>PECA</b> IBER: STAF STAF	anoe and Laurelwood, San Bernardino ( 185802437 RTED 11/15/13 COMPLETED: 11/15/13 RTED COMPLETED:		ILL / PROBEN THING (ft): ITUDE: DUND ELEV (	<u>V-4</u>		<u>1 OF</u>	2 EASTI LONG	ING (ft): ITUDE: ELEV (ft):	Stantec
D D	RILLING RILLING	G EQUII G METH	PMEN IOD: <b>F</b>	ABC Liovin T: R-3 Hollow Stem Auger NT: NA	INIT STA WEL		HOLE DEF DEPTH (ff	IAMETER (in): <b>10</b>				
i	Time & Depth (feet)	Graphic Log	nscs	Description	Sample	Time Sample ID	Measured Recov. (feet)	Blow Count	Headspace PID (units)	Depth (feet)		Well Construction
		-		6" Concrete								<ul> <li>12" Traffic Rated Well Box</li> <li>4" Dia Sch 40 PVC Blank</li> </ul>
	5		SM	SILTY SAND ; SM; 10YR 3/4 dark yellowish brown; moist; no odor; 20-30% fines; light density				3 4 6		5-	-	<ul> <li>Hydrated Bentonite Annular Sea</li> </ul>
	10-		ML SP	SANDY SILT ; ML; 10YR 5/6 yellowish brown; moist; slight HC odor; 50% fine to medium grained sand; medium density POORLY GRADED SAND ; SP; 10YR 6/3 pale brown; fine-grained; strong HC odor				4 6 8		10-		
	15 <sup>.</sup>		ML	SILT ; ML; 10YR 5/4 yellowish brown; moist; slight HC odor; 5-10% fine grained sand				4 5 6		15-		— 4" Dia Sch 4 PVC 0.020" Slot set in Filter Pack Sand
			SM	SILTY SAND ; SM; 10YR 4/3 dark yellowish brown; moist; strong HC odor; 40-50% fines				4 7 9				

PROJECT NUMBER: DRILLING: STAI INSTALLATION: STAI DRILLING COMPANY: DRILLING EQUIPMEN	anoe and Laurelwood, San Bernardino ( 185802437 RTED 11/15/13 COMPLETED: 11/15/13 RTED COMPLETED: ABC Liovin T: R-3 Hollow Stem Auger	NOF LATI GRC INIT STA WEL	RTHING (ft): ITUDE: DUND ELEV ( IAL DTW (ft): TIC DTW (ft): LL CASING D GGED BY: <b>HR</b>	ft): Not Er Not Er	PAGE ncount ncoun ER (in):	2 OF tered tered	2 EASTI LONG TOC E BORE WELL BORE CHEC	NG (ft): ITUDE: :LEV (ft): HOLE DEPTH (ft): <b>30.5</b> DEPTH (ft): HOLE DIAMETER (in): <b>10</b> KED BY: Well Construction
25- - - - - - - - - - - - - - - - - - -	SAME AS ABOVE ; 2.5Y 4/2 dark grayish brownSAME AS ABOVE Borehole terminated at 30.5 feet.				6 8 9 12	Hei		<ul> <li>-6" Threaded End Cap</li> </ul>

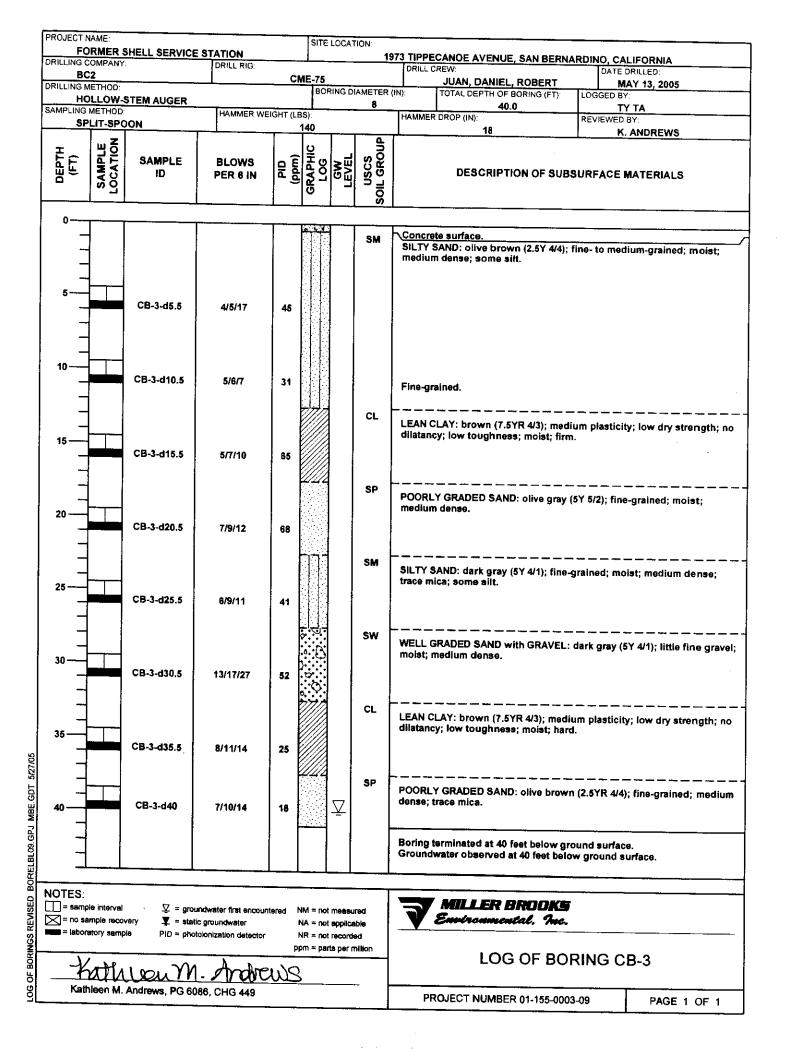


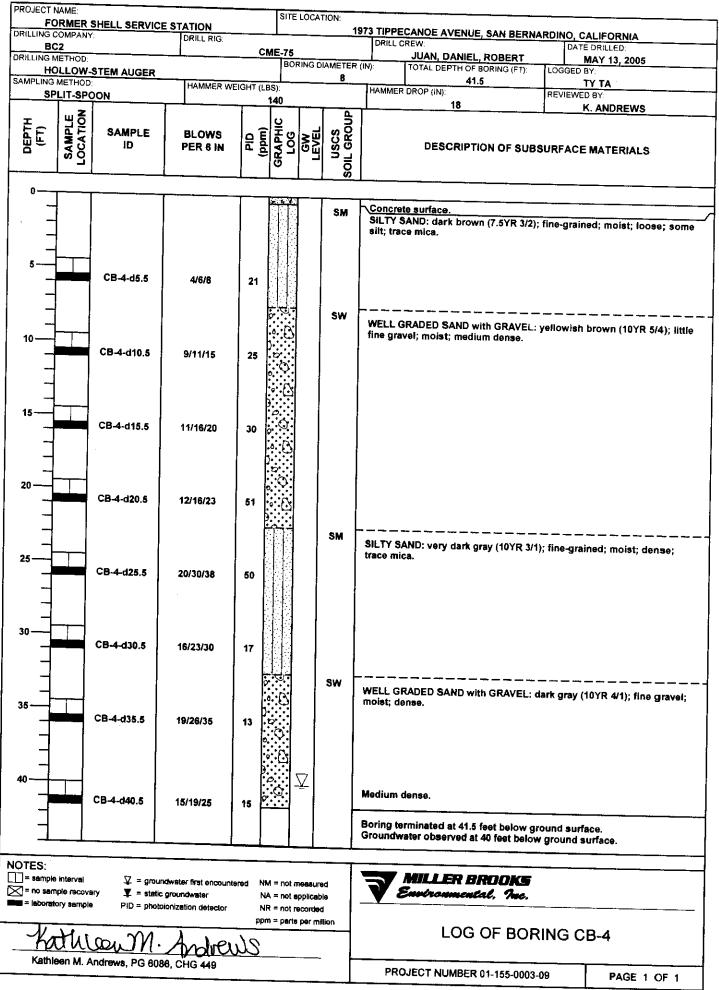
# **APPENDIX C2.7**

Equilon Enterprises / Shell GeoTracker Case I.D. T0607100504

PROJECT NAME:				SITE LOC	TE LOCATION:								
FORMER	SHELL SERVICE	DRILL RIG		L <u></u>	1	1973 TIPPECANOE AVENUE, SAN BERNARDINO, CALIFORNIA							
BC2	•	DRILL RIG:	СМ	E-75		DRILL	CREW:	DATE	E DRILLED:				
DRILLING METHOD					DIAMETER	(IN):	JUAN, DANIEL, ROBERT TOTAL DEPTH OF BORING (FT):		MAY 13, 2005				
SAMPLING METHOD	STEM AUGER			<u> </u>	8		41.0		ΤΥ ΤΑ				
SPLIT-SP		HAMMER W		3): <b>40</b>		HAMME	R DROP (IN):	REVIEWED	D BY:				
DEPTH (FT) SAMPLE LOCATION	SAMPLE	BLOWS	TT	0	SCS GROUP				K. ANDREWS				
	ID	PER 6 IN	OId (mdd)	GRAPH LOG GW	USCS USCS SOIL GRO	i	DESCRIPTION OF SUB	MATERIALS					
0	<u> </u>	·····	<u> </u>		·								
	1				SM		SAND, allor brown (0.5% ((4)						
						trace f	SAND: olive brown (2.5Y 4/4); ine gravel; moist; dense; trace	nne- to me mica.	dium-grained; some silt;				
5	· ·												
	CB-1-d5.5	15/23/30	12.6										
			1										
10													
	CB-1-d10.5	8/11/17	30				the state of the s						
_							llve brown (2.5Y 5/4); medium	dense.					
_				1	CL	┝							
						LEAN	CLAY: brown (7.5YR 4/3); med	ium plastic	ity; low dry strength; no				
15						dilatan	cy; low toughness; moist; har	d.	•••••••••••••••••••••••••••••••••••••••				
	CB-1-d15.5	11/13/15	31			ļ							
				<i>u</i> 44	SP	┝╺╴╴╴							
						POORI	Y GRADED SAND: olive gray	(5Y 5/2); fin	1e-grained; moist;				
20						mediu	n dense.						
	CB-1-d20.5	9/13/17	181										
_													
_					SM								
						SILTY	SAND: olive gray (5Y 5/2); fine-	grained; m	ioist; medium dense;				
25						some s	li <b>t</b> .						
	CB-1-d25.5	8/11/14	140										
-													
					sw								
						WELL	GRADED SAND with GRAVEL:	dark gray (	(5Y 4/1); little fine gravel;				
30——			1 5	-		moist;	very dense.						
	CB-1-d30.5	12/28/50	36										
-			0										
-	1			77/7	CL			·					
						dilatan	CLAY: brown (7.5YR 4/3); medi cy; low toughness; moist; hard	um plastici 4	ity; low dry strength; no				
35	00.4.497.5						i i i i i i i i i i i i i i i i i i i	1.					
	CB-1-d35.5	8/13/19	18										
~~													
	1			ררו	SM								
	İ					dense:	AND: light olive brown (2.5Y ) trace mica.	5/4); fine-gr	rained; wet; medium				
40	0.0			⊻		,							
-	CB-1-d40	10/16/24	17	11									
-						Boring	terminated at 41 feet below gr	ound surfa	ce.				
						Ground	water observed at 40 feet belo	w ground	surface.				
			LI	1	<u> </u>			·	·······				
IOTES:			·				······································						
= sample interval		undwater first encou	nteneri Nil	/ = not mea	Auror		MILLER BROOKS						
= no sample reco		ic groundwater		4 = not appli		V	Environmental, Inc.						
≠ laboratory samp		toionization detector		R = not reco	, i								
			ppr	n = parta pe	r million								
That	$10 - \overline{1}$	1 A . P.	0	\			LOG OF BO	KING C	JB-1				
	ulen IV	1. Andr	tus										
Kathleen M.	Andrews, PG 60	86, CHG 449			[	P	ROJECT NUMBER 01-155-000	3-09	PAGE 1 OF 1				

PROJECT NAME:				SITE LOCA	TION:								
FORMER S	SHELL SERVICE				19	1973 TIPPECANOE AVENUE, SAN BERNARDINO, CALIFORNIA							
BC2	•	DRILL RIG:	CM	E-75		DRILL CREW: DATE DRILLED:							
DRILLING METHOD:		······································		BORING	IAMETER	JUAN, DANIEL, ROBERT         MAY 13, 2005           R (IN):         TOTAL DEPTH OF BORING (FT):         LOGGED BY:							
HOLLOW-	STEM AUGER			•	8	40.0 TY TA							
SPLIT-SPC		HAMMER WE		S): 40		HAMMER DROP (IN): REVIEWED BY:							
DEPTH (FT) SAMPLE LOCATION	SAMPLE ID	BLOWS PER 6 IN	T	GRAPHIC LOG GW LEVEL	USCS SOIL GROUP	DESCRIPTION OF SUBSURFACE MATERIALS							
,,,	<u> </u>	<u> </u>	i		Ŭ,								
0			<del></del>	an a	,								
-					SM	Concrete surface.							
5	CB-2-d5.5	5/6/8	24			SILTY SAND: olive brown (2.5Y 4/4); fine-grained; moist; medium dense some silt.							
	CB-2-d10.5	6/8/12	30										
	CB-2-d15.5	7/9/11	29		CL	LEAN CLAY: brown (7.5YR 4/3); medium plasticity; low dry strength; no dilatancy; low toughness; moist; firm.							
20	CB-2-d20.5	6/8/9	250		SP	POORLY GRADED SAND: brown (7.5YR 4/3); fine-grained; moist; medium dense.							
25	CB-2-d25.5	5/7/10	221		SM	SILTY SAND: olive brown (2.5Y 4/4); fine-grained; moist; medium dense.							
30	CB-2-d30.5	17/26/40	50		SW	WELL GRADED SAND with GRAVEL: olive brown (2.5Y 4/4); fine gravel; moist; dense.							
35	CB-2-d35.5	6/9/13	26		CL	LEAN CLAY: brown (7.5YR 4/3); medium plasticity; low dry strength; no dilatancy; low toughness; moist; firm.							
40	CB-2-d40	8/12/17	11	 	SP	POORLY GRADED SAND: olive brown (2.5YR 4/4); fine-grained; medium dense; trace mica.							
						Boring terminated at 40 feet below ground surface. Groundwater observed at 40 feet below ground surface.							
NOTES: = sample interva = no sample reco = laboratory sam	overy 🗶 = stat	Indwater first encou lo groundwater toionization detector	4	IM = not meas VA ≃ not applie VR = not recor	able	Miller BRODKS Environmental, Inc.							
Kath	Learm.	Andrew	p¢	m = parts per	LOG OF BORING CB-2								
	. Andrews, PG 60					PROJECT NUMBER 01-155-0003-09 PAGE 1 OF 1							



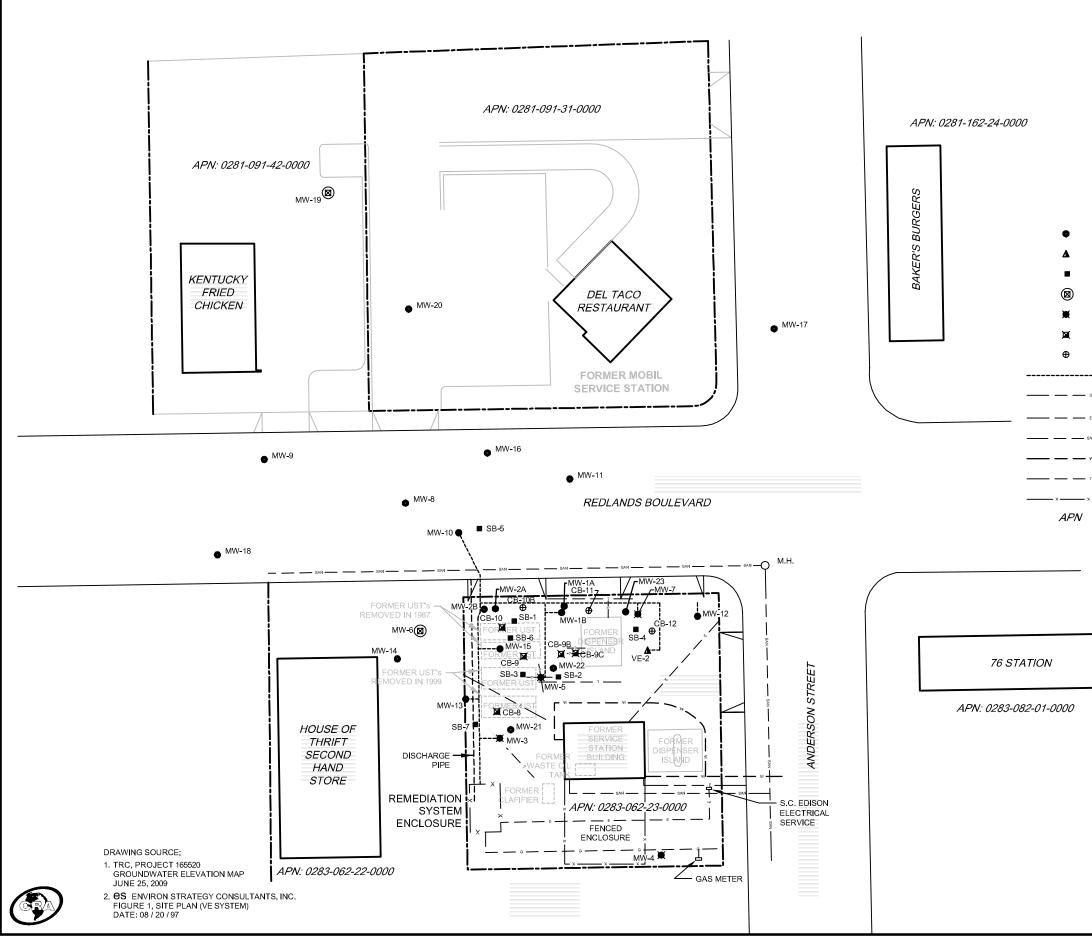


LOG OF BORINGS REVISED BORELBL09.GPJ MBE.GDT 5/27/05

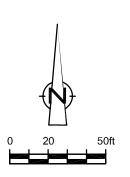


### **APPENDIX C2.8**

Unocal #2417 GeoTracker Case I.D. T0607100008



<sup>53972-95(007)</sup>GN-WA001 MAR 15/2010



#### LEGEND

GROUNDWATER MONITORING WELL

VAPOR EXTRACTION WELL

PREVIOUS CONFIRMATION SOIL BORING

MONITORING WELL SLATED FOR DESTRUCTION

DESTROYED MONITORING WELL

ATTEMPTED CONFIRMATION SOIL BORING

CONFIRMATION SOIL BORING (2009)

- ---- VAPOR EXTRACTION PIPING
- ----- GAS LINE TRENCH
- \_\_\_\_ ELECTRICAL LINE TRENCH
- \_\_\_\_ SANITARY SEWER LINE
- WATER SERVICE LINE
- UNKNOWN UNDERGROUND UTILITY LINE
- ----- FENCE

ASSESSOR'S PARCEL NUMBER

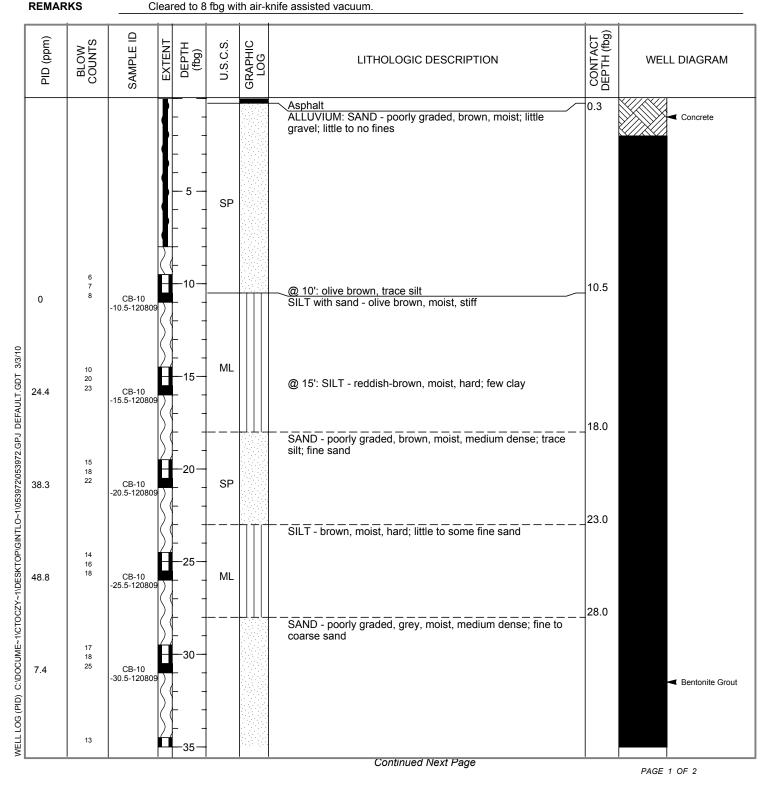
figure 2

SITE PLAN FORMER 76 STATION No. 2417 24891 REDLANDS BOULEVARD *Loma Linda, California* 



#### **BORING / WELL LOG**

CLIENT NAME	ConocoPhillips	BORING/WELL NAME CB-10B	
JOB/SITE NAME	Former 76 Station No. 2417	DRILLING STARTED 03-Dec-09	
LOCATION	24891 Redlands Blvd. Loma Linda, California	DRILLING COMPLETED 09-Dec-09	
PROJECT NUMBER	053972	WELL DEVELOPMENT DATE (YIELD)	NA
DRILLER	ABC Liovin Drilling	GROUND SURFACE ELEVATION	1063.00 ft above msl
DRILLING METHOD	Hollow-stem auger	TOP OF CASING ELEVATION	NA
BORING DIAMETER	8 Inch	SCREENED INTERVALS	NA
LOGGED BY	Tara Morton-Bernas	DEPTH TO WATER (First Encountered)	54.00 fbg
REVIEWED BY	Jim Schneider PG 7914	DEPTH TO WATER (Static)	NA 💆





# **BORING / WELL LOG**

**CLIENT NAME JOB/SITE NAME** LOCATION

ConocoPhillips Former 76 Station No. 2417 24891 Redlands Blvd. Loma Linda, California **BORING/WELL NAME DRILLING STARTED** 

CB-10B 03-Dec-09

DRILLING COMPLETED 09-Dec-09

Continued from Previous Page

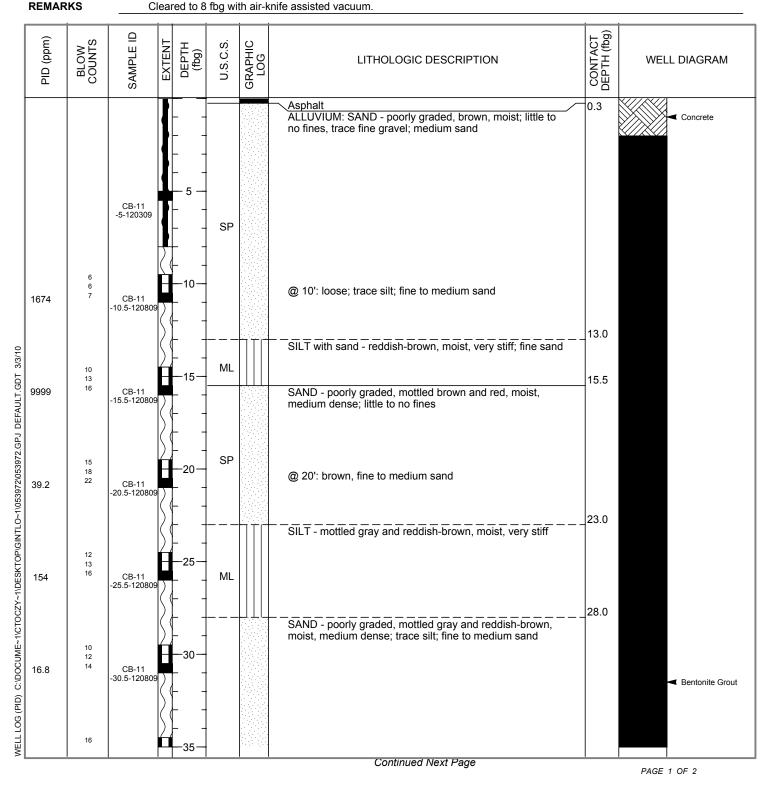
(mq	NTS ST	<u>О</u> щ	ENT	TH (	S.S.	⊇ T			ACT (fbg)		
PID (ppm)	BLOW COUNTS	SAMPLE ID	EXTENT	DEPTH (fbg)	U.S.C.S.	GRAPHIC LOG	LITHOLOGIC DESCRIPTION		CONTACT DEPTH (fbg)	WELL	DIAGRAM
27.9	18 23	CB-10 -35.5-120809					@ 35': mottled brown and olive gray; trace silt; fine sand				
8.9	15 18 22	CB-10 -40.5.5-12080		  - 40 	- - - -		@ 40': brown				
35.9	4 20 14	CB-10 -45.5-120809		 - 45 	SP		@ 45': mottled brown and olive gray				
52.2	10 12 14	CB-10 -50.5-120809		50 	-		@ 50': trace silt				
41.2	16 18 20	CB-10 -55.5-120809	\ 	 55 	-		@ 54': brown, saturated; medium sand	Ţ			
54	13 14 15	CB-10 -60.5-120809		 60 <i></i>			@ 60': dark brown; fine sand		61.0	B	ottom of Boring 0 61 fbg
										PAGE 2 0	OF 2

WELL LOG (PID) C:\DOCUME~1\CTOCZY~1\DESKTOP\GINTLO~1\053972\053972.GPJ DEFAULT.GDT 3/3/10



#### **BORING / WELL LOG**

CLIENT NAME	ConocoPhillips	BORING/WELL NAME CB-11	
JOB/SITE NAME	Former 76 Station No. 2417	DRILLING STARTED 03-Dec-09	
LOCATION	24891 Redlands Blvd. Loma Linda, California	DRILLING COMPLETED 08-Dec-09	
PROJECT NUMBER	053972	WELL DEVELOPMENT DATE (YIELD)	NA
DRILLER	ABC Liovin Drilling	GROUND SURFACE ELEVATION	1063.00 ft above msl
DRILLING METHOD	Hollow-stem auger	TOP OF CASING ELEVATION	NA
BORING DIAMETER	8 Inch	SCREENED INTERVALS	NA
LOGGED BY	Tara Morton-Bernas	DEPTH TO WATER (First Encountered	I) NA 🖓
REVIEWED BY	Jim Schneider PG 7914	DEPTH TO WATER (Static)	NA





# **BORING / WELL LOG**

CLIENT NAME JOB/SITE NAME LOCATION

ConocoPhillips Former 76 Station No. 2417 24891 Redlands Blvd. Loma Linda, California BORING/WELL NAME DRILLING STARTED DRILLING COMPLETED 08-Dec-09

CB-11 03-Dec-09

Continued from Previous Page

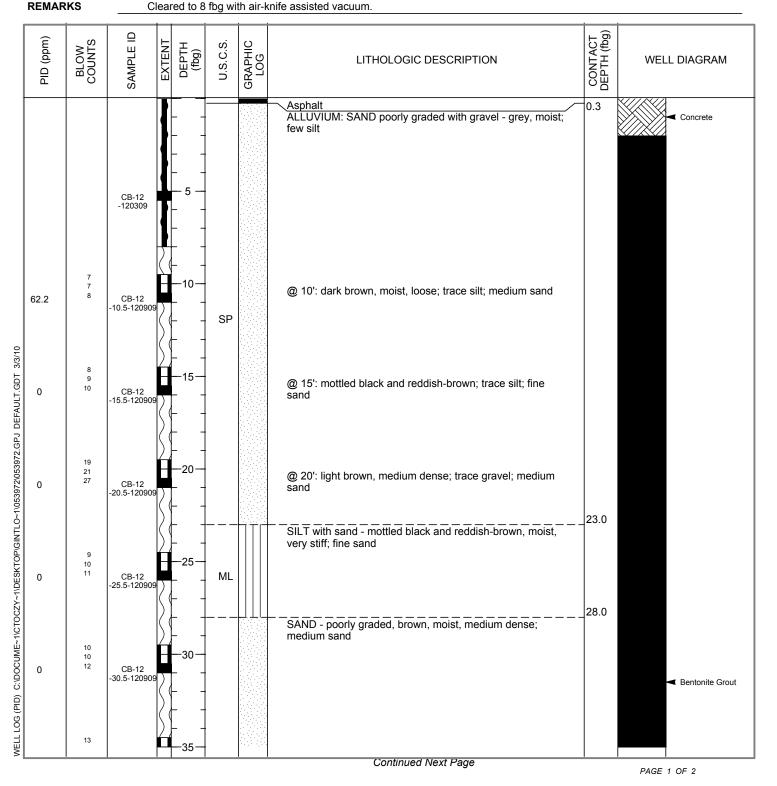
PID (ppm)	BLOW COUNTS	SAMPLE ID	EXTENT	DEPTH (fbg)	U.S.C.S.	GRAPHIC LOG	LITHOLOGIC DESCRIPTION	CONTACT DEPTH (fbg)	WELL DIAGRAM
18.4	18 20	CB-11 -35.5-120809					@ 35': dark brown		
17.5	13 14 15	CB-11 -40.5-120809		 40 	SP				
23.4	6 6 12	CB-11 -45.5-120809	)     	 45 			@ 45': mottled brown and black, loose, moist; fine sand; trace silt		
22.6	10 11 12	CB-11 -50.5-120809		 50 	— — — . ML		SILT with sand - mottled brown and black, moist, stiff, fine sand	48.0	
DEFAULT.GDT 3/3/10 18.4	11 13 13	CB-11 -55.5-120809			SP		SAND - poorly graded, brown, moist, medium dense; fine sand	53.0	
WELL LOG (PID) C:DOCUME-1\CTOCZY~1\DESKTOP\GINTLO-1\053972.053972.GPJ DEFAULT.GDT 3/3/10 	12 13 14	CB-11 -60.5-120809		60 <i></i>				61.0	Bottom of Boring @ 61 fbg
CTOCZY~1\DESKTOP\GIN									
LOG (PID) C:\DOCUME~1									
MELL									PAGE 2 OF 2



Conestoa-Rovers & Associates, Inc. 175 Technology Dr. Suite 150 Irvine, CA 92618 Telephone: 949-648-5200 Fax: 949-648-5299

#### **BORING / WELL LOG**

CLIENT NAME	ConocoPhillips	BORING/WELL NAME CB-1	2
JOB/SITE NAME	Former 76 Station No. 2417	DRILLING STARTED 07-D	ec-09
LOCATION	24891 Redlands Blvd. Loma Linda, California	DRILLING COMPLETED 08-De	ec-09
PROJECT NUMBER	053972	WELL DEVELOPMENT DATE (Y	IELD) NA
DRILLER	ABC Liovin Drilling	GROUND SURFACE ELEVATIO	N 1062.50 ft above msl
DRILLING METHOD	Hollow-stem auger	TOP OF CASING ELEVATION	NA
BORING DIAMETER	8 Inch	SCREENED INTERVALS	NA
LOGGED BY	Tara Morton-Bernas	DEPTH TO WATER (First Encou	Intered) NA $\overline{\Sigma}$
REVIEWED BY	Jim Schneider PG 7914	DEPTH TO WATER (Static)	NA <u>Y</u>
		,	





# **BORING / WELL LOG**

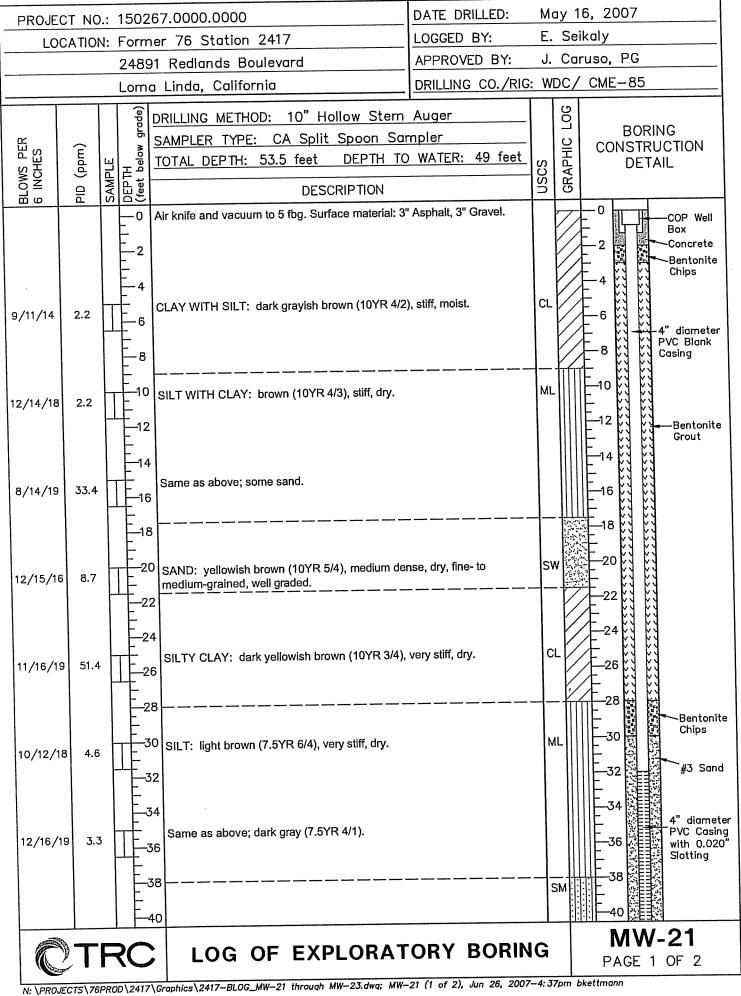
CLIENT NAME JOB/SITE NAME LOCATION

ConocoPhillips Former 76 Station No. 2417 24891 Redlands Blvd. Loma Linda, California BORING/WELL NAME DRILLING STARTED DRILLING COMPLETED 08-Dec-09

CB-12 07-Dec-09

Continued from Previous Page

PID (ppm)	BLOW COUNTS	SAMPLE ID	EXTENT	DEPTH (fbg)	U.S.C.S.	GRAPHIC LOG	LITHOLOGIC DESCRIPTION	CONTACT DEPTH (fbg)	WELL DIAGRAM
0	15 18	CB-12 -35.5-120909	• • • •		SP		@ 35': fine to medium sand; trace silt		
0	12 13 14	CB-12 -40.5-120905					@ 40': medium to coarse sand; some angular gravel @ 40.25': fine sand; few silt	_43.0	
0	19 10 10	CB-12 -45.5-120905		 45 			SILT - mottled black and brown, moist, stiff; trace sand		
0	18 19 21	CB-12 -50.5-120909	) ( 	 50 	ML				
0	8 10 12	CB-12 -55.5-120905	ζ( ] 	 55			SAND - poorly graded, mottled black and brown, moist, medium dense; trace silt; fine to medium sand	53.0	
0	17 19 21	CB-12 -60.5-120905		 60 <i></i>	SP			61.0	Bottom of Borir @ 61 fbg
									PAGE 2 OF 2



PROJE	CT NC	).:	1502	67.0000.0000	DATE DRILLED:	Mc	iy 1	6, 2007
				er 76 Station 2417	LOGGED BY:			kaly
			2489	1 Redlands Boulevard	APPROVED BY:			uso, PG
			Lom	a Linda, California	DRILLING CO./RIG	: W[		CME-85
BLOWS PER 6 INCHES	PID (ppm)	SAMPLE	JEPTH (feet below gro	DESCRIPTION	npler WATER: 49 feet		GRAPHIC LOG	
17/25/35	3.3	Г	- 40	SILTY SAND: dark gray (7.5YR 4/1), dense, moist poorly graded.	t, fine-grained sand,	SM		
15/20/26	3.0	$\left  \right $	- 42	CLAVE growing brown (10VP 5/2) hard moist		CL		42
	10.7	$\left  \right $	£	CLAYEY SILT: dark yellowish brown (10YR 4/6), I	hard, moist.	ML		
13/28/33			<u></u> +44	Same as above.				447, E 17, 4" diameter
21/28/30	4.6	Ш	F-46					46 With 0.020"
			E					Slotting
			- 48				X	#3 Sand
		ŀ	F50					-50
			F				╁┼┤┾┼	-52 -End Cap
15/18/25	3.0	h	52	SILT: grayish brown (10YR 5/2), very stiff, moist.		ML		
		┝	-54	Bottom of Boring 53.5 fbg		1		54
			E	Approximate materials Used:				
			-56	2.21 cu.ft. of Concrete				
			E58	1.37 cu.ft. of Bentonite Chips				58
			E	11.45 cu.ft. of Bentonite Grout 10 cu.ft. of #3 Sand				
			60					
			E-6:	2				62
			F.	-				
			-6	4				64 
			- 6					E66
			E					
			-6	8				68
			F_					E70
			<u>–</u> 7	0				
			<u> </u> _7	2				72
			Ē					
			-7	4				
			E_7	76				-76
				78				—78 —
				30				
C	) <b>T</b>	F	RC	LOG OF EXPLORAT	ORY BORIN	G		<b>MW-21</b> PAGE 2 OF 2

N: \PROJECTS\76PROD\2417\Graphics\2417-BLOG\_MW-21 through MW-23.dwg; MW-21 (2 of 2), Jun 26, 2007-4; 37pm bkettmann

PROJECT NO.: 150267.0000.0000 DATE DRILLED: May 16, 2007												
					LOGGED BY:	Ε.	Seikaly					
		24	891		APPROVED BY:		Caruso, PG					
		Lo	ma	Linda, California	DRILLING CO./RIG:	WD	C/ CME-85					
BLOWS PER 6 INCHES	(mqq) Olq			DRILLING METHOD: 10" Hollow Stem A SAMPLER TYPE: CA Split Spoon Sam TOTAL DEPTH: 53.5 feet DEPTH TO DESCRIPTION		BORING CONSTRUCTION DETAIL						
8/13/15	5.3		0 A 2 4	Nir knife and vacuum to 5 fbg. Surface material: 3" A		CL	0 COP Well Box Concrete Bentonite Chips 4 4 4 4 4 4 4 4 4 4 4 4 4					
10/15/18	8.2		10 g -12	SILT WITH CLAY: brown (10YR 4/3), stiff, dry, som	ne sand. 	ML						
12/15/17	5.1			SANDY SILT: olive brown (2.5Y 4/4), very stiff, slig gravel.	ghtly moist, some	ML						
35/ 50 for 6"	10.2		-20	SAND WITH GRAVEL: light brown (2.5Y 3/4), ven medium- to coarse-grained sand, well graded.	y dense, slightly moist,	sw						
15/18/25	199		-26	SANDY SILT: very dark gray (7.5YR 3/1), very stif [hydrocarbon odor and some staining]	ff, slightly moist.	ML						
25/33/50	0 13.6			SAND: yellowish brown (10YR 5/4), dense, moist, sand, well graded, some silt.	, fine- to coarse-grained	sw	Bentonite					
21/2/46	5 104			SAND: brown (10YR 4/3), dense, moist, fine-grain graded.	ned sand, poorly	SP						
Ô	) ) <b>T [</b>	<b> </b>	40	LOG OF EXPLORATO	DRY BORING	SM	40 MW-22 PAGE 1 OF 2					

N: \PROJECTS\76PROD\2417\Graphics\2417-BLOG\_MW-21 through MW-23.dwa; MW-22 (1 of 2), Jun 26, 2007-4:37pm bkettmann

PROJE	CT NC	).:	1502	67.0000.0000	DATE DRILLED:	Ma	y 16, 2007		
				ner 76 Station 2417	LOGGED BY: E. Seikaly				
				01 Redlands Boulevard	APPROVED BY:	J.	Caruso, PG		
			Lom	a Linda, California	DRILLING CO./RIG	: WD	C/ CME-85		
BLOWS PER 6 INCHES	(mqq)	μ	H below grade)	DRILLING METHOD: 10" Hollow Stem SAMPLER TYPE: CA Split Spoon Sar TOTAL DEPTH: 53.5 feet DEPTH TO			BORING CONSTRUCTION DETAIL		
SNCI	pin (F	SAMPLE	DEPTH (feet be	DESCRIPTION		USCS	GRA		
<u>а</u> о 20/30/50 20/22/28 10/27/33 17/25/36	240 103 137 51.4		-40 $-42$ $-44$ $-44$ $-44$ $-48$ $-50$ $-52$ $-54$ $-54$	SILTY SAND: grayish brown (10YR 5/2), dense, c poorly graded. SILTY CLAY: dark yellowish brown (10YR 4/6), st CLAYEY SILT: yellowish brown (10YR 5/6), hard, Same as above. Same as above. SILT: grayish brown (10YR 5/2), very stiff, wet, se Bottom of Boring 53.5 fbg Aprroximate materials Used: 2.21 cu.ft. of Concrete 1.37 cu.ft. of Bentonite Chips 11.45 cu.ft. of Bentonite Grout 10 cu.ft. of #3 Sand	iff, moist.	SM CL ML	40 42 44 4" diameter PVC Casing with 0.020" Slotting -48 -50 -52 -58 -58 -60 -62 -64 -68 -70 -72		
			بتبابيا أبين	72 74 76 78			74 76 78 78		
	) <b>T</b>	F		LOG OF EXPLORAT	ORY BORIN	IG	-80 MW-22 PAGE 2 OF 2		

N: \PROJECTS\76PROD\2417\Graphics\2417-BLOG\_MW-21 through MW-23.dwg; MW-22 (2 of 2), Jun 26, 2007-4; 38pm bkettmann

PROJEC	PROJECT NO.: 150267.0000.0000 DATE DRILLED: May 16, 2007											
			ner 76 Station 2417	LOGGED BY:	E. 3	Seikaly						
			91 Redlands Boulevard	APPROVED BY:	J. (	Caruso, PG						
		Lom	a Linda, California	DRILLING CO./RIG	: WDC/ CME-85							
BLOWS PER 6 INCHES	(mqq) Olq	SAMPLE DEPTH (feet below grade)	DRILLING METHOD: 10" Hollow Stem SAMPLER TYPE: CA Split Spoon Sa TOTAL DEPTH: 53.5 feet DEPTH TO DESCRIPTION			BORING CONSTRUCTION DETAIL						
	<u>ц</u> .		Air knife and vacuum to 5 fbg. Surface material: 3	" Asphalt, 1.5' sand with		COP Well						
7/10/25	2.4		gravel. CLAY WITH SILT: dark grayish brown (10YR 4/2	), stiff, slightly moist.	<u>GP</u>	2 Box Concrete Bentonite Chips 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4						
10/12/15	6.3		SANDY SILT: olive brown (2.5Y 4/4), stiff, slightl			12 × × × Bentonite Grout						
17/30	6.7		SAND WITH GRAVEL: light olive brown (2.5Y 5 medium- to coarse-grained sand, well graded, tra	i/4), dense, slightly moist, ace silt.	sw							
18/21/24	15.7		CLAY WITH SILT: dark grayish brown (10YR 4/	2) very stiff, dry.	CL							
11/16/19	6.6		<sup>0</sup> SILTY CLAY: brown (7.5YR 4/3), stiff, dry.		CL	-30 -32 -32 -32 -32 -32 -32 -32 -33 -33 -33						
21/25/3	1 5.9		SILTY SAND: olive brown (2.5 4/4) dense, dry, graded.	fine-grained, poorly	SM							
C	) <b>T</b>	RC	LOG OF EXPLORAT			MW-23 PAGE 1 OF 2						

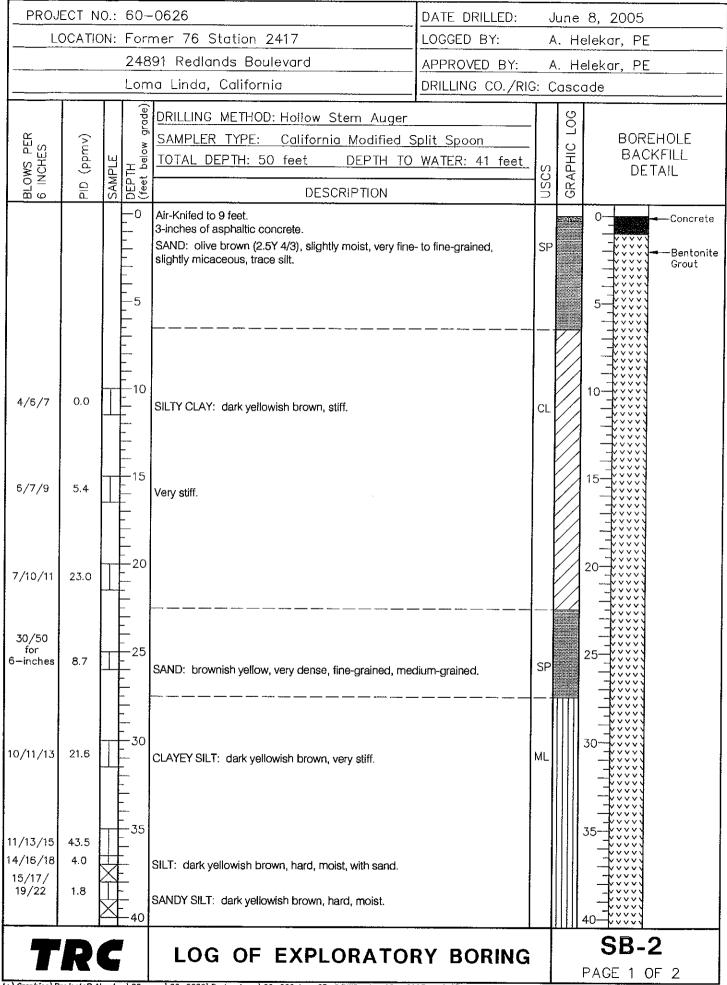
N:\PROJECTS\76PR0D\2417\Graphics\2417-BLOG\_MW-21 through MW-23.dwg; MW-23 (1 of 2), Jun 26, 2007-4:38pm bkettmann

PROJE	CT NC	).:	1502	67.0000.0000	DATE DRILLED:	Мс	iy 16	5, 2007	
				ner 76 Station 2417	LOGGED BY: E. Seikaly				
			2489	)1 Redlands Boulevard	APPROVED BY:	J.	Car	uso, PG	
			Lom	a Linda, California	DRILLING CO./RIG	: WE	<u>)c/</u>	CME-85	
н Н Н Н Н Н Н Н Н Н Н Н Н Н Н Н Н Н Н Н	(-		5	DRILLING METHOD: 10" Hollow Stem SAMPLER TYPE: CA Split Spoon Sar	npler		IC LOG	BORIN	
SHES P P	(mqq)	Ч	Н below	TOTAL DEPTH: 53.5 feet DEPTH TO	WATER: 49 feet	ស	GRAPHIC	DETA	
BLOWS PER 6 INCHES	DID (	SAMPLE	DEPTH (feet be	DESCRIPTION		NSCS	GR,		
18/26/35	9.4	Π	40 	Same as above; moist.		SM		- 40	
27/38/46	7.4	Π	- 42	SANDY SILT: olive brown (2.5Y 4/4), hard, moist,		ML		- <sup>42</sup> 跳影	
35/39/43	6.7	Ħ	- - 44	CLAYEY SILT: olive brown (2.5Y 4/4), hard, moist		ML		- 44	
33/37/49	8.1	H	$\frac{1}{2}$	Same as above.					" diameter VC Casing
		$\left  \right $						二 <sup>46</sup> 除非影,	vith 0.020" Slotting
			Ξ-48				Шŀ	- 48	-
			Ē						
			50					50 KEN	
			F-52	Same as above; wet, some sand.				-52	End Cap
28/35/37	11.1		$\vdash$						<sup>—</sup> #3 Sand
				Bottom of Boring 53.5 fbg				54 	
			F-56	Aprroximate materials Used:				-56	
			F	2.21 cu.ft. of Concrete					
			58 E	1.37 cu.ft. of Bentonite Chips 11.45 cu.ft. of Bentonite Grout					
			-60	10 cu.ft. of #3 Sand				-60	
			E						
			E-62	2					
			F64	1				-64	
			Ē					66	
			60 E	5	·			E	
			68	3				-68	
			F						
			E-70	D					
			-7	2				-72	
			Ē.						
				<sup>4</sup>					
			-7	6				-76	
			F.					78	
			7  _	8					
			8	0				-80	
			RC	LOG OF EXPLORAT	ORY BORIN	G		MW- PAGE 2	

	K(					F E	XPI		ATOR	Y BOR	ING			_	<b>с 1</b> С	_	
					<u> </u>		· · ·				<u>+</u>	'	للكريرين	<u> </u>	SB-	1	]
			- - -40											40			
15/17/23	0.0	┢╷	Ē	gradaed CLAYEY S								: ML					
12/12/14	1.7	ſ		SILTY SAN	ID: darl	(brown	, mediu	um dense	, fine-grain	ed, poolry		SM					
12/13/15	4.7	Γ	-35	SILT: dark	cgray, v	ery stiff,	moist.					ML		35-			
			Ē						-								
		Γ	Ē	 													
10/12/13	0.0	Π		SAND: graded.	ayish br	own, m	edium	dense, m	edium-grai	ined, poorly		SP		30-			ĺ
			F	O A N D													
			F					• <b></b>									
9/10/12	5.1		F														
		Ļ	F-25	CLAYEY S	SILT: da	rk brow	n, verv	stiff.				ML		25			
			F	<b> </b>		<u> </u>							ΠÍ				
,, ,, ,, ,, ,, ,, ,, ,, ,, ,, ,, ,, ,,	0.0	$\parallel$															
7/9/11	0.0	$\mathbb{H}$	<u>+</u> 20	SILTY CLA	AY: darl	< brown	, very s	tiff.				CL	//	20			
			Ē			<u></u>											
		Ľ	+					,				-					
5/7/8	3.9	Π	+-15 	SILT: bro	wn, stiff,	, some :	sand, s	ome clay	<b>'.</b>			мL		15-			
											i			-			
			Ē														
5/7/10	0.0		₽'ĭ -	1	Y SANE	): very	pale br	own, mea	dium dense	e, coarse-grain	ied,	sw		10-			
														-			
	ĺ									· ·							
4/5/7		ľ	Ŧ	No recove	эгу. — — —							l					
, <b>1</b> - 1			<u>+</u> 5	moist, fine	-							ļ		5-			
				FILL (SAN	VDY GR	AVEL):	ne nne dark y	gravel. ellowish t	 prown (10Y	R 4/4), slightly	<u> </u>	GP					out
			E	FILL (SAN	ND): da	rk yellov	wish br	own (10Y	′R 4/4), slig	htly moist, fine	- to	sw		- - -		-Be	ntonite
			Eo	Air-Knifed 4-inches c			rete							0-		<b>-</b> _Co	ncrete
BLO 6 ⊳	B	č	DEPT-				DE	SCRIPT	ION			USCS	GRA				
BLOWS PEF 6 INCHES	(ppmv)	Ľ			TOTAL DEPTH: 51.5 feet DEPTH TO WATER: 41.5 feet								GRAPHIC			CKFILI TAIL	_
PER	Î								dified Sp	olit Spoon						EHOL	
					G MET	HOD: I	Hollow	/ Stem	Auger	· · · · · · · · · · · · · · · · · · ·			100	Τ			
	-		Loi	ma Lindo	a, Coli	fornia	,			DRILLING C	0./RI						
			24	891 Redi	ands	Boule	vard			APPROVED	BY:				ar, PE	· · ·	
L	OCATI	٥N	l: Fo	rmer 76	Statio	on 24	17			LOGGED BY			~~		<u>2000</u> ar, PE		
PRO	PROJECT NO.: 60-0626							DATE DRILI	LED:		lune	e 7	2005				

_ PROJ	PROJECT NO.: 60-0626 DATE DRILLED						
L(	OCATIO	DN: For	mer 76 Station 2417	LOGGED BY:		lelekar, Pl	
		248	891 Redlands Boulevard	APPROVED BY:		lelekar, Pl	
		Lor	na Linda, California	DRILLING CO./RIC			
		(ade	DRILLING METHOD: Hollow Stem Auger		0		
н Н Н	5			Split Spoon	LOG	BOR	EHOLE
S P HES	(vmqq)	рејо Т Г	TOTAL DEPTH: 51.5 feet DEPTH TO	WATER: 41.5 feet	E	BAC	CKFILL
BLOWS PER 6 INCHES	) ald	SAMPLE DEPTH (feet below	DESCRIPTION		USCS GRAPHIC	DE	TAIL.
					<u></u> О мl	40	
15/19/21	0.0						-Bentonite Grout
14/15/20	0.0		CLAY: very dark grayish brown, hard, slightly mo	st.	CL		
10 40 40		45				45-2222	
16/18/19	0.0		Brown.				
17/19/20	0.0		Slightly moist, some silt.			50	
			Bottom of boring at 51.5 feet below grade.				
		-		1			
		55					
						55— —	
		-					
		-					
		-60				60-	
		E				-	
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		-65		¢		65	
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		-70				70	
						-	
-	Í	-75				75	
		E					
		-80				80-	
-	R		LOG OF EXPLORATOR			SB-	1
			LOG OF EAFLURATUR	I DUNING		PAGE 2 (	

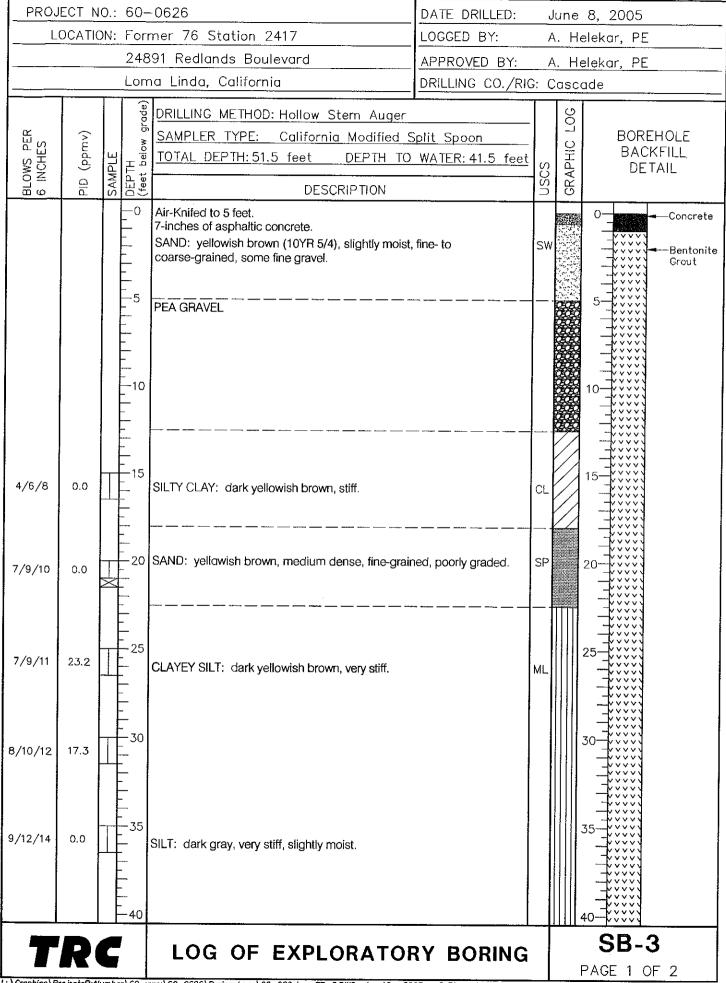
L:\Graphics\ProjectsByNumber\60-xxxx\60-0626\Boring Logs\60-626 Log SB-1.DWG Jun 16, 2005 - 3:28pm rhughes



L: \Graphics \ProjectsByNumber \50-xxxx \60-0626 \Boring Logs \60-626 Log SB-2.DWG Jun 16, 2005 - 2:27pm rhughes

PROJE	PROJECT NO.: 60-0626 DATE DRILLED						lune	8, 2005	
LC	CATIC	)N:	Forr	ner 76 Station 2417	LOGGED BY:	A	л. Н	elekar, PE	
			248	91 Redlands Boulevard	APPROVED BY:	Δ	<u>. Н</u>	elekar, PE	
			Lom	a Linda, California	DRILLING CO./RIC	G: (	asc	ade	
			grade)	DRILLING METHOD: Hollow Stem Auger			LOG		
н Н Н Н	Ŷ		l p ≯	SAMPLER TYPE: California Modified S	plit Spoon	ľ			EHOLE
VS F CHE	(vmqq)	Ш	ре Бе	TOTAL DEPTH: 50 feet DEPTH TO	WATER: 41 feet	S.	GRAPHIC		KFILL TAIL
BLOWS PER 6 INCHES	БÜ	SAMI	DEPTH (feet below •	DESCRIPTION		<b>USCS</b>	GRA		
17/19/23	1.7	H	40	CLAYEY SILT: wet.	N	ML	Ш М	40	- Bentonite Grout
18/20/24 50 for 6-inches	6.7 0.0			SANDY CLAY: dark yellowish brown, hard, moist. SILTY CLAY: dark yellowish brown, slightly moist.		CL			
0-inches			45					45	
				No recovery.				;;;;; 50;;;;;	
				Bottom of boring at 50 feet below grade.					
								-	
			-						
			55					55-	
			-						
			-						
			-						
-			60					60-	
			-					 	
			-						
			-						
			- 65					65	
			-					-	
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			70 					70	
			-					_	
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			- 					80	
-	R	C		LOG OF EXPLORATOR	RY BORING			SB-	
			'					PAGE 2 (	DF 2

L: \Graphics \ProjectsByNumber \60-xxxx \60-0626 \Boring Logs \60-626 Log SB-2.0WG Jun 17, 2005 - 8:22am Iwinters



L: \Graphics \ProjectsByNumber \60-xxxx \60-0626 \Boring Logs \60-626 Log SB-3.DWG Jun 16, 2005 - 2:31pm rhughes

PROJE	CT N	0.: 60	DATE DRILLED:	June	8, 2005							
LO	CATI	DN: Foi	rmer 76 Station 2417	LOGGED BY:	Α. Η	elekar, PE						
	·	24	891 Redlands Boulevard	APPROVED BY:	A. H	elekar, PE						
	~~~	Lor	ma Linda, California	DRILLING CO./RIG	: Casc	ade						
BLOWS PER 6 INCHES	(ppmv)	E	B DRILLING METHOD: Hollow Stem Auger SAMPLER TYPE: California Modified TOTAL DEPTH: 51,5 feet DEPTH TO	Split Spoon	USCS GRAPHIC LOG	BOREHOLE BACKFILL						
BLOWS INCF	PID (p	SAMPLE DEPTH (feet below	DESCRIPTION									
16/18/21	0.0					40Bentonite						
13/16/18	0.0		CLAYEY SILT: dark yellowish brown, hard, moist SANDY SILT: dark yellowish brown, hard, wet.		ML							
15/16/ 19/21	0.3	Ţ,	CLAYEY SILT: dark yellowish brown, hard, moist									
						45						
17/19/21	0.0											
		X	Bottom of boring at 51.5 feet below grade.									
						55						
		- - 60				60						
		-70				70						
		-										
						75						
		-80				80-						
T	R	٢	LOG OF EXPLORATO	RY BORING		SB-3						
						PAGE 2 OF 2						

PROJ	ECT N	10.:	60-	-0626	DATE DRILLED:	 J	une	7, 2005
L(	OCATI	ON:	For	ner 76 Station 2417	LOGGED BY:			elekar, PE
			248	91 Redlands Boulevord	APPROVED BY:	β	. Не	elekar, PE
			Lon	na Linda, California	DRILLING CO./RIC			
BLOWS PER 6 INCHES	PID (ppmv)	SAMPLE	DEPTH (feet below grade)	DRILLING METHOD: Hollow Stem Auger SAMPLER TYPE: California Modified S TOTAL DEPTH: 51.5 feet DEPTH TO DESCRIPTION	plit Spoon WATER: 40 feet	USCS	GRAPHIC LOG	BOREHOLE BACKFILL DETAIL
4/6/7	0.4			Air-Knifed to 5 feet. 3-inches of asphaltic concrete. 3-inches to 1.3 feet construction gravel. SILTY SAND: light brown (7.5YR 6/4), slightly mois coarse-grained, construction debris (concrete). SILTY CLAY: very dark grayish brown, stiff.	t, fine- to	SM		0 
5/6/8	0.2			Dark grayish brown.				10
4/5/7 8/10/11	0.4		15  20 20	SAND: light olive brown, medium dense, medium-g graded.	grained, poorly	SP		15 
10/15/17	0.0		25	GRAVELLY SAND: light olive brown, dense, slightly coarse-grained, well graded.	— — — — — — — — — — — — — — — — — — —	SW		
14/17/18	1.8			SILTY SAND: olive brown, dense, slightly moist, fine graded.	e-grained, poorly	SM		
17/18/20 10/40/50 for 6-inches 36/50 for 6-inches	0.0 0.0		- - - - - -	SAND: light olive brown, dense, slightly moist, fine-g graded. SILTY SAND: olive brown, very dense, wet, fine-grai graded. GRAVELLY SAND: gray, very dense, wet, coarse-gr graded.	ned, poorly ained, medium	SP SM: SP	3	
	R	C	r\60-x	LOG OF EXPLORATOR	Y BORING		F	<b>SB-4</b> PAGE 1 OF 2

PROJ	ECT N	lune	7, 2005						
L(		DN:	For	mer 76 Stotion 2417	LOGGED BY:	· · ·	-	elekar, Pl	· · · · · · · · · · · · · · · · · · ·
			248	91 Redlands Boulevard	APPROVED BY:	A	. н	elekar, Pl	
			Lorr	na Linda, Californio	DRILLING CO./RI				
BLOWS PER 6 INCHES	(hpmv)		H below grade)	DRILLING METHOD: Hollow Stem Auger SAMPLER TYPE: California Modified S	-	IC LOG	BOREHOLE BACKFILL		
NCH	, PP	<b>NPLE</b>	DEPTH (feet belo	TOTAL DEPTH: 51.5 feet DEPTH TO	WATER: 40 feet	USCS	<b>GRAPHIC</b>		TAIL
9   9	Did	SAI	DEF (fee	DESCRIPTION		Ω	GR.		
38/40/50 for	0.0		F 40	SANDY SILT: olive brown, hard, moist.		ML		40	-Bentonite Grout
6-inches 30/40/50 for 6-inches				CLAYEY SILT: olive brown, hard, moist.					Grout
36/38/40	0.0	×	- - 45	SILT: with clay.				45	
17/21/34	0.0		E .	CLAY: olive brown, hard, slightly moist. Bottom of boring at 51.5 feet below grade.	····	CL		50	
				bottom of boring at 51.5 reet below grade.					
			— —55						
								55	
			-						
			-						
	:		-60					60-	
			-				ĺ		
			-65					65	
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			-70					70	
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			-80			-+		80	
T	P	C		LOG OF EXPLORATOR	Y BORING			SB-	
								PAGE 2 (	DF 2

PROJECT N	NO.: 60-	-0626	DATE DRILLED:	Ju	ine i	8, 2005
LOCATI	ON: For	mer 76 Station 2417	LOGGED BY:	Α.	Hel	lekor, PE
	248	391 Rediands Boulevard	APPROVED BY:	Α.	Hel	lekor, PE
	Lon	no Linda, California	DRILLING CO./RIG	: Cc	asca	de
н э	grade)	ISAMPLED IVDE Colifornia Madifield C		LOG	BOREHOLE	
BLOWS PER 6 INCHES PID (ppmv)	SAMPLE DEPTH (feet below			SCS	GRAPHIC	BACKFILL DETAIL
			S	Я		
		Air-Knifed to 5 feet. 10-inches of asphaltic concrete.				O-Concrete
		SAND: yellowish brown (10YR 5/4), slightly moist, coarse-grained, trace silt, trace fine gravel.	very fine- to	SP		
4/5/7 10.0		SILTY CLAY: dark brown, stiff.				
6/7/10 2.9		CLAYEY SILT: dark brown, very stiff.		ML	1	
6/7/8 2.6		Brown, with clay.				
6/8/9 1,949	20	SILT: dark brown, very stiff, with clay.			2	
7/9/11 32.5	25	CLAYEY SILT			2	5 1 1 1 1 1 1 1 1 1 1 1 1 1
8/10/12 17.3		SAND: dark brown, medium dense, fine-grained, p	ooorly graded.	5P	30	
14/15/17 8.1					35	5
15/16/20 7.4		SILT: dark brown, hard, slightly moist.		1L		
15/20/50 1.6 for 6-inches		Moist.				_ <u>*</u> ****
TR	<b>C</b>	LOG OF EXPLORATOR	AY BORING	SP	₩ <u>40</u> ₽,	SB-5 Age 1 of 2

PROJ	ECT N	0.:	60-	0626	DATE DRILLED:	J	lune	8, 2005	
L(	OCATIO	DN:	Forr	ner 76 Station 2417	LOGGED BY:	A	ν. Η	elekar, PE	
			248	91 Redlands Boulevard	APPROVED BY:	A	. н	elekar, PE	
			Lom	a Linda, California	DRILLING CO./RI	G: (	) asc	ade	······
			grade)	DRILLING METHOD: Hollow Stem Auger		LOG			
Ц Ц	$\overline{\mathbf{S}}$		a dr	SAMPLER TYPE: California Modified S	plit Spoon	-		BOR	EHOLE
IS P	ppn	Щ	belo belo		WATER: 42.5 feet		H		KFILL
BLOWS PER 6 INCHES	(vmqq) alq	AMF	DEPTH (feet below	DESCRIPTION		USCS	GRAPHIC		TAIL
27/50	16.7		40	GRAVELLY SAND: dark gray, very dense, moist, d	coarse-grained well	sw	1.24913	40	Bentonite
for 6—inches				graded.	granica, non	0"			Grout
20/50 for	19.0	μ	-	SANDY SILT: dark brown, hard, wet.		ML	M		
6-inches		İ	E						
15 /00 /04	0.0	$\vdash$	-45					45	
15/22/24	0.9		F	Moist, no sand.					
· · ·			F						
			F					_;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;	
14/16/18	33.6	T	-50 E	CLAYEY SILT: slightly moist.				50	
		┢┷		Bottom of boring at 51.5 feet below grade.	······································	-			
			F.					55—	
			-					-	
								-	
			-60					60-	
			-						
								-	
			-65					65-	
			-						
			70					70-	
		ĺ	-						
			-75					75	
			-						
			_						
			-						
			-80					80-	
-	R	ſ		LOG OF EXPLORATOR		Ţ		SB-	5
				LOG OF EAFLURATUR				PAGE 2 (	

PROJE	ECT N	0.:	60-	-0626	DATE DRILLED:		lune	7, 2005
LC	CATIC	DN:	Forr	ner 76 Station 2417	LOGGED BY:			elekar, PE
			248	91 Redlands Boulevard	APPROVED BY:			
	<u></u> .		Lorr	ia Linda, California	DRILLING CO./RIG			
EC BROWS PER 6 INCHES	(Audd) Carlo		248 Lorr (PEDIII - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1	91 Redlands Boulevard na Linda, California DRILLING METHOD: Hollow Stem Auger SAMPLER TYPE: California Modified S	APPROVED BY: DRILLING CO./RIG	A	Casco CRAPHIC LOG	elekar, PE
7/14/15	2.2			SAND: dark grayish brown, medium dense, mediur graded. — — — — — — — — — — — — — — — — — —	n-grained, poorly	P		30 + + + + + + + + + + + + + + + + + + +
14/17/18 14/17/4 17/19/ 20/22	6.8 10.8 0.0		- 	SILTY SAND: dark gray, dense, moist. Very dark grayish brown. CLAYEY SILT: dark grayish brown, hard, moist.		SM		
	R	C		LOG OF EXPLORATOR		SW 2	<u>لا الحمد</u> ج	<b>SB-6</b> PAGE 1 OF 2

PROJECT	NO.: 60-	-0626	DATE DRILLED:	June	7, 2005
LOCA	TION: For	mer 76 Station 2417	LOGGED BY:	Α. Η	elekar, PE
	248	91 Redlands Boulevard	APPROVED BY:	<u>А. Н</u>	elekar, PE
	Lon	na Linda, California	DRILLING CO./RIG	: Casc	ade
Harris Sale     (Allowed Sale       SMON     Old       19/21/22     0.0       17/19/22     0.0       15/17/     0.0       19/21     0.0       17/19/22     0.0       17/19/21     0.0	Lorr (epsilon diage) (epsilon diage) (feet pelow diage) (feet pelow diage)	DRILLING METHOD: Hollow Stem Auger SAMPLER TYPE: California Modified S TOTAL DEPTH: 51.5 feet DEPTH TO DESCRIPTION	DRILLING CO./RIG Split Spoon WATER: 41.5 feet m-grained, medium	: Casc	BOREHOLE BACKFILL DETAIL 40
	60 				60- 
TR	26	LOG OF EXPLORATO	RY BORING		SB-6 PAGE 2 OF 2

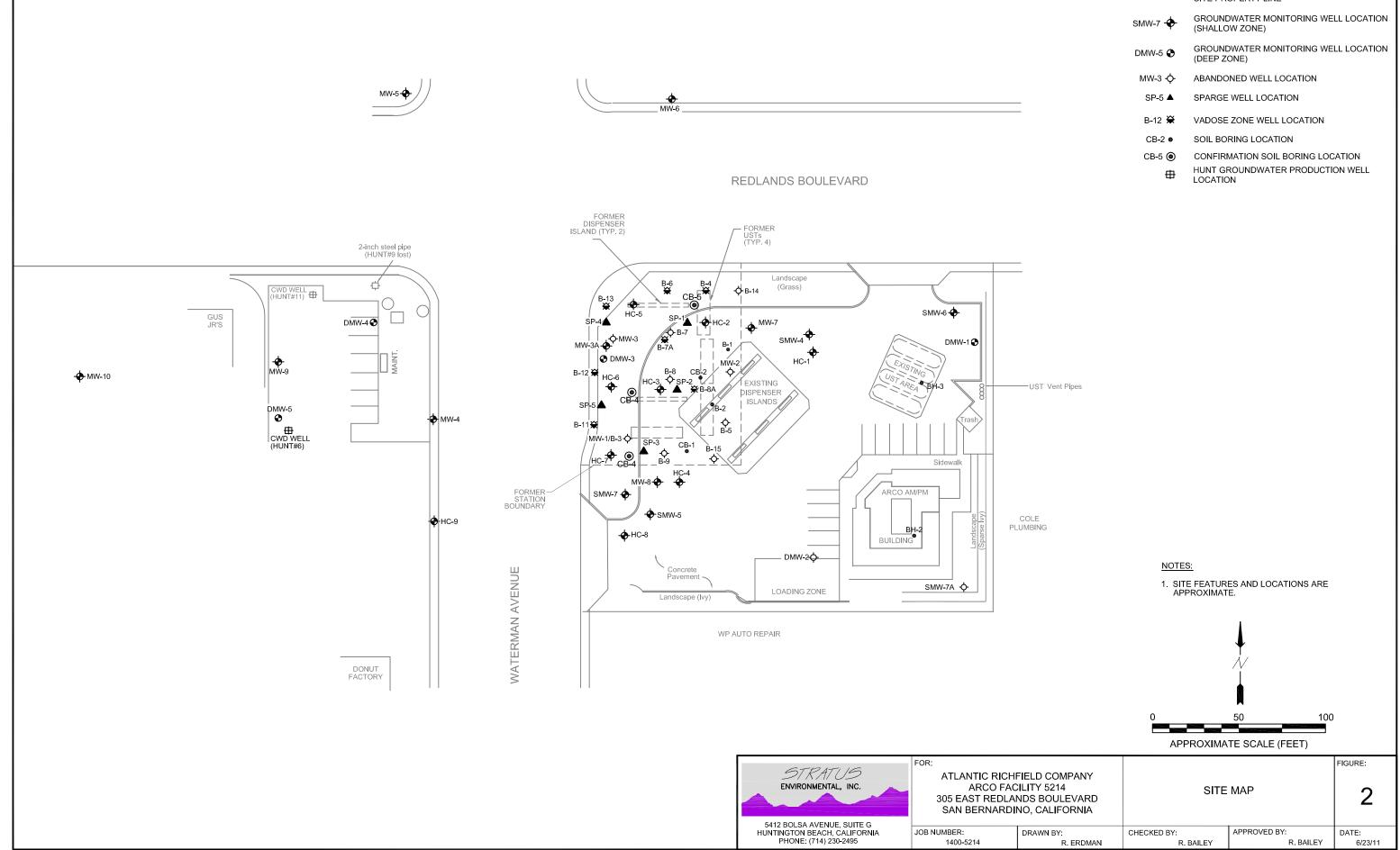
PROJECT NO .: 60-	-0626	DATE DRILLED:	June 8, 2005
LOCATION: For	mer 76 Station 2417	LOGGED BY:	A. Helekar, PE
248	391 Redlands Boulevard	APPROVED BY:	A. Helekar, PE
Lon	na Linda, California	DRILLING CO./RIG:	Cascade
BLOWS PER 6 INCHES PID (ppmv) SAMPLE 1'1 DEPTH (feet below grade)	SAMPLER TYPE: California Modified S TOTAL DEPTH: 51.5 feet DEPTH TO	WATER 115 feet	SUSS 0 0 0 0 0 0 0 0 0 0 0 0 0
	SAND: dark yellowish brown (10YR 4/4), slightly n coarse-grained, some fine gravel. SANDY GRAVEL: dark yellowish brown (10YR 4/ gravel is fine-grained. PEA GRAVEL		SW
	SAND: yellowish brown, dense, fine-grained, poor	y graded.	
	SILTY CLAY: dark yellowish brown, hard.	c	CL 25-100000 -1000000 -1000000 -1000000 -1000000 -1000000 -1000000 -1000000 -1000000 -1000000 -1000000 -1000000 -1000000 -1000000 -1000000 -1000000 -1000000 -1000000 -1000000 -1000000 -1000000 -1000000 -10000000 -100000000
10/15/16 0.0	SILT: dark gray, medium hard.	М	1L
14/15/18 0.0	Slightly moist, trace clay.		
	Moist, some clay.		
14/15/ 0.0 17/22	SAND: dark gray, dense, moist, fine-grained, poor	y graded. S	
TRC	LOG OF EXPLORATOF		40

PROJE	ECT N	0.:	60-	-0626	DATE DRILLED:	J	une	8, 2005	<u> </u>
LC	DCATIO	DN:	For	mer 76 Station 2417	LOGGED BY:	A	. Н	elekar, PE	-
			248	91 Redlands Boulevard	APPROVED BY:	Α	<u>. н</u>	elekar, PE	
			Lorr	a Linda, California	DRILLING CO./RIC	9: C	asc	ade	
BLOWS PER 6 INCHES	(vmqq) UIA	SAMPLE	DEPTH (feet below grade)	DRILLING METHOD: Hollow Stem Auger SAMPLER TYPE: California Modified S TOTAL DEPTH: 51.5 feet DEPTH TO DESCRIPTION	plit Spoon WATER: 41.5 feet	USCS	GRAPHIC LOG	BAC	EHOLE KFILL TAIL
13/15/17 13/17/19	0.0 0.0			GRAVELLY SAND: dark gray, dense, wet, coarse- graded.	grained, well	SP SW		40-2000	- Bentonite Grout
15/17/18	0.0			SILT: dark yellowish brown, hard, slightly moist, w	ith clay.	ML		45	
14/16/18	0.0		50 	Moist. Bottom of boring at 51.5 feet below grade.				50	
			- - - - - - -					55	
			-60 					60	
			- - - - - - - - 70					70	
			75				an de la constante	75	
T	R	C		LOG OF EXPLORATOR	Y BORING		F	SB-	1



# **APPENDIX C2.9**

ARCO #5214 GeoTracker Case I.D. T0607100180



LEGEND	
	SITE PROPERTY LINE
sмw-7 - <del>ф</del>	GROUNDWATER MONITORING WELL LOCATION (SHALLOW ZONE)
DMW-5	GROUNDWATER MONITORING WELL LOCATION (DEEP ZONE)
мw-з -ф-	ABANDONED WELL LOCATION
SP-5 🔺	SPARGE WELL LOCATION
B-12 💥	VADOSE ZONE WELL LOCATION
CB-2 •	SOIL BORING LOCATION
CB-5 🔘	CONFIRMATION SOIL BORING LOCATION
н н	HUNT GROUNDWATER PRODUCTION WELL

		ate <u>Decembe</u> Iler JDK Drilli		_					Client         ARCO 5214           Address         305 Redlands Blvd.			
<u> </u>						Addr						
			ing Forem	_ Drillin	CA		San Berna					
	hole diam.: 8"	od Limited A		•			1400-5214					
		ng: 30 feet bg		Dept		па	Steve Kali	ed By:	Logo			
	7	ill: Liquid gro		•								
	<u>L</u>	er: 22 feet bg	oth to Wat	. Dept								
PI	Descriptions of Materials		LITHC	Depth	mple	Sai	Blow	ample				
(PP	and Conditions		COLUM	Scale	Recov.	-600080-6	Count		Туре			
			1	<sup>1</sup>								
			2	2								
	Cleared 0 - 5 feet with Air Knife			— <u> </u>								
				— <sup>3</sup>		•••••						
[			4	4								
75.	rk olive grey (5Y 3/2), 80% very fine to fine sand, some	SILTV SA	5 SM	X X 5			3 4					
''	d, moderately sorted, loose, moist.			x <sup>-</sup>	100%	9:15	<del>4</del> 5	CB-3-5'	E			
		1	6	6								
1			7	— "								
			'	— '								
1			3	8								
				g								
				x			4					
3.4	e grey (5Y 4/2), 50% very fine to fine sand, some silt,	SILTY SA	SM	X <u>1</u> 0			6					
	oderately sorted, medium dense, damp.	trace med		×	100%	9:20	9	CB-3-10'	Е			
				<u> </u>								
			2	1 2								
ĺ				<u> </u>								
			ŧ	1 4								
10.4	e brown (2.5Y 4/4), 80% fine sand, some silt, moderately	SIL TV SAL	SM	$ \begin{array}{r} 1 \\ x \\ x \\ x \\ x \\ x \\ x \end{array} $			5 8					
'0.	damp.	sorted, me			100%	9:25	13	CB-3-15'	E			
				1 6								
			, .	<u> </u>								
			3	1 8								
		5		1 9								
			' [	x			5					
				x 2 0			9					
	nlanter along Waterman Avenue midwey between welle	Comment Boring loc					Sample	core Soil S				
	planter along Waterman Avenue midway between wells	HC-7 and						icore Soli a imple Inter				
									2.			
	and the second second second second second second second second second second second second second second secon											
	5787705											
i Birina												
and the second second second second second second second second second second second second second second second		1										

<u>....</u>

so	IL BORIN	IG LOG			Borin	g No.	CB-3 Sh	eet 2 of 2
Clie	nt	ARCO 52	14			Dat	te December 17, 2007	
	ress	305 Redia		/d.	-		er JDK Drilling, Inc.	
		San Bern			- Drillin	g Forema		
Proi	ect No.	1400-521			-		d Limited Access HSA hole diam.: 8"	
-	ged By:	Steve Ka			- Dept		g: 30 feet bgs	
					Depti		r: 22 feet bgs	
					-		·	
	Sample	Blow	Sa	mple	Depth	LITHO	Descriptions of Materials	PID
Туре		Count		Recov.	Scale		and Conditions	(PPM)
E 	CB-3-20'	15	9:30	100%	1	SP	SAND (SP), dark grey (2.5Y 4/1), 80% fine to medium sand, some coarse sand, well sorted, medium dense, damp.	0.0
					2 2	. ▼		
					2 3 2 4 X 2 X 2 X 5			
		4			x <sup>24</sup>			
E	CB-3-25'	6 9	9:50	20%	IX I	ML	SANDY SILT (ML), dark olive brown (5Y 3/2), 85% silt, some fine sand, tra medium sand, medium stiff, wet.	ce 0.0
					2 6			
					2 7 2 6			
					_2 6			
		8			x <sup>29</sup>			
Ē	CB-3-30'	12 17	10:00	100%	x <u>3</u> 0 x	ML	SANDY SILT (ML), dark olive brown (5Y 3/2), 70% silt, some fine sand, tra medium sand, stiff, moist.	ce 0.0
		 			<u> </u>			
					3 2			
		ļ			33			
					$\frac{3}{-3}$			
		   			$\frac{-3}{37}$			
					$     \begin{array}{r}         3 4 \\         3 5 \\         3 6 \\         3 7 \\         3 8 \\         3 9 \\         4 0     \end{array} $			
					3 9			
					4 0			
E = E X = S	ncore Soil S ample Inter	l Sample val	<u>                                     </u>	1	I		Comments: Boring located in grass planter along Waterman Avenue midway between v HC-7 and SP-3.	vells
							STRATUS	

·

so	dress 305 Redlands B San Bernardino bject No. 1400-5214-01			Borin	g No. C	CB-4 Sheet 1	of 2	
Clie	Int <u>ARCO 5214</u> Iress <u>305 Redlands Blvo</u> San Bernardino, C lect No. <u>1400-5214-01</u>					Date	December 17, 2007	
Add	ress			vd.	-		r JDK Drilling, Inc.	
					- Driilin	ng Foreman		
Proj	ect No.				-		Limited Access HSA hole diam.: 8"	
-	ged By:	Steve Kal			- Depi	th of boring:	25 feet bgs	
							: Liquid grout	
					Dept		22 feet bgs X	
·					•••			
					-			
	Sample	Blow	Sa	imple	Depth	LITHO	Descriptions of Materials	PID
Туре	No.	Count	Time	Recov.	Scale	COLUMN	and Conditions	(PPM)
					— ,			
			·}	+	1			
					2			
					— <u> </u>		Cleared 0 - 5 feet with Air Knife	
			•   • • • • • • • • • • • • • • • • • •	•	— <sup>3</sup>			
					4			
		5			×	SM		00 F
E	CB-4-5'	8 9	8:15	100%	X5 x5	SIVI	SILTY SAND (SM), dark olive grey (5Y 3/2), 80% very fine to fine sand, some silt, trace medium sand, moderately sorted, medium dense, moist.	20.5
		Ŭ			6			
					- '			
	1							
					~9			
		9 12			$\hat{x}_{1}^{-1}$	SM	SILTY SAND (SM), olive (5Y 4/3), 70% very fine to fine sand, some silt, trace	2.1
E	CB-4-10'	18	8:20	100%	X <u>1</u> 0 X_		medium sand, moderately sorted, dense, damp.	
					1 1			
					<u> </u>			
			+					1
					1 3			
					1 4			
		8			x			1
		12			X <u>1</u> 5	ML	SANDY SILT (ML), olive brown (2.5Y 4/4), 80% silt, some fine sand, moderately	8.4
E	CB-4-15'	12	8:25	100%	X6		sorted, medium dense, damp.	
					<u> </u>			[
		<u>-</u>		ļ	1 9			[
		7 11			$\frac{X}{X 2 0}$			
			l	I	<u>,, , , , , , , , , , , , , , , , , , ,</u>		Comments:	
	ncore Soil :						Boring located in grass planter along Waterman Avenue midway between wells	
x = S	ample Inter	rval					HC-6 and HC-3.	
							578A745	
							TAVIZ (IABLE ALC. P.C.	

SOIL BOR	ING LOG	Boring No.	СВ-4		Sheet 2 of 2	
Client	ARCO 5214	Date	December 17, 2007			
Address	305 Redlands Blvd.	Driller	JDK Drilling, Inc.			
	San Bernardino, CA	Drilling Foremar	Troy			
Project No.	1400-5214-01	Method	Limited Access HSA	hole diam.: 8"		
Logged By:	Steve Kalina	Depth of boring	: 25 feet bgs			
		Backfill:	Liquid grout			
		Depth to Water:	22 feet bgs			

	Sample	Blow	Sa	mple	Depth	LITHO	Descriptions of Materials	PID
Туре	No.	Count	Time	Recov.	Scale	COLUMN	and Conditions	(PPM
E	CB-4-20'	17	8:30		X1 2 1 2 2	SP V	SAND (SP), dark grey (2.5Y 4/1), 90% fine to medium sand, little silt, trace coarse sand, well sorted, medium dense, moist.	0.0
E	CB-4-25'	4 6 9	9:50	20%	$\begin{array}{c} 2 & 3 \\ 2 & 4 \\ x & 2 \\ x & 2 \\ x \\ x \\ x \end{array}$	SP	SAND (SP), dark grey (2.5Y 4/1), 90% fine to medium sand, little silt, trace coarse sand, well sorted, medium dense, wet.	0.0
	CB-4-20	9	9.50	2076	2 6 2 7		Coarse sand, wen solled, medium dense, wel.	
					2 8 2 9 3 0			- -
					$\frac{3}{3}1$			
					$\frac{3}{3} 3$ $\frac{3}{3} 4$			:
					$\frac{3}{3}5$			
					$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$			
					$\frac{3}{4} 9$			
= Er = Sa	ncore Soil S ample Interv	ample al	L				<b>Comments:</b> Boring located in grass planter along Waterman Avenue midway between wells HC-6 and HC-3.	; ;



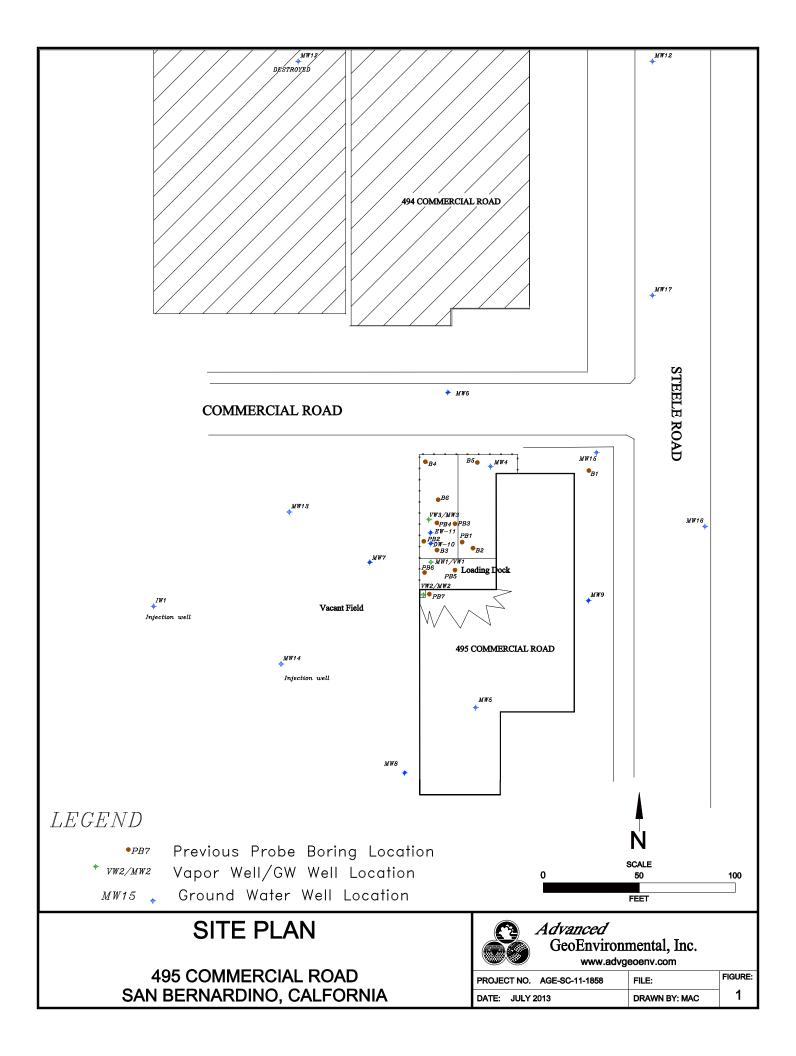
SOIL BOI	RING LOG	i		Borin	g No. C	CB-5 Sheet	1 of 2
Client	ARCO 52	214			Date	December 17, 2007	
Address	305 Redi	ands Blvd	ł.	•		- JDK Drilling, Inc.	
	San Bern	ardino, C	A	Drillin	ig Foreman	n Troy	
Project No.	1400-521			,		Limited Access HSA hole diam.: 8"	
_ogged By:	Steve Ka	lina		Dep	th of boring:	25 feet bgs	
					Backfill:	Liquid grout	
				Dept		22 feet bgs	
Sample	Blow	Sample		Depth	LITHO	Descriptions of Materials	PID
Type No.	Count	Time R	00000000000	Scale	COLUMN	and Conditions	(PPM)
				. <u> </u>			
				1			
				2			
		1				Cleared 0 - 5 feet with Air Knife	
				3			
				— <sub>4</sub>			
	6			x			
	6 5'7	10:45	4009/	X 5	ML	SANDY SILT (ML), olive brown (2.5Y 4/4), 80% silt, some fine sand, moderately sorted, medium dense, damp.	0.0
E CB-5-		10:45	100%	×6		soned, medium dense, damp.	
		·}					
				7			
				— <sub>8</sub>			
		·		°			
				9			
	7			X0 X0	SM	SILTY SAND (SM), olive (5Y 4/3), 70% very fine to fine sand, some silt, trace	0.0
E CB-5-1		10:50	100%	x		medium sand, moderately sorted, medium dense, damp.	0.0
				<u>1</u> 1			
				1 2	1 mm		
				1 3	-		
		·		$x \frac{1}{1} 4$			
	10			x <u>1</u> 5	ML	SANDY SILT (ML), olive brown (2.5Y 4/4), 80% silt, some very fine to fine sand,	21.1
E CB-5-1	5' 12	10:55	100%	Х		moderately sorted, medium dense, damp.	
				1 6			
				1 7			
				<u>1</u> 7 <u>1</u> 8			
		<u> </u>		8			•
				9			
	8	[		x			
	9	I	<u> </u>	X 2 0		Comments:	
= Encore Se	il Sample					Boring located in grass planter along Redland Boulevard approximately 10 feet	
= Sample Ir						north of sparge well SP-1.	
						STRATUS	
						and the second second second second second second second second second second second second second second secon	

so	OIL BORING LOG           lient         ARCO 5214           ddress         305 Redlands Blvg				Borin	g No.	CB-5 Sheet 2	2 of 2
Clie	nt	ARCO 52	14			Dat	te December 17, 2007	
				/d.	•		er JDK Drilling, Inc.	
		San Bern			Drilline	g Forema		
Proi	ect No.	1400-521		0,1	- 2000		d Limited Access HSA hole diam.: 8"	
-	ged By:	Steve Ka			- Dent		g: 25 feet bgs	
	ged by.	01676114			Dept			
							II: Liquid grout	
<u> </u>					Depti	i to Water	r: 22 feet bgs	
								10.000.0000
	Sample	Blow	Sa	mple	Depth	LITHO COLUMN	Descriptions of Materials	PID
Туре	No.	Count		Recov.	Scale		and Conditions	(PPM)
Е	CB-5-20'	19	11:00	100%	X1	SM	SILTY SAND (SM), dark olive grey (2.5Y 3/3), 70% fine to medium sand, some silt, trace coarse sand, moderately sorted, medium dense, moist.	0.0
					2 2			
					2 3			
			1		1	-		
					<u>2</u> 4			
		6 9			X 5	SP	SAND (SP), dark grey (2.5Y 4/1), 90% fine to medium sand, little silt, trace	0.0
E	CB-5-25'	11	11:05	100%	· ^		coarse sand, well sorted, medium dense, wet.	
					2 6			
					7			
	<b> </b>				_2 8			
					2 9			
		10			X			
		13			<u>х з</u> о	SP	SAND (SP), dark grey (2.5Y 4/1), 95% fine to medium sand, trace silt, well	0.0
Е	CB-5-30'	16	11:10	100%	X1		sorted, medium dense, wet.	
					3 2			
					33			
		<b> </b>	<b> </b>		3 4			
					3 5			
		1	11					
		<b> </b>	<u> </u>		3 6			
					$\frac{3}{3}7$			
		<b> </b>	<u> </u>					
					39			
				Ē				
					4 0			
E≂E X≃S	ncore Soil S ample Inter	Sample val	L				Comments: Boring located in grass planter along Redland Boulevard approximately 10 feet north of sparge well SP-1.	
							STRATUS	

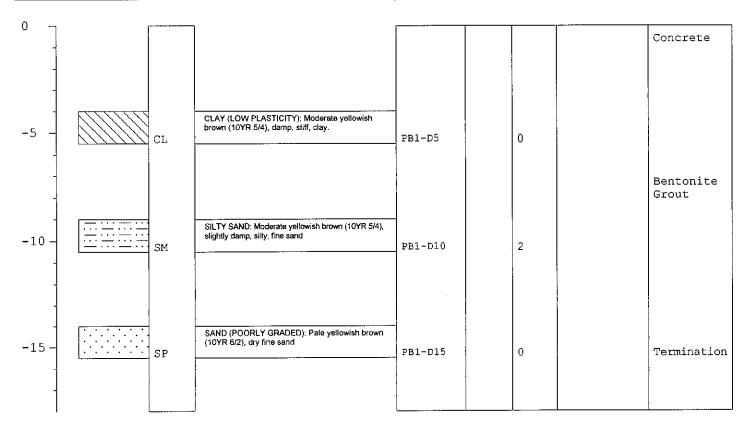


# **APPENDIX C2.10**

Eric Realty Property GeoTracker Case I.D. T10000001230



#### **FIELD BORING LOG** Advanced GeoEnvironmental, Inc. Boring No.: **PB-1** 3315 East Miraloma Avenue, Suite 117 Total Depth: 15 Feet Anaheim, California 92806 **PROJECT INFORMATION DRILLING INFORMATION** Drilling Co.: Project: **EnviroProbe Eric Realty Property** Site Location: Driller: Art San Bernardino Rig Type: Geoprobe 5400 Job No.: SB 607A7.930 Method of Drilling: **Direct Push** Logged By: **D. Becker** Project Manager: Sampling Methods: **Direct Push R.** Loeffler Dates Drilled: 12/14/01 Hammer Wt./Drop Water level during drilling NOTES: Q Water level in completed well T PID SOIL USCS SAMPLE Blows BORING WELL DEPTH SOIL DESCRIPTION ppm SYMBOLS NUMBER /6 in. COMPLETION DESC.

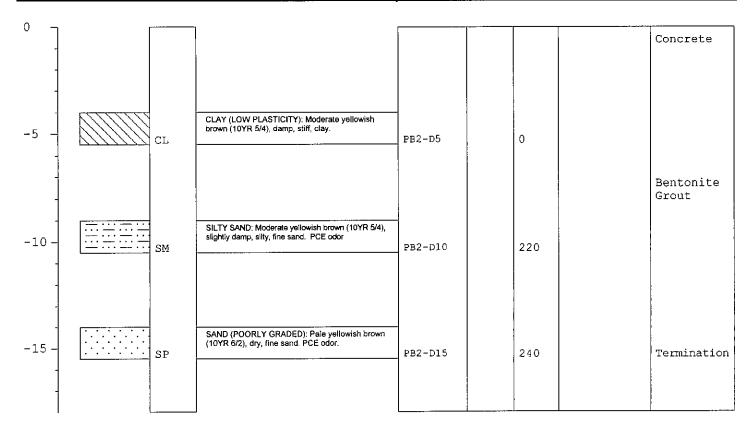




## **FIELD BORING LOG**

Boring No.: Total Depth: PB-2 15 Feet

	PROJEC'	T INF	ORMATION		D	RILLI	NG IN	FORMATIO	N	
Project:		E	ric Realty Property	D	rilling Co.:		F	EnviroProbe		
Site Loca	Site Location: San Bernardino		an Bernardino	D	riller:		A	Art		
Job No.:	Job No.: SB 607A7.930			Rig Type:				Geoprobe 5400		
Logged By: <b>D. Becker</b>			Method of Drilling:			I	Direct Push			
Project N	lanager:	R	. Loeffler	Sa	ampling Me	ethods:	I	)irect Push		
Dates Dr	illed:	1	2/14/01	Hammer Wt./Drop						
NOTES:						er level du er level in	-	-		
DEPTH	SOIL SYMBOLS	USCS	SOIL DESCRIPTION		SAMPLE NUMBER	Blows /6 in.	PID ppm	BORING COMPLETION	WELL DESC.	

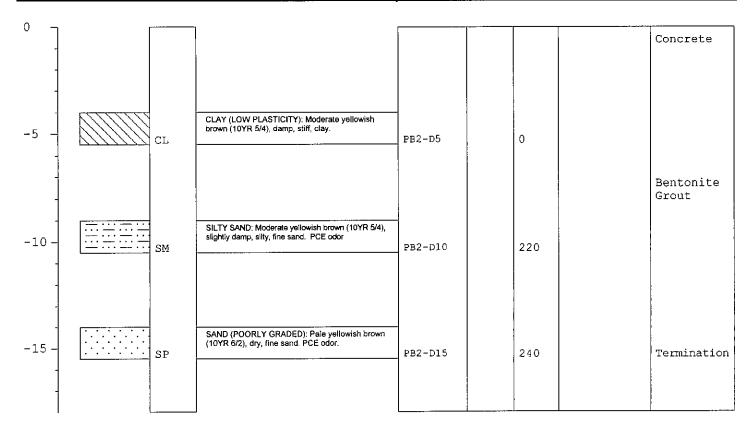




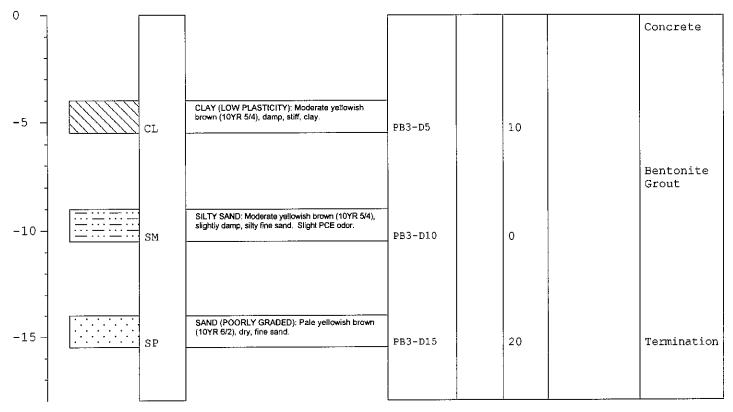
## **FIELD BORING LOG**

Boring No.: Total Depth: PB-2 15 Feet

	PROJEC'	T INF	ORMATION		D	RILLI	NG IN	FORMATIO	N	
Project:		E	ric Realty Property	D	rilling Co.:		F	EnviroProbe		
Site Loca	Site Location: San Bernardino		an Bernardino	D	riller:		A	Art		
Job No.:	Job No.: SB 607A7.930			Rig Type:				Geoprobe 5400		
Logged By: <b>D. Becker</b>			Method of Drilling:			I	Direct Push			
Project N	lanager:	R	. Loeffler	Sa	ampling Me	ethods:	I	)irect Push		
Dates Dr	illed:	1	2/14/01	Hammer Wt./Drop						
NOTES:						er level du er level in	-	-		
DEPTH	SOIL SYMBOLS	USCS	SOIL DESCRIPTION		SAMPLE NUMBER	Blows /6 in.	PID ppm	BORING COMPLETION	WELL DESC.	



	<b>Idvanced</b> <b>GeoEnvironmental, I</b> 3315 East Miraloma Avenue, Suite Anaheim, California 92806			Bori	LD ng N l Dep			
PROJECT	INFORMATION	DRILLING INFORMATION						
Project:	Eric Realty Property	Drilling	Co.:		E	InviroProbe		
Site Location:	San Bernardino	Driller:				Art		
Job No.:	SB 607A7.930	Rig Typ	be:		0	Geoprobe 5400		
Logged By:	D. Becker	Method of Drilling:				Direct Push		
Project Manager:	R. Loeffler	Sampling Methods: Direct Push						
Dates Drilled:	12/14/01	Hamme	r Wt./D	Drop				
NOTES:		∑ ¥		evel duri evel in c	-	-	-	
DEPTH SOIL U SYMBOLS	JSCS SOIL DESCRIPTION	SAM NUN		Blows /6 in.	PID ppm	BORING COMPLETION	WELL DESC.	



# **FIELD BORING LOG**

Boring No.: Total Depth: PB-4 15 Feet

	PROJEC	T INFO	ORMATION		D	RILLI	NG IN	<b>FORMATIO</b>	N	
Project:		Ε	ric Realty Property	D	rilling Co.:		J	EnviroProbe		
Site Loca	ation:	S	an Bernardino	Driller:			ł	Art		
Job No.:	Job No.: <b>SB 607A7.930</b>			Rig Type:				Geoprobe 5400		
Logged I	Logged By: <b>D. Becker</b>				Method of Drilling:			Direct Push		
Project N	Aanager:	R	. Loeffler	Sampling Methods:			I	Direct Push		
Dates Dr	illed:	1	2/14/01	Hammer Wt./Drop						
NOTES:						er level du er level in	-			
DEPTH	SOIL SYMBOLS	USCS	SOIL DESCRIPTION		SAMPLE NUMBER	Blows /6 in.	PID ppm	BORING COMPLETION	WELL DESC.	

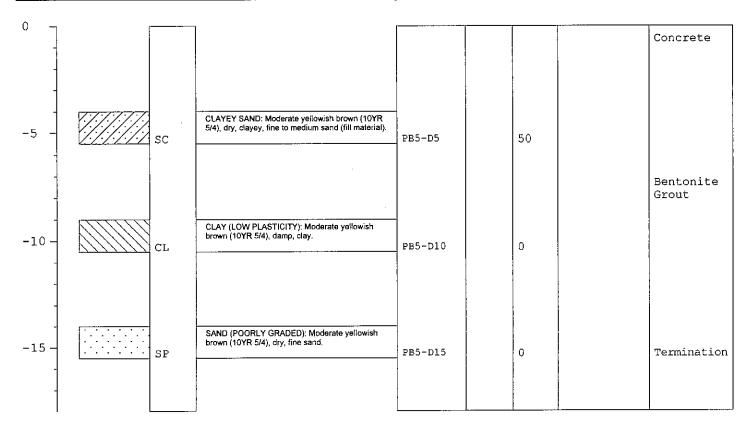
					Concrete
	CL	CLAY (LOW PLASTICITY): Moderate yellowish brown (10YR 5/4), damp, stiff, clay.	PB3-D5	10	Bento <b>n</b> ite Grout
-10 -	<u></u>	SILTY SAND: Moderate yellowish brown (10YR 5/4), slightly damp, silty fine sand. Slight PCE odor.	₽B3-D10	0	
-15		SAND (POORLY GRADED): Pale yellowish brown (10YR 6/2), dry, fine sand.	PB3-D15	20	Termination



#### FIELD BORING LOG

Boring No.: Total Depth: PB-5 15 Feet

	PROJEC	T INF(	ORMATION		D	RILLI	NG IN	FORMATIO	N	
Project:		Е	ric Realty Property	D	rilling Co.:		I	EnviroProbe		
Site Loca	ation:	S	an Bernardino	Driller:			F	Robert		
Job No.:	SB 607A7.930			Rig Type:				Power Probe		
Logged I	Logged By: D. Becker				Method of Drilling:			Direct Push		
Project N	Manager:	R	. Loeffler	Sampling Methods:			Ι	Direct Push		
Dates Dr	rilled:	12	2/14/01	Hammer Wt./Drop						
NOTES:				<ul> <li>✓ Water level during drilling</li> <li>✓ Water level in completed well</li> </ul>						
DEPTH	SOIL SYMBOLS	USCS	SOIL DESCRIPTION		SAMPLE NUMBER	Blows /6 in.	PID ppm	BORING COMPLETION	WELL DESC.	



### FIELD BORING LOG

Boring No.: Total Depth: PB-6 20 Feet

	PROJEC	T INFO	ORMATION		D	RILLI	NG IN	FORMATIO	N	
Project:		Ε	ric Realty Property	D	rilling Co.:		I	EnviroProbe		
Site Loca	Site Location: San Bernardino		an Bernardino	Driller:			I	Robert		
Job No.:	Job No.: <b>SB 607A7.930</b>			Rig Type:				Power Probe		
Logged I	Logged By: <b>D. Becker</b>				Method of Drilling:			Direct Push		
Project N	/lanager:	R	. Loeffler	Sa	ampling Me	ethods:	I	Direct Push		
Dates Dr	illed:	12	2/14/01	Hammer Wt./Drop						
NOTES:						er level du er level in	•	-		
DEPTH SOIL USCS SYMBOLS		SOIL DESCRIPTION	L DESCRIPTION		Blows /6 in.	PID ppm	BORING COMPLETION	WELL DESC.		

0 ]	ſ				:	1	Concrete
5		SC	CLAYEY SAND: Moderate yellowish brown (10YR 5/4), dry, clayey fine to medium sand. PCE odor.	PB6~D5		100	
-10		CL	CLAY (LOW PLASTICITY): Moderate yellowish brown (10YR 5/4), damp, clay.	PB6-D10		50	Bentonite
-15 - -		SP	SAND (POORLY GRADED): Pale yellowish brown (10YR 6/2), dry, fine sand. PCE odor.	PB6-D15		100	Grout
- -20 - -		CL	CLAY (LOW PLASTICITY): Moderate yellowish brown (10YR 5/4), damp clay. PCE odor.	PB6-D20		110	Termination

### FIELD BORING LOG

Boring No.: Total Depth: PB-6 20 Feet

	PROJEC	T INFO	ORMATION	DRILLING INFORMATION						
Project:		Ε	ric Realty Property	Drilling Co.:			I	EnviroProbe		
Site Loca	Site Location: San Bernardino		Driller:			I	Robert			
Job No.: SB 607A7.930		Rig Type:			I	Power Probe				
Logged By: <b>D. Becker</b>		Method of Drilling:			I	Direct Push				
Project N	/lanager:	R	. Loeffler	Sampling Methods:			I	Direct Push		
Dates Dr	illed:	12	2/14/01	Hammer Wt./Drop						
NOTES:	NOTES:					er level du er level in	•	-		
DEPTH SOIL USCS SOIL DESCRIPTION			SAMPLE NUMBER	Blows /6 in.	PID ppm	BORING COMPLETION	WELL DESC.			

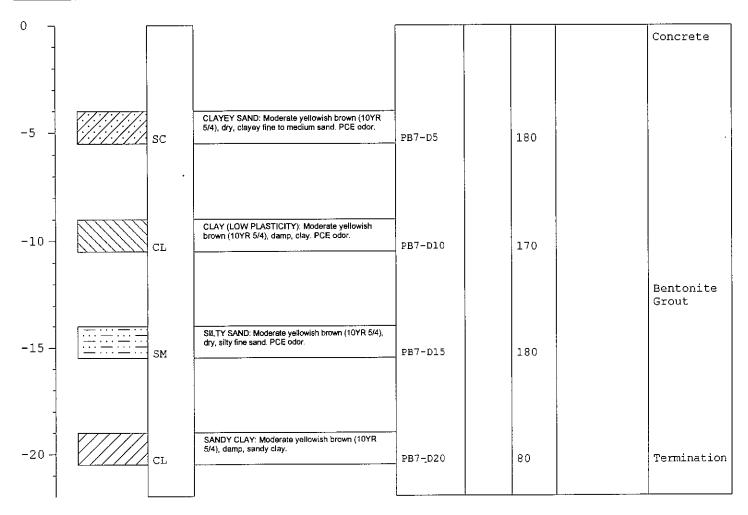
0 ]	ſ				:	1	Concrete
5		SC	CLAYEY SAND: Moderate yellowish brown (10YR 5/4), dry, clayey fine to medium sand. PCE odor.	PB6~D5		100	
-10		CL	CLAY (LOW PLASTICITY): Moderate yellowish brown (10YR 5/4), damp, clay.	PB6-D10		50	Bentonite
-15 - -		SP	SAND (POORLY GRADED): Pale yellowish brown (10YR 6/2), dry, fine sand. PCE odor.	PB6-D15		100	Grout
- -20 - -		CL	CLAY (LOW PLASTICITY): Moderate yellowish brown (10YR 5/4), damp clay. PCE odor.	PB6-D20		110	Termination



## FIELD BORING LOG

Boring No.: Total Depth: PB-7 20 Feet

1	PROJEC	T INFO	ORMATION	DRILLING INFORMATION						
Project:		Ε	ric Realty Property	Drilling Co.:			ŀ	EnviroProbe		
Site Loca	Site Location: San Bernardino		an Bernardino	Driller:			F	Robert		
Job No.: SB 607A7.930		Rig Type:			F	Power Probe				
Logged I	Logged By: <b>D. Becker</b>		Method of Drilling:			Ι	Direct Push			
Project N	/anager:	R	. Loeffler	Sampling Methods:			Ι	Direct Push		
Dates Dr	illed:	12	2/14/01	Hammer Wt./Drop						
NOTES:				<ul> <li>Water level during drilling</li> <li>Water level in completed well</li> </ul>						
DEPTH	SOIL SYMBOLS	USCS	SOIL DESCRIPTION		SAMPLE NUMBER	Blows /6 in.	PID ppm	BORING COMPLETION	WELL DESC.	



		Ge 3315	enced oEnvironmental, In East Miraloma Avenue, Suite neim, California 92806		1	SIELI Boring Cotal D		G LOG B8 50 feet			
	PROJEC	T INF	ORMATION	•	DRILLING INFORMATION						
Project	t:	F	Eric Realty	Drillin	g Co.:		Cascade Dri	lling, Inc.			
Site Lo	ite Location: 495 East Commercial Re						Steve				
Job No	ob No.: SB 607A7.930				pe:		CME 75 LA	R			
Logged	Logged By: Diane Becker				l of Drillin	ng:	10 in. Hollow	v Stem Auger			
Project	Project Manager: Diane Becker				ng Metho	ts:	California S	plit Spoon			
Dates 1	Dates Drilled: 07-11-02				er Wt./Dro	p	140 LB., 30 I	IN.			
NOTE	S:				Water leve Water leve	l during di l in compl	—	-			
DEPTH	I SOIL SYMBOLS	USCS	SOIL DESCRIPTION	SAN NUN		ws PID in. ppn					
0		ML	SANDY SILT (LOW PLASTICITY): Moderate yeliowish brown (10YR 5/4), dry, sandy silt SILT (LOW PLASTICITY): Moderate yeliowish (10YR 5/4), dry, silt	B8-D		/50 0 50 0					
- 15 -		SM	SILTY SAND: Moderate yellowish brown (10YF dry, sity fine sand	B8-D	18,/22	<b>/32</b> 0					
-20 -		CL	SANDY CLAY: Moderate yellowish brown (10 5/4), alightly damp, sandy, ality clay	<b>ҮР</b> В8-D7	20 12/16	/22 0					
-25-		CL	SANDY CLAY: Moderate yellowish brown (10 5/4), damp, sandy clay	YR	5 6/9	/8 0					

	Advanced GeoEnvironmental, In 3315 East Miraloma Avenue, Suite Anaheim, California 92806		ING LOG B8 50 feet				
PROJEC	T INFORMATION		DRILLI	NG INFORM	IATION		
Project:	Eric Realty	Drilling	Drilling, Inc.				
Site Location:	495 East Commercial Rd.	Driller:	Driller: Steve				
Job No.:	SB 607A7.930	Rig Typ	Type: CME 75 LAR				
Logged By:	Diane Becker	Method	of Drilling: 10 in. Hollow Stem A				
Project Manager:	Diane Becker	Samplir	ng Methods:	-			
Dates Drilled:	07-11-02	Hamme	r Wt./Drop	140 LB.,	30 IN.		
NOTES:		<ul> <li>Water level during drilling</li> <li>Water level in completed well</li> </ul>			-		
DEPTH SOIL SYMBOLS	USCS SOIL DESCRIPTION		SAMPLE         Blows         PID         BORING           NUMBER         /6 in.         ppm         COMPLETION				

- 30 -	CL	SANDY CLAY: Moderate yellowish brown (10YR 5/4), damp, sandy clay	B8-D30	17/18/21	0	
- 35 -	SM	SILTY SAND: Moderate yellowish brown (10YR 5/4), damp, silly fine sand	B8-D35	36/50	0	
-40 -	SC	CLAYEY SAND: Moderate yellowish brown (10YR 5/4), damp, clayey fine sand	B8-D40	16/50	0	
-45 -	CL	SANDY CLAY: Moderate yellowish brown (10YR 5/4), damp, sandy clay	B8-D45	16/19/25	0	
- 50 -	CL	SANDY CLAY: Moderate yellowish brown (10YR 5/4), wet, sandy clay	B8-D50	17/21/22	0	
-55 -			24			

		Ge 3315	enced oEnvironmental, II East Miraloma Avenue, Suite neim, California 92806			Boi	ELD ring N al Dej		LOG W1/VW1 feet
	PROJEC	T INF	ORMATION			RILL	ING IN	FORMATIO	N
Project:			Cric Realty	Drilling				Cascade Drillin	
Site Loc	ation:		95 East Commercial Rd.	Driller:	-			Steve	5,
Job No.:		S	B 607A7.930	Rig Ty	De:		(	CME 75 LAR	
Logged By: Diane Becker				Method	-	rilling:		l0 in. Hollow S	tem Auger
Project Manager: Diane Becker				Sampli		-		California Split	-
Dates Drilled: 07-10-02				Hamme	-			140 LB., 30 IN.	-
NOTES:					Wate	r level du	ring drill	ing	-
DEPTH	SOIL SYMBOLS	USCS	SOIL DESCRIPTION		IPLE 1BER	Blows /6 in.	r ***	BORING COMPLETION	WELL DESC.
- 25 -		CL	SANDY CLAY: Moderate yellowish brown (10 5/4), slightly damp, sandy clay, leas than 10% g		D25	7/11/18	0		
-30		SM	SILTY SAND: Moderate yellowish brown (10YR slightly damp, silty fine sand	15/4), MW1 - I	530	7/8/9	0		
- 35 -		SC	SILTY SAND: Moderate yellowish brown (10YR sightly damp, ally, clayey fine sand	5/4), MW1-I	535	15/17/21	0		
-40 -			No Recovery - rock fragments in shoe			60	0		
-45 -		CL	CLAYEY SILT (LOW PLASTICITY): Dark yelk brown (10YR 4/4), damp, day and sik, less than aand		Í	8/11/14	0		

	Advanced GeoEnvironmental, In 3315 East Miraloma Avenue, Suite Anaheim, California 92806		Bo	ELD BOI ring No.: cal Depth:	RING LOG MW1/VW1 70 feet		
PROJEC	T INFORMATION		DRILL	ING INFOR	MATION		
Project:	Eric Realty	Drilling	Co.:	Cascad	le Drilling, Inc.		
Site Location:	495 East Commercial Rd.	Driller:		Steve	Steve		
Job No.:	SB 607A7.930	Rig Typ	e:	CME 7	75 LAR		
Logged By:	Diane Becker	Method	of Drilling:	10 in. l	Hollow Stem Auger		
Project Manager:	Diane Becker	Samplin	g Methods:	Califor	California Split Spoon		
Dates Drilled:	07-10-02	Hamme	r Wt./Drop	140 LE	3., 30 IN.		
NOTES:		<ul> <li>✓ Water level during drilling</li> <li>✓ Water level in completed well</li> </ul>					
DEPTH SOIL SYMBOLS	DEPTH SOIL USCS SOIL DESCRIPTION				DRING WELL PLETION DESC.		
•	11		· • •	<u> </u>			

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-50 - CL SANDY CLAY: Dark yellowish damp, sandy clay	MW1-D50	9/16/17	0		
-55 - SP	oderate yellowish medium sand MW1-D55	15 <i>/27/2</i> 8	0		
- 60 - SC	wish brown (10YR n sand MWL-D60	8/17/20	0		
- 65 - SAND (POORLY GRADED); M brown (10YR 5/4), wet, fine sand	oderate yelkowish Jess than 10% elit				
-70 - SAND (POORLY GRADED): M brown (10YR 5/4), wet, fine sand	oderate yellowish less than 10% silt			·	Termination

	Advanced GeoEnvironmental, In 3315 East Miraloma Avenue, Suite Anaheim, California 92806		FIELD BORING LOBoring No.:MW2/Total Depth:70 feet					
PROJEC	T INFORMATION		D	RILLI	NG II	NFORMATIC	)N	
Project:	Eric Realty	Drilling	Co.:		(	Cascade Drilli	ng, Inc.	
Site Location:	Driller:			5	Steve			
Job No.:	SB 607A7.930	Rig Type:				CME 75 LAR		
Logged By:	Diane Becker	Method of Drilling:			-	10 in. Hollow Stem Auger		
Project Manager:	Diane Becker	Sampling Methods:			(	California Split Spoon		
Dates Drilled:	07-10-02	Hammer Wt./Drop 140 LB., 30 IN.						
NOTES:	<ul> <li>Water level during drilling</li> <li>Water level in completed well</li> </ul>					-		
DEPTH SOIL SYMBOLS	SAM NUM		Blows /6 in.	PID ppm	BORING COMPLETION	WELL DESC.		

-20-	-							
-20-	-	SM		MW2-D20	8/10/12	0		
- 25 -		сг	CLAYEY SILT (LOW PLASTICITY): Moderate yellowish brown (10YR 5/4), damp, clayey silt, less than 10% fine sand	MW2-D25	7/12/15	0	- -	
- 30 -		sc	SANDY CLAY: Dark yellowish brown (10YR 4/2), damp, slity, sandy clay	MW2-D30	8/10/12	0		
<del>-</del> 35 -		SM	SILTY SAND: Moderate yellowish brown (10YR 5/4), damp, silty line sand	MW2-D35	22/50	0		
-40 -		SM	SILTY SAND: Moderate yellowish brown (10YR 5/4), damp, slify fine aand	MW2-D40	12/15/50	0		
- - 45 -		CL	CLAYEY SILT (LOW PLASTICITY): Dark yeliowish brown (10YR 4/2), damp, day		17/21/30	0		
-	<b>x</b>	ĺ						

	Ge 3315	enced OEnvironmental, II SEast Mirałoma Avenue, Suite heim, California 92806		FIELD BORING LOBoring No.:MW2/VTotal Depth:70 feet				W2/VW2	
PRO,	DRILLING INFORMATION								
Project: Eric Realty				Drilling Co.:			Cascade Drilling, Inc.		
Site Location:	4	95 East Commercial Rd.	Driller:			5	Steve		
Job No.:		SB 607A7.930	Rig Type:			(	CME 75 LAR		
Logged By: Diane Becker		Diane Becker	Method of Drilling:			1	10 in. Hollow Stem Auger		
Project Manager: Diane Becker			Sampling Methods:			(	California Split Spoon		
Dates Drilled: 07-10-02			Hammer Wt./Drop			1	140 LB., 30 IN.		
NOTES:	<ul> <li>Water level during drilling</li> <li>Water level in completed well</li> </ul>								
DEPTH SOI SYMBO		SOIL DESCRIPTION	SAM NUN	PLE 1BER	Blows /6 in.	PID ppm	BORING COMPLETION	WELL DESC.	
• • • • • • • • • • • • •	••••••	· · ·	······						

-50 -		CL	CLAY (LOW PLASTICITY): Dark yellowish brown (10YR 4/4), moist clay, lless than 10% sand	MW2-D50	6/11/12	0		
- 55 -	SZ	SP	SAND (FOORLY GRADED): Moderate yellowish brown (10YR 5/4), moist, fine to medium sand	MW2-D55	7/15/19	0		
-60 -		SM	SILTY SAND: Dark yelkowish brown (10YR 5/4), damp, sity fine sand	- MW2-D60	7/18/23	0		
-65 -		SM	SILTY SAND; Dark yel/owish brown (10YR 5/4), damp, silly fine sand	MW2-D65	12/14/18			
- <b>7</b> 0 -			SAND (POORLY GRADED): Moderate yellowish brown (10YR 5/4), moist, fine sand					Termination
1	l				· · · · · · · · · · · · · · · · · · ·		, , , , , , , , , , , , , , , , , , ,	

	<b>oEnvironmental, I</b> <b>oEnvironmental, I</b> East Miraloma Avenue, Suite heim, California 92806			Bor	ELD ing N al Dej		LOG W3/VW3 feet		
PROJE	DRILLING INFORMATION								
Project: Eric Realty				ng Co.:	Cascade Drillin	rilling, Inc.			
Site Location:	4	95 East Commercial Rd.	Driller:			:	Steve		
Job No.:	S	B 607A7.930	Rig Type:			(	CME 75 LAR		
Logged By:	ſ	Diane Becker	Method of Drilling:			1	10 in. Hollow Stem Auger		
Project Manager: Diane Becker			Sampling Methods: California Split Spoon					Spoon	
Dates Drilled:	. 0	7-11-02	Hammer Wt./Drop 140 LB., 30 IN.						
NOTES:	<ul> <li>Water level during drilling</li> <li>Water level in completed well</li> </ul>								
DEPTH SOIL SYMBOLS	USCS	SOIL DESCRIPTION		MPLE IMBER	Blows /6 in.	PID ppm	BORING COMPLETION	WELL DESC.	
		SILTY CLAY (LOW PLASTICITY): Moderate yellowish brown (10YR 5/4), slightly damp, slity	clay						

-5 -	CL	SILTY CLAY (LOW PLASTICITY): Moderate yellowish brown (10YR 5/4), slightly damp, silty clay	MW3-D5	7/8/10	0	
-10 -	SP	SAND (POORLY GRADED): Moderate yellowish brown (10YR 5/4), slightly damp, fine sand	MW3-D10	12/14/16	0	
-15 -	CL	CLAY (LOW PLASTICITY): Moderate yellowish brown (10YR 5/4), damp, clay	MW3-D15	6/9/9	0	
- 20 -	CL	SANDY CLAY: Dark yellowish brown (10YR 4/2), damp, sandy clay	MW3 - D20	8/10/12	0	
- 25 -	CL	SILTY CLAY (LOW PLASTICITY): Moderate yellowish brown (10YR 5/4), damp, sitty clay		7/12/15	0	

	Ge 3315	n <i>ced</i> oEnvironmental, II East Miraloma Avenue, Suite heim, California 92806			FIELD BORING LOO Boring No.: MW3/ Total Depth: 70 feet				
PROJE	CT INF	ORMATION		·	RILL	ING IN	FORMATIO	N	
Project:	Eric Realty	Drill	ling Co.:		(	Cascade Drillin	g, Inc.		
Site Location:	4	95 East Commercial Rd.	Driller:			5	Steve		
Job No.:	S	B 607A7.930	Rig '	Туре:		(	CME 75 LAR		
Logged By:	Ι	Diane Becker	Met	hod of D	rilling:	1	0 in. Hollow S	tem Auger	
Project Manager:	Ι	Diane Becker	Sam	pling Me	ethods:	(	California Split	Spoon	
Dates Drilled:	0	7-11-02	Ham	nmer Wt.	/Drop	1	40 LB., 30 IN.		
NOTES:		<ul> <li>Water level during drilling</li> <li>Water level in completed well</li> </ul>							
DEPTH SOIL SYMBOL	s USCS	SOIL DESCRIPTION	SAMPLE NUMBERBlows /6 in.PID ppmBORING COMPLETION				WELL DESC.		

-			No recovery					
-30 -				MW3-D30		0		
-								
-			SILTY SAND: Moderate yellowish brown (10YR 5/4), damp, sitly fine sand				-	
-35 -		SM		MW3-D35	22/50	0		
-								
			CLAYEY SAND: Moderate yellowish brown (10YR 5/4), damp, silty fine sand					
-40 -	<u> </u>	SP		MW3-D40	12/15/50	0		
-								
45			SAND (POORLY GRADED): Moderate yellowish brown (10YR 5/4), damp, sity fine sand					
-45 -		SM		MW3-D45	17/21/30	0		
-								j
- 50 -			SILTY SAND: Moderate yellowish brown (10YR 5/4), wet, sitty fine sand					
- 00 -		SM		MW3-D50	6/11/12	0		
-				æ11.				
_ 5 5			SANDY SILT: Moderate yellowish brown (10YR 5/4), wet, sandy silt, less than 10% clay				ļ	
- 55 -		SM		MW3-D55	7/15/19	0		
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		3315	onced oEnvironmental, In East Miraloma Avenue, Suite neim, California 92806			Bor	ELD ing N al Dej		LOG w3/vw3 feet
	PROJEC	T INF	ORMATION		]	DRILLI	NG II	FORMATIO	N
Project:		F	Eric Realty	Dr	illing Co.	:	(	Cascade Drillin	g, Inc.
Site Loc	ation:	4	95 East Commercial Rd.	. Driller:				Steve	
Job No.:		S	B 607A7.930	Rig Type:				CME 75 LAR	
Logged I	Logged By: Diane Becker			Method of Drilling:			1	10 in. Hollow St	tem Auger
Project Manager: Diane Becker			Sampling Methods:				California Split Spoon		
Dates Dr	rilled:	0	7-11-02	Ha	mmer Wi	t./Drop	1	140 LB., 30 IN.	
NOTES:				<ul> <li>Water level during drilling</li> <li>Water level in completed well</li> </ul>					
DEPTH	SOIL SYMBOLS	USCS	SOIL DESCRIPTION		SAMPLE NUMBER	Blows /6 in.	PID ppm	BORING COMPLETION	WELL DESC.
- 60 -		SM	SILTY SAND: Moderate yellowish brown (10YF wet, ailty fine sand	15/4),		15/18/25			
-65 -			SILTY SAND: Moderate yellowish brown (10YR wet, slity fine sand	1 5/4),		14.45.01			

SAND (POORLY GRADED): Moderate yellowish brown (10YR 5/4), wet, fine sand, less than 10% silt :•:• -70-SP

.

SM

-65-

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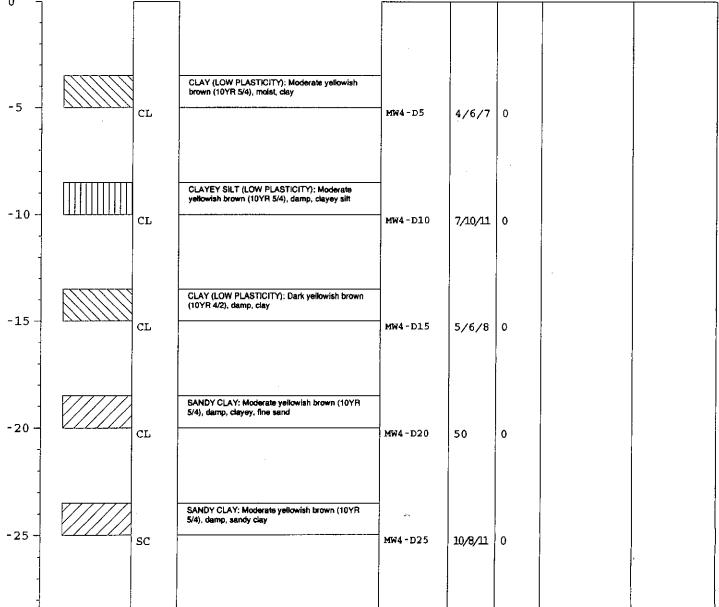
17/19/23

14/15/21

Termination

44

	Advanced GeoEnvironmental, II 3315 East Miraloma Avenue, Suite Anaheim, California 92806		i -	Bor	ELD ing N al Dej		LOG W4 feet		
PROJECT	<b>FINFORMATION</b>	DRILLING INFORMATION							
Project:	Eric Realty	Drilling	; Co.:		(	Cascade Drillir	ıg, Inc.		
Site Location:	495 East Commercial Rd.	Driller:			Ś	Steve			
Job No.:	SB 607A7.930	Rig Type:				CME 75 LAR			
Logged By:	Diane Becker	Method	of D	rilling:	1	l0 in. Hollow S	tem Auger		
Project Manager:	Diane Becker	Sampling Methods:				California Split Spoon			
Dates Drilled:	07-12-02	Hamme	r Wt.	/Drop	1	140 LB., 30 IN.			
NOTES:		\ ▼		r level du r level in a	-	-	-		
DEPTH SOIL SYMBOLS	USCS SOIL DESCRIPTION		SAMPLEBlowsPIDBORINGNUMBER/6 in.ppmCOMPLETION						
0		••••••				<b>.</b>	· · · · ·		



		Ge 3315	enced oEnvironmental, II East Miraloma Avenue, Suite heim, California 92806			Boi	ELD ring N al Dej		LOG W4 feet
	PROJEC	CT INF	ORMATION	DRILLING INFORMAT					N
Project:		F	Eric Realty	Drilling Co.:			(	Cascade Drillin	g, Inc.
Site Loc	ation:	4	95 East Commercial Rd.	Dri	iller:		5	Steve	
Job No.:	.ogged By:		SB 607A7.930		g Type:			CME 75 LAR	
Logged	Logged By: Project Manager:		Diane Becker		thod of I	Drilling:		10 in. Hollow S	-
-	Project Manager: Dates Drilled:		Diane Becker		npling M			California Split	Spoon
		0	7-12-02	Hai	mmer W	-		140 LB., 30 IN.	
NOTES:	:					ter level du ter level in	•	-	
DEPTH	DEPTH SOIL SYMBOLS		SOIL DESCRIPTION			Blows	T	BORING COMPLETION	WELL DESC.
- 30 -		SP	SAND (POORLY GRADED): Moderate yellow brown (10YR 5/4), damp, fine sand		<b>MW4</b> - D30	11/13/15	0		
-35 -		SP	SAND (POORLY GRADED): Moderate yellow brown (10YR 5/4), damp, fine to medium san	đ	MW4-D35	50	0		
-40 -		SP	SAND (POORLY GRADED): Moderate yellow brown (10YR 5/4), damp, fine to medium san than 10% clay	d, less	MW4-D40	36/36	0		
-45 -		SP	SAND (POORLY GRADED): Moderate yellow brown (10YR 5/4), damp, fine to medium sam	d	MW4-D45	15/50	0		
-			SILTY SAND: Moderate yellowish brown (10YF	ł 5/4),					

 
 SM
 MW4 - D50
 21/50
 0

 SILTY SAND: Moderate yellowish brown (10YR 5/4), wet, sity, fine sand
 MW4 - D55
 21/23/29
 0

-50 --

-55-

		Ge 3315	o <b>Environmental, In</b> East Miraloma Avenue, Suite heim, California 92806			Bor	ELD ing N al Dep				
	PROJEC	T INF	ORMATION	DRILLING INFORMATION							
Project:		Cric Realty	Dı	rilling Co.:		(	Cascade Drillin	g, Inc.			
Site Loca	tion:	4	95 East Commercial Rd.	Driller:				Steve			
Job No.:		S	B 607A7.930	Ri	g Type:		(	CME 75 LAR			
Logged B	y:	r	Diane Becker	М	ethod of D	rilling:	1	0 in. Hollow St	tem Auger		
Project M	lanager:	r	)iane Becker	Sa	mpling Me	ethods:	(	California Split	Spoon		
Dates Dri	lled:	0	7-12-02	Ha	ammer Wt.	./Drop	1	40 LB., 30 IN.			
NOTES:				<ul> <li>Water level during drilling</li> <li>Water level in completed well</li> </ul>							
DEPTH	SOIL SYMBOLS	USCS	SOIL DESCRIPTION	N SAMPLE Blows PID BORING NUMBER /6 in. Ppm COMPLETION				WELL DESC.			

-60 -	SM	SiLTY SAND: Moderate yellowish brown (10YR 5/4), wet, slity fine sand	19,	A9/23	
- 65	SP	SAND (POORLY GRADED): Moderate yellowish brown (10YR 5/4), wet, fine sand	17,	124/25	
-70 -	SP	SAND (POORLY GRADED): Moderate yellowish brown (10YR 5/4), wet, fine sand	21,	/21,430	Termination

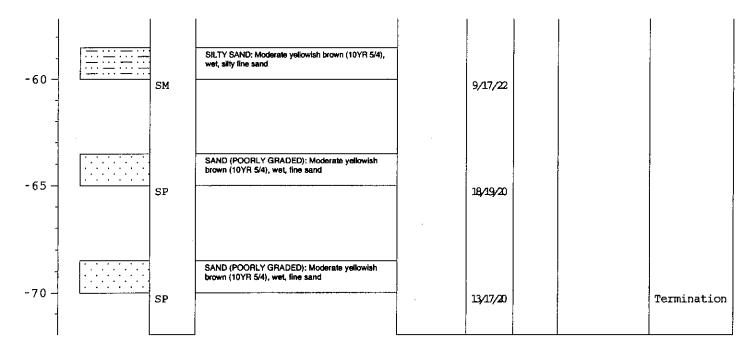
	Advanced GeoEnvironmental, Inc. 3315 East Miraloma Avenue, Suite 117 Anaheim, California 92806									
	PROJEC	T INF	ORMATION		D	RILLI	ING IN	FORMATIO	N	
Project:		F	Fric Realty	Drilling	Co.:			Cascade Drillin	g, Inc.	
Site Loc	ation:	4	95 East Commercial Rd.	Driller:			5	Steve		
Job No.:	:	S	B 607A7.930	Rig Typ	e:		(	CME 75 LAR		
Logged	By:	Γ	)iane Becker	Method	of Di	rilling:	]	l0 in. Hollow St	em Auger	
Project 1				Samplin	ıg Me	thods:	(	California Split	Spoon	
Dates D	rilled:	0	7-11-02	Hamme	r Wt./	/Drop	1	140 LB., 30 IN.		
NOTES	:			☑ ▼		r level du r level in	_			
DEPTH	SOIL SYMBOLS	USCS	SOIL DESCRIPTION	SAM NUM		Blows /6 in.	PID ppm	BORING COMPLETION	WELL DESC.	
-5 -		ML SM	SILT (LOW PLASTICITY): Moderate yellowish I (10YR 5/4), dry, slit SANDY SILT: Moderate yellowish brown (10YR dry, sandy slit CLAY (LOW PLASTICITY): Moderate yellowis brown (10YR 5/4), dry, slity sand	MW5 - D		22/50 17/18/21	0			
-15 -		SM	SAND (POORLY GRADED): Moderate yellow brown (10YR 5/4), slightly damp, fine sand	ish MW5-D		8/9/11	0			
- 25 -		CL	SANDY CLAY: Moderate yellowish brown (10' 5/4), damp, sandy clay	YR Mw5-D	25	10/8/11	0			

(			nced		FIELD BORING						
		3315	OEnvironmental, In East Miraloma Avenue, Suite heim, California 92806		Boring No.:MW5Total Depth:70 feet						
	PROJEC	T INF	ORMATION		DRILLING INFORMATION						
Project:		E	Cric Realty	Drilling	Co.:		(	Cascade Drillin	ıg, Inc.		
Site Loo	cation:	4	95 East Commercial Rd.	Driller:			5	Steve			
Job No.	•	S	B 607A7.930	Rig Typ	e:		(	CME 75 LAR			
Logged	By:	Ľ	)iane Becker	Method	of D	rilling:	1	0 in. Hollow S	tem Auger		
Project	Manager:	Ľ	)iane Becker	Samplir	ng Mo	ethods:	(	California Split	t Spoon		
Dates D	rilled:	0	7-11-02	Hamme	r Wt.	/Drop	1	40 LB., 30 IN.			
NOTES	:		✓ Water level during drilling ✓ Water level in completed well								
DEPTH	SOIL SYMBOLS	USCS	SOIL DESCRIPTION	SAM NUM		Blows /6 in.	PID ppm	BORING COMPLETION	WELL DESC.		
- 30 -		CL	SILTY CLAY (LOW PLASTICITY): Moderate yellowish brown (10YR 5/4), damp, silty clay	MW5-D	930	10/15/16	0				
- 35 -		SM	SILTY SAND: Moderate yellowish brown (10YF damp, silty fine sand	1.54), MW5 - D	35	9/16/17	0				
- 40 -		CL	SANDY CLAY: Moderats yellowish brown (10 5/4), damp, sandy clay	YR MW5-D	40	19/50	0				
-45		sc	CLAYEY SAND: Moderate yellowish brown (1 5/4), damp, clayey fine sand	0YR MW5-D	45	7/11/18	0				
- 50 -		SP	SAND (POORLY GRADED): Moderate yellow brown (10YR 5/4), wet, fine sand	1sh 	50	16/18/25	0				
- 55 -		SM	SILTY SAND: Moderate yellowish brown (10YR wet, sitty, fine sand	5/4}, MW5-D	55	17/18/23	0				

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	3315	nced oEnvironmental, II East Miraloma Avenue, Suite neim, California 92806			Вог	ELD ring N al Dep		LOG W5 feet	
PROJE	CT INF	ORMATION		)N					
Project:	I	Cric Realty	Drilli	ng Co.:			Cascade Drillir	ıg, Inc.	
Site Location:	ocation: 495 East Commercial Rd				Driller: Steve				
Job No.:	S	B 607A7.930	Rig T	ype:		(	CME 75 LAR		
Logged By:	ľ	Diane Becker	Meth	od of D	rilling:	]	l0 in. Hollow S	tem Auger	
Project Manager:	Ι	Diane Becker	Samp	ling M	ethods:	(	California Spli	t Spoon	
Dates Drilled:	0	7-11-02	Hamr	ner Wt	./Drop	]	140 LB., 30 IN.		
NOTES:			<ul> <li>Water level during drilling</li> <li>Water level in completed well</li> </ul>						
DEPTH SOIL SYMBOL	S USCS	SOIL DESCRIPTION		MPLE JMBER	Blows /6 in.	PID ppm	BORING COMPLETION	WELL DESC.	



2.5

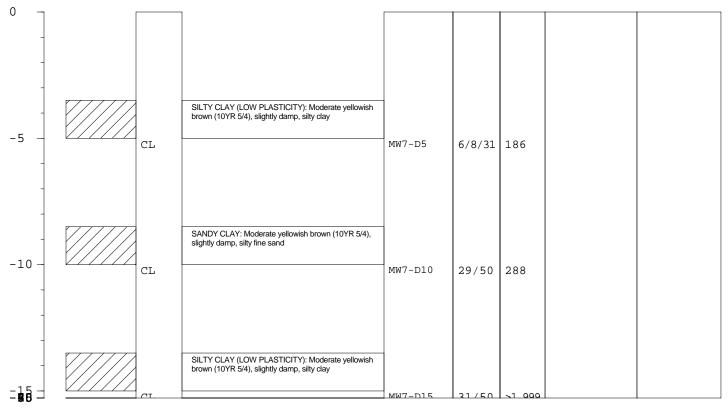
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		3315	<i>nced</i> oEnvironmental, II East Miraloma Avenue, Suite beim, California 92806				Bor	E <b>LD</b> ing N al Dep	lo.:		
	PROJEC	TINE	ORMATION	DRILLING INFORMATION							
Project			Cric Realty	Dı	rilling (				Cascade Dr		
Site Lo	ocation:	4	95 East Commercial Rd.	Dı	riller:			S	Shannon	-	
Job No	o.:	S	B 607A7.930	Ri	ig Type	e:		(	CME 95		
Logge	d By:	Diane Becker				of Dr	illing:	1	0 in. Hollo	w St	em Auger
	oject Manager: Diane Becker				umpling	g Mei	thods:		California S	-	Spoon
Dates 1	Dates Drilled: 02-06-03					Wt./	Drop	1	40 LB., 30	IN.	
NOTÉ	S:						level du	=	-	-	
DEPTH	H SOIL	USCS	SOIL DESCRIPTION		<b>≖</b> SAMP		level in Blows	PID	BORIN	G	WELL
	SYMBOLS				NUME		/6 in.	ppm	COMPLET		DESC.
0 ]											
-5 -	<i>\</i>	CL	SILTY CLAY (LOW PLASTICITY): Moderate yellowish brown (10YR 5/4), slightly damp, si sandy clay		MW6-D5	5	16/26/35	27.3			
-10 -		SM	SILTY SAND: Moderate yellowish brown (10Yf slightly damp, silty fine sand	R 5/4),	MW6-D1	.0	26/50	0			
- 15 - 15 -		CL	CLAY (LOW PLASTICITY): Moderate yellow brown (10YR 5/4), slightly damp, sandy clay		MW6-D1	.5	12/20/44	26.5			
-20 -		SM	SILTY SAND: Moderate yellowish brown (10Yf slightly damp, silty fine sand	R 5/4),	<b>MW6</b> −D2	20	39/50	25.8			
- - 25 - - -	·····	SP	SAND (POORLY GRADED): Moderate yellov brown (10YR 5/4), slightly damp, fine to mediu		MW6-D2		60 for 6"	0			
-30 -		CL	SILTY CLAY (LOW PLASTICITY): Moderate yellowish brown (10YR 5/4), slightly damp, si silty clay		MW6-D3	0	40/50	23.7			
-35 -		SM	SANDY SILT (LOW PLASTICITY): Moderate yellowish brown (10YR 5/4), slightly damp, san		MW6-D3	15	32/50	36.2			
		:								ļ	

		Ge 3315	<i>nced</i> 0 <b>Environmental, Ir</b> East Miraloma Avenue, Suite leim, California 92806			Bor	ELD ing N al Dej		-
	PROJEC	T INF	ORMATION		D	RILLI	NG IN	NFORMATIO	N
Project: Eric Realty Drilling Co.: Cascade Drilling, I							g, Inc.		
Site Loc	ation:	4	95 East Commercial Rd.	Drill	er:		Ş	Shannon	
Job No.:	:	S	B 607A7.930	Rig 🛛	Гуре:		(	CME 95	
Logged	By:	Γ	)iane Becker	Meth	od of D	rilling:		10 in. Hollow St	tem Auger
Project l	Manager:	L	)iane Becker	Samj	oling Me	thods:	(	California Split	Spoon
Dates D	rilled:	0	2-06-03	Ham	mer Wt.	/Drop		140 LB., 30 IN.	
NOTES:						r level du	-	-	
DEBTH	8011	LINCO	SOIL DESOBRETION			r level in	complete PID	1	WELL
DEPTH	SOIL SYMBOLS	USCS	SOIL DESCRIPTION		AMPLE UMBER	Blows /6 in.	ppm	BORING COMPLETION	WELL DESC.
-40		SM ML	SILTY SAND: Moderate yellowish brown (10Yf slightly damp, silty, fine sand CLAYEY SILT (LOW PLASTICITY): Moderat yellowish brown (10YR 5/4), moist, clayey, san	e dy silt	16-D40 16-D45	60 for 6" 50	8.9 24.1		
-50 -		SC	CLAYEY SAND: Moderate yellowish brown ( 5/4), moist, cleyey sand		16-D50	for 6" 19/50	4.8		
-55 -		SM	SILTY SAND: Moderate yellowish brown (10YF saturated, silty, fine sand		16-D55	30/50	0		
-60 - -		SM	SILTY SAND; Moderate yellowish brown (10YF saturated, silty fine sand	"	16-D60	30/50	0		
-65 -		SM	SILTY SAND: Moderate yellowish brown (10YF saturated, silty fine sand		r6-D65	60 for 6"	0		
-70 -		SM	SILTY SAND: Moderate yellowish brown (10YF saturated, silty, fine sand		'6-D70	60 for	0		Termination

A	dvanced GeoEnvironmental, In 381 Thor Place Brea, California 92821	nc.	FIELD BORING LOGBoring No.:MW7Total Depth:70 feet						
PROJECT	INFORMATION		DRIL	LING IN	FORMATIO	N			
Project:	Eric Realty	Drilling	; Co.:		Cascade Drillin	g, Inc.			
Site Location:	495 East Commercial Rd.	Driller:		S	Shannon				
Job No.:	SB 607A7.930	Rig Typ	be:		CME 95				
Logged By:	Diane Becker	Method	of Drillin	g: 1	l0 in. Hollow St	tem Auger			
Project Manager:	Diane Becker	Samplii	ng Method	s: (	California Split	Spoon			
Dates Drilled:	02-05-03	Hamme	er Wt./Droj	) 1	140 LB., 30 IN.				
NOTES:		$\checkmark$							
DEPTH SOIL SYMBOLS	JSCS SOIL DESCRIPTION	SAM NUM	PLE Blov 1BER /6 i		BORING COMPLETION	WELL DESC.			

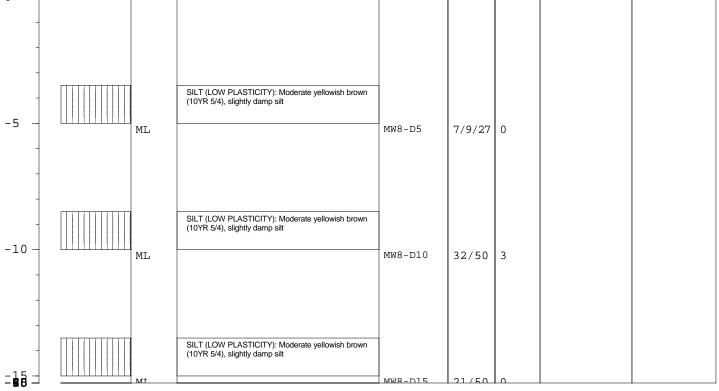


	Advanced GeoEnvironmental, Inc. 381 Thor Place Brea, California 92821					FIELD BORING LOGBoring No.:MW7Total Depth:70 feet					V7
	PROJEC	T INF	ORMATION		DRILLING INFORMATION						N
Project:		E	cric Realty	Dri	lling	Co.:		0	Cascade Dr	rilling	g, Inc.
Site Loca	ation:	4	95 East Commercial Rd.	Dri	ller:			S	hannon		
Job No.:					g Туре	e:		(	CME 95		
Logged	By:	Diane Becker Method				of D	rilling:	1	0 in. Hollo	w St	em Auger
	Manager:	D	Diane Becker	1			ethods:		California S	-	Spoon
Dates Dr	rilled:	0	<b>02-05-03</b> Hamme				/Drop	1	40 LB., 30	IN.	
NOTES:					▽ ▼		er level du er level in	-	-	-	
DEPTH	SOIL SYMBOLS	USCS	SOIL DESCRIPTION		SAMP NUME		Blows /6 in.	PID ppmv	BORIN COMPLET		WELL DESC.
_		CL		P	MW7-D2	25	15/30/50	1,714			
-30 -		CL	SANDY CLAY: Moderate yellowish brown (10YF slightly damp, sandy clay		MW7-D3	0	27/60	98.7			
-35 -		CL	SILTY CLAY (LOW PLASTICITY): Moderate ye brown (10YR 5/4), slightly damp, silty day		MW7-D3	\$5	21/50	30.1			
- 40 - <b>56</b>		CL	SILTY CLAY (LOW PLASTICITY): Moderate ye brown (10YR 5/4), slightly damp, silty clay		MW7-D4	0	27/50	4.0			

	Advanced GeoEnvironmental, Inc. 381 Thor Place Brea, California 92821 PROJECT INFORMATION						FIELD BORING LOGBoring No.:MW7Total Depth:70 feet						
PROJE	CT INFC	ORMATION	DRILLING INFORMATION										
Project:	ric Realty	Drillin	g Co.:		(	Cascade Drillin	g, Inc.						
Site Location:	5 East Commercial Rd.	Driller	:		S	Shannon							
Job No.:	SI	B 607A7.930	Rig Type:				CME 95						
Logged By:	Di	iane Becker	Metho	d of D	rilling:	1	10 in. Hollow Stem Aug						
Project Manager:	Di	iane Becker	Sampli	ing Me	ethods:	(	California Split Spoon						
Dates Drilled:	02	2-05-03	Hamm	er Wt.	/Drop	1	40 LB., 30 IN.						
NOTES:			\ ▼		er level du	U U		-					
DEPTH SOIL SYMBOL	S USCS	SOIL DESCRIPTION		APLE MBER	Blows /6 in.	PID ppmv	BORING COMPLETION	WELL DESC.					

- - -55 -	SP	SAND (POORLY GRADED): Moderate yellowish brown (10YR 5/4), saturated, fine sand	MW7-D55	25/28/40	0		
- - -60 -	SP	SAND (POORLY GRADED): Moderate yellowish brown (10YR 5/4), saturated, fine sand	MW7-D60	21/50	0		
- -65 -70 -=	SM	SILTY SAND: Moderate yellowish brown (10YR 5/4), saturated, silty, clayey, fine sand	MW7-D65	34/50	0		

	Advanced GeoEnvironmental, In 381 Thor Place Brea, California 92821	nc.		Bor	E <b>LD</b> ing N al Dep				
PROJECT	<b>INFORMATION</b>	DRILLING INFORMATION							
Project:	Eric Realty	Drilling	; Co.:		0	Cascade Drilling, Inc.			
Site Location:	Driller:			S	Shannon				
Job No.:	SB 607A7.930	Rig Tyj	pe:			CME 95			
Logged By:	Diane Becker	Method of Drilling:			1	0 in. Hollow St	em Auger		
Project Manager:	Diane Becker	Sampling Methods:			(	California Split Spoon			
Dates Drilled:	02-05-03	Hamme	er Wt.	/Drop	1	40 LB., 30 IN.			
NOTES:		\ ▼		er level dur er level in d	•	-			
DEPTH SOIL SYMBOLS	USCS SOIL DESCRIPTION	SAM NUM	PLE IBER	Blows /6 in.	PID ppmv	BORING COMPLETION	WELL DESC.		

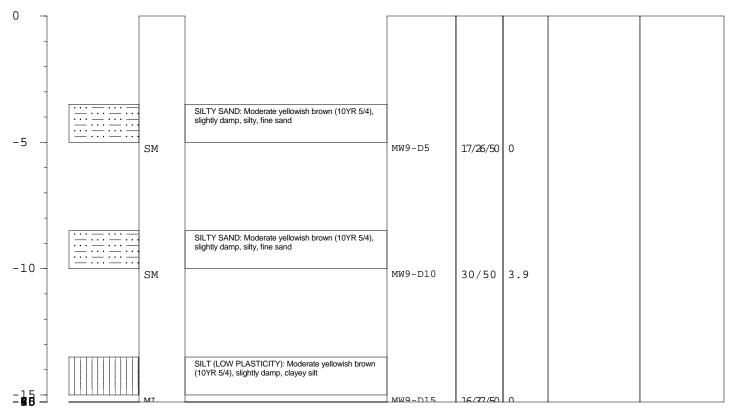


	Advanced GeoEnvironmental, Inc. 381 Thor Place Brea, California 92821						Bor	E <b>LD</b> ing N al Dep		MV	
	PROJEC	T INF	ORMATION	DRILLING INFORMATION						N	
Project:		E	ric Realty	Dri	illing	Co.:		(	Cascade 1	Drillin	g, Inc.
Site Loc	Site Location:495 East Commercial Rd.				iller:			S	hannon		
Job No.:		S	B 607A7.930	Rig Typ				(	CME 95		
Logged	By:	D	oiane Becker	Me	thod	of D	rilling:	1	0 in. Hol	llow St	em Auger
Project N	Manager:	D	Diane Becker	San	nplin	g Me	ethods:	0	Californi	a Split	Spoon
Dates Dr	rilled:	0	2-05-03	Haı	mmer	Wt.	/Drop	1	40 LB., 3	30 IN.	
NOTES:					▽ ▼		er level du er level in	0	0	-	
DEPTH	SOIL SYMBOLS	USCS	SOIL DESCRIPTION		SAMI NUM		Blows /6 in.	PID ppmv	BOR COMPLI		WELL DESC.
-		SC		I	MW8-D2	25	20/50	0			
- 30		CL	SANDY CLAY: Moderate yellowish brown (10YF slightly damp, sandy clay		MW8-D:	30	60 for 6"	0			
-35 -		ML	SILT (LOW PLASTICITY): Moderate yellowish (10YR 5/4), slightly damp silt		MW8-D3	35	35/50	0			
- 40 - <b>50</b>		CL	SILTY CLAY (LOW PLASTICITY): Moderate yel brown (10YR 5/4), slightly damp, silty day		MW8-D4	40	60	0			

		381 7	<i>nced</i> D <b>Environmental, II</b> Chor Place , California 92821	nc.	LOG W8 feet				
	PROJEC	T INF	ORMATION		Ι	ORILLI	NG IN	FORMATIO	N
Project:		Ε	ric Realty	Drillin	ng Co.:		(	Cascade Drillin	g, Inc.
Site Loc	ation:	495 East Commercial		Rd. Driller: Shannon					
Job No.:		S	B 607A7.930	Rig T	ype:		CME 95		
Logged	By:	D	biane Becker	Metho	d of D	rilling:	1	l0 in. Hollow St	tem Auger
Project N	Manager:	D	Diane Becker Sampling Metho		ethods:	(	California Split	Spoon	
Dates D	rilled:	0	2-05-03	Hamn	ner Wt.	./Drop	1	40 LB., 30 IN.	
NOTES:						er level dui er level in d	c		
DEPTH	SOIL SYMBOLS	USCS	SOIL DESCRIPTION		MPLE MBER	Blows /6 in.	PID ppmv	BORING COMPLETION	WELL DESC.
-									

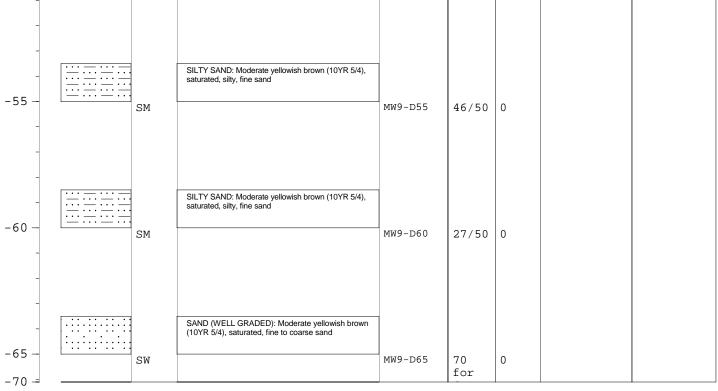
- - -55 -	SP	SAND (POORLY GRADED): Moderate yellowish brown (10YR 5/4), saturated, fine sand	MW8-D55	60 for 6"	0	
- - -60 -	SM	SILTY SAND: Moderate yellowish brown (10YR 5/4), saturated, silty, fine sand	MW8-D60	60 for 6"	0	
- - -65 -70 - <del>-</del>	SC	CLAYEY SAND: Moderate yellowish brown (10YR 5/4), saturated, clayey, fine sand	MW8-D65	60 for	0	

A A	Advanced GeoEnvironmental, In 381 Thor Place Brea, California 92821	FIELD BORING LOGBoring No.:MW9Total Depth:70 feet						
PROJECT	'INFORMATION		DRILL	ING IN	FORMATIO	N		
Project:	Eric Realty	Drilling	Co.:	C	Cascade Drillin	g, Inc.		
Site Location:	495 East Commercial Rd.	Driller:		S	Shannon			
Job No.:	SB 607A7.930	Rig Typ	be:	C	CME 95			
Logged By:	Diane Becker	Method	of Drilling:	1	0 in. Hollow St	em Auger		
Project Manager:	Diane Becker	Samplii	ng Methods:	C	California Split	Spoon		
Dates Drilled:	02-06-03	Hamme	er Wt./Drop	1	40 LB., 30 IN.			
NOTES:		\	Water level d Water level in	-	-			
DEPTH SOIL SYMBOLS	JSCS SOIL DESCRIPTION	SAM NUN	BORING COMPLETION	WELL DESC.				

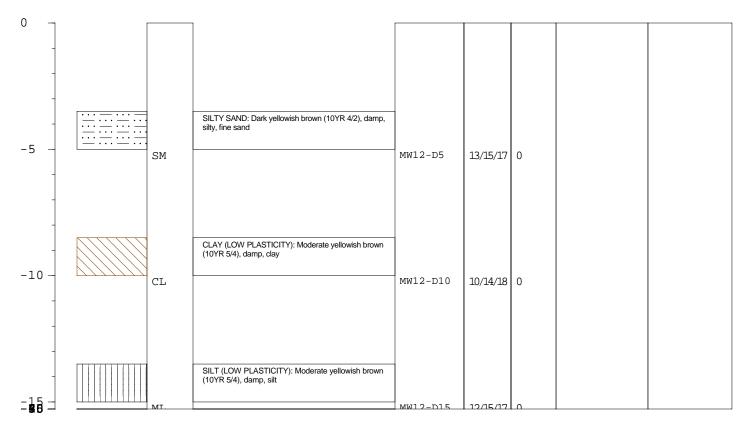


	Advanced GeoEnvironmental, Inc. 381 Thor Place Brea, California 92821						Bor	E <b>LD</b> ing N al Dep		MV	
	PROJEC	T INF	ORMATION	DRILLING INFORMATION						N	
Project:		Ε	ric Realty	Drilling Co.: Cascade Drilling,					g, Inc.		
Site Loc	Site Location:495 East Commercial Rd.Driller							S	Shannon		
Job No.:		S	B 607A7.930	Rig	g Typ	e:		(	CME 95		
Logged	By:	D	iane Becker	Me	thod	of D	rilling:	1	0 in. Ho	llow St	em Auger
Project N	Manager:	D	iane Becker	Sar	nplin	g Me	thods:	(	Californi	a Split	Spoon
Dates Dr	rilled:	0	2-06-03	Ha	mmei	Wt.	/Drop	1	40 LB.,	30 IN.	
NOTES:				<ul><li>Water level during drill</li><li>Water level in complete</li></ul>						-	
DEPTH	SOIL SYMBOLS	USCS	SOIL DESCRIPTION		SAMI NUM	PLE	Blows /6 in.	PID ppmv	BOR COMPL		WELL DESC.
_		SC			MW9-D:	25	20/50	7.6			
- 30		SW	SAND (WELL GRADED): Moderate yellowish (10YR 5/4), slightly damp, fine to coarse sand		MW9-D:	30	70 for 6"	0			
- 35 -		SP	SAND (POORLY GRADED): Moderate yellow brown (10YR 5/4), slightly damp, fine to mediun	n sand	MW9-D:	35	31/50	0			
-40 - - <b>50</b> -		ML	SILT (LOW PLASTICITY): Moderate yellowish (10YR 5/4), slightly damp, clayey, sandy silt		MW9-D4	40	40/50	0			

	381 7	nced DEnvironmental, In Thor Place , California 92821	nc.		Bor	E <b>LD</b> ing N al Dep		LOG W9 feet	
PRO	ECT INF	ORMATION	DRILLING INFORMATION						
Project:	Ε	ric Realty	Drilli	ng Co.:		(	Cascade Drillin	g, Inc.	
Site Location:	4	95 East Commercial Rd.	Drille	r:		S	Shannon		
Job No.:	S	B 607A7.930	Rig Type:			(	CME 95		
Logged By:	D	iane Becker	Method of Drilling:			1	0 in. Hollow S	tem Auger	
Project Manager	r: <b>D</b>	iane Becker	Samp	ling M	ethods:	(	California Split Spoon		
Dates Drilled:	0	2-06-03	Hamr	ner Wt	./Drop	1	40 LB., 30 IN.		
NOTES:			Z		er level du er level in	-	-	-	
DEPTH SOII SYMBO		SOIL DESCRIPTION	~~~~	MPLE MBER	Blows /6 in.	PID ppmv	BORING COMPLETION	WELL DESC.	
-	• 		<b>!</b>						

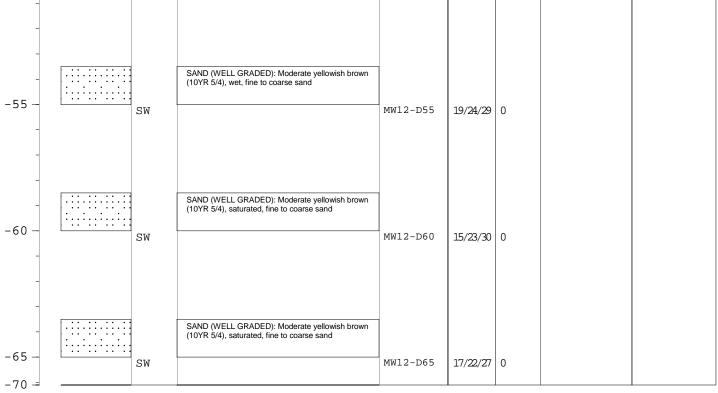


	Advanced GeoEnvironmental, Inc 381 Thor Place Brea, California 92821						BORING I lo.: MV oth: 70 f	V12	
PROJEC	T INF	ORMATION	DRILLING INFORMATION						
Project:	E	Cric Realty	Drilling Co.:				Cascade Drilling	g, Inc.	
Site Location:	4	95 East Commercial Rd.	I. Driller: Izzy						
Job No.:	S	B 607A7.930	Ri	g Type:		(	CME 85		
Logged By:	Ľ	). Becker	М	ethod of D	rilling:	1	0 in. Hollow St	em Auger	
Project Manager:	Ľ	). Becker	Sa	mpling Me	ethods:	(	California Split	Spoon	
Dates Drilled:	0	4-02-04	Ha	ammer Wt.	/Drop	1	40 LB., 30 IN.		
NOTES:				er level du er level in	-	-			
DEPTH SOIL SYMBOLS	USCS	SOIL DESCRIPTION		SAMPLE NUMBER	Blows /6 in.	PID ppmv	BORING COMPLETION	WELL DESC.	

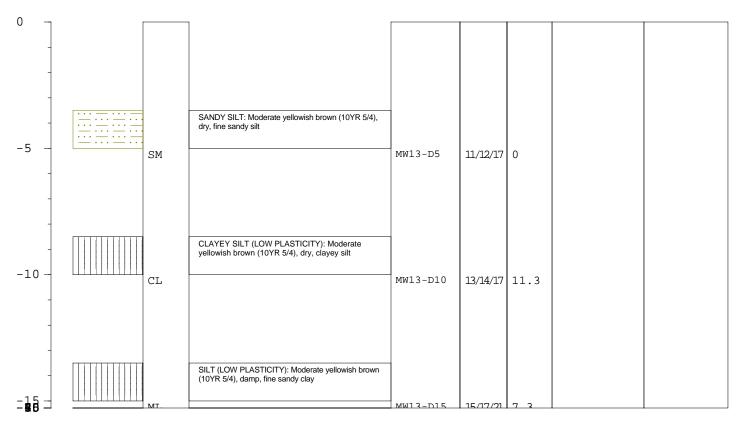


		<i>nced</i> D <b>Environmental, II</b> Thor Place , California 92821	nc.			Bor	E <b>LD</b> ing N al Dep		M	LOG W12 feet	
	PROJEC	T INF	ORMATION	DRILLING INFORMATION							
Project:		E	cric Realty	Drill	ing C	o.:		0	Cascade	Drillin	g, Inc.
Site Loc	ation:	4	95 East Commercial Rd.	Drill	er:			Ι	zzy		
Job No.:		S	B 607A7.930	Rig 7	Type:			0	CME 85		
Logged	By:	D	. Becker	Meth	od of	f Dr	illing:	1	0 in. Ho	llow St	em Auger
	Manager:		. Becker	Samj	oling	Me	thods:		Californi	-	Spoon
Dates Dr	rilled:	0	4-02-04	Ham	mer V	Wt./	/Drop	1	40 LB.,	30 IN.	
NOTES:							r level du			-	
DEPTH	EPTH SOIL USCS SOIL DESCRIPTION		Sz	AMPL UMBE	Æ	r level in Blows /6 in.	PID	d well BOR COMPL		WELL DESC.	
-		SW		MW	12-D2	5	14/16/16	0			
- 30		SC	SANDY CLAY: Moderate yellowish brown (10YF damp, fine sandy clay	,	12-D3	0	22/25/26	0			
-35 -		ML	CLAYEY SILT (LOW PLASTICITY): Moderate yellowish brown (10YR 5/4), damp, clayey silt	t	12-D3	5	20/23/24	0			
-40 - <b>56</b>		CL	SILTY CLAY (LOW PLASTICITY): Moderate yel brown (10YR 5/4), damp, silty clay		12-D4	:0	20/24/27	0			

	381	nced oEnvironmental, I Fhor Place , California 92821	nc.		Bor	E <b>LD</b> ing N al Dep		LOG V12 feet	
PROJ	ECT INF	ORMATION		Γ	ORILLI	NG IN	FORMATIO	N	
Project:	· ·					(	Cascade Drillin	g, Inc.	
Site Location:	Driller:				Izzy				
Job No.:	S	B 607A7.930	Rig Type:				CME 85		
Logged By:	Ι	). Becker	Method of Drilling:				10 in. Hollow Stem Auger		
Project Manager:	Ι	). Becker	Sampling Methods:			0	California Split Spoon		
Dates Drilled:	0	4-02-04	Hamr	ner Wt.	/Drop	1	40 LB., 30 IN.		
NOTES:			\ ▼		er level du er level in	-	-		
DEPTH SOIL SYMBO	LS USCS	SOIL DESCRIPTION		MPLE JMBER	Blows /6 in.	PID ppmv	BORING COMPLETION	WELL DESC.	
- · · ·					·				

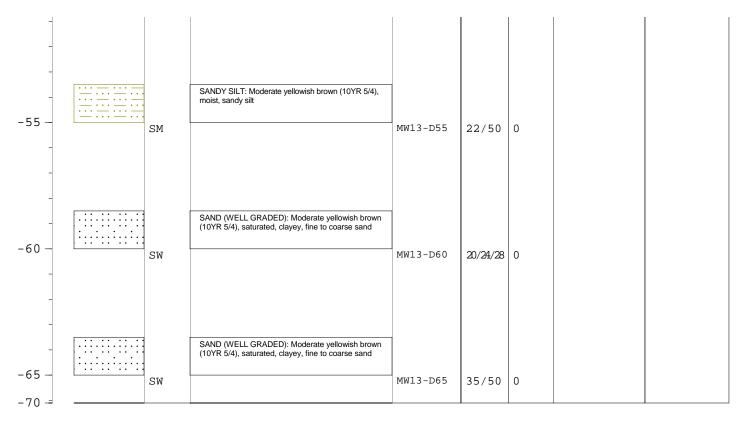


	381	<i>nced</i> 0 <b>Environmental, I</b> I Thor Place , California 92821	nc.	FIELD BORING LOGBoring No.:MW13Total Depth:70 feet					
PROJEC	T INF	ORMATION		Ι	ORILLI	NG IN	FORMATIO	N	
Project:	Project: Eric Realty					(	Cascade Drilling	g, Inc.	
Site Location:	Driller:				Izzy				
Job No.:	S	B 607A7.930	Rig Type:				CME 85		
Logged By:	Ľ	). Becker	Method of Drilling:			1	10 in. Hollow Stem Auger		
Project Manager:	Ľ	). Becker	Samp	Sampling Methods:			California Split Spoon		
Dates Drilled:	0	4-01-04	Hamn	ner Wt.	/Drop	1	40 LB., 30 IN.		
NOTES:	\ ▼		er level du er level in	•	-				
DEPTH SOIL SYMBOLS	USCS	SOIL DESCRIPTION	SAMPLE Blows PID BORING NUMBER /6 in. PID COMPLETION				WELL DESC.		

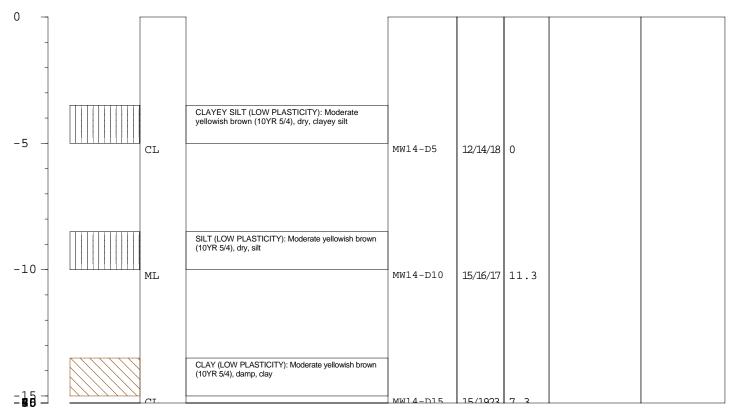


	Advanced GeoEnvironmental, 381 Thor Place Brea, California 92821 PROJECT INFORMATION						Bor	E <b>LD</b> ing N al Dep		LOG W13 feet	
	PROJEC	T INF	ORMATION	DRILLING INFORMATION							
Project:		E	ric Realty	Dril	lling C	Co.:		(	Cascade Drilli	ng, Inc.	
Site Loc	Location:495 East Commercial Rd.Driller			ller:			Ι	ZZY			
Job No.:		S	B 607A7.930	Rig	Type:	•		(	CME 85		
Logged	By:	D	. Becker	Met	thod of	f Dr	illing:	1	0 in. Hollow	Stem Auger	
-	Manager:	D	. Becker				thods:		California Spl	_	
Dates Dr				nmer V	Wt./	Drop	1	40 LB., 30 IN	l <b>.</b>		
NOTES:       Image: Water level during drill         Image: Water level in complete       Water level in complete						-					
DEPTH	DEPTH SOIL USCS SOIL DESCRIPTION SYMBOLS				SAMPL NUMBI	LE	Blows /6 in.	PID	BORING COMPLETIO	WELL N DESC.	
-		SC		M	4W13-D2	25	17/19/25	92.5			
- 30		SC	SANDY CLAY: Moderate yellowish brown (10YF damp, fine sandy clay		4W13-D3	30	18/23/23	237.1			
-35		SP	SANDY CLAY: Moderate yellowish brown (10YF damp, silty, sandy clay	,,	4W13-D3	35	18/20/21	30.6			
-40 - <b>50</b>		SW	SAND (WELL GRADED): Moderate yellowish (10YR 5/4), damp, fine to medium sand		4W13-D4	10	50	0			

	<i>dvanced</i> GeoEnvironmental, In 381 Thor Place Brea, California 92821	nc.	<b>FIEI</b> Borin Total	NG LOG MW13 70 feet			
PROJECT	INFORMATION		DRILLIN	G INFORMA	TION		
Project:	Eric Realty	Drilling	Co.:	Cascade D	Prilling, Inc.		
Site Location:	495 East Commercial Rd.	Driller:		Izzy	Izzy		
Job No.:	SB 607A7.930	Rig Typ	be:	<b>CME 85</b>			
Logged By:	D. Becker	Method	of Drilling:	10 in. Holl	ow Stem Auger		
Project Manager:	D. Becker	Samplii	ng Methods:	California	Split Spoon		
Dates Drilled:	04-01-04	Hamme	r Wt./Drop	140 LB., 3	0 IN.		
NOTES:		⊻	Water level during Water level in con		-		
DEPTH SOIL US	SCS SOIL DESCRIPTION	SAM NUM	NG WELL TION DESC.				

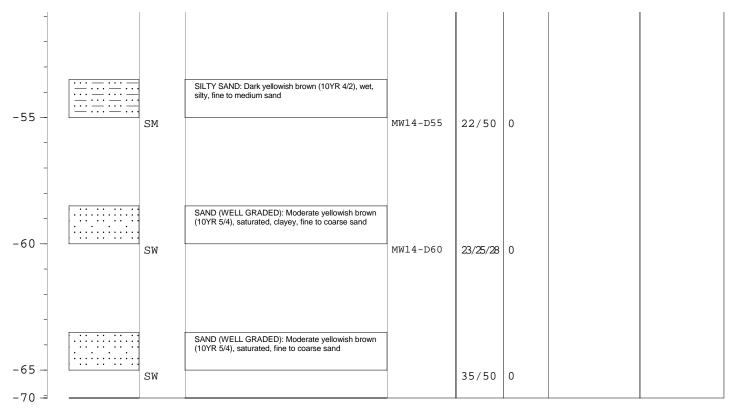


	<i>dvanced</i> GeoEnvironmental, II 381 Thor Place Brea, California 92821	nc.	<b>FIE</b> Borin Total	NG LOG MW14 70 feet			
PROJECT	INFORMATION		DRILLIN	G INFORMA	ATION		
Project:	Eric Realty	Drilling	, Co.:	Cascade I	Drilling, Inc.		
Site Location:	495 East Commercial Rd.	Driller:		Izzy	Izzy		
Job No.:	SB 607A7.930	Rig Typ	be:	CME 85			
Logged By:	D. Becker	Method	of Drilling:	10 in. Hol	low Stem Auger		
Project Manager:	D. Becker	Samplii	ng Methods:	California	a Split Spoon		
Dates Drilled:	04-01-04	Hamme	er Wt./Drop	140 LB., 3	0 IN.		
NOTES:		\	Water level durin Water level in co		-		
DEPTH SOIL US	SCS SOIL DESCRIPTION	SAMPLE Blows PID BORING NUMBER /6 in. PID COMPLETION					

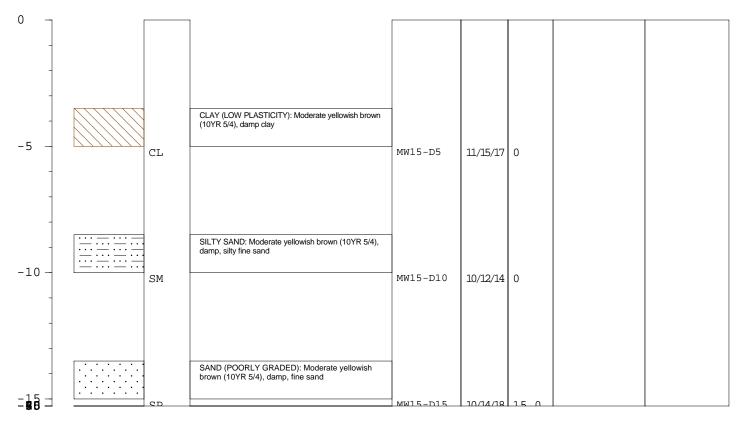


	Advanced GeoEnvironmental, In 381 Thor Place Brea, California 92821 PROJECT INFORMATION						Bor	E <b>LD</b> ing N al Dep		M	LOG V14 feet
	PROJEC	T INF	ORMATION			D	RILLI	NG IN	FORM	IATIO	N
Project:		E	ric Realty	Dril	ling C	Co.:		0	Cascade	Drillin	g, Inc.
Site Loc	ation:	on: <b>495 East Commercial Rd.</b> Driller			ler:			Ι	zzy		
Job No.:		S	B 607A7.930	Rig	Type	:		(	CME 85		
Logged	By:	D	. Becker	Met	hod o	of Di	rilling:	1	0 in. Ho	ollow St	em Auger
Project N	Manager:	D	. Becker	Sam	npling	; Me	thods:	0	Californ	ia Split	Spoon
Dates Dr	rilled:	0	4-01-04	Hammer Wt./Drop				1	40 LB.,	30 IN.	
NOTES:   \sum Water level during drilling     \sum Water level in completed well					_						
DEPTH	SOIL SYMBOLS	USCS	SOIL DESCRIPTION		SAMPI NUMB	LE	Blows /6 in.	PID		RING LETION	WELL DESC.
-		SM		M	IW14-D2	25	19/22/25	92.5			
-30 -		SC	SANDY CLAY: Dark yellowish brown (10YR 4 damp, fine sandy clay		IW14-D3	30	23/27/30	237.1			
-35		SC	SANDY CLAY: Dark yellowish brown (10YR 4 damp, fine sandy clay		IW14-D3	35	19/23/27	30.6			
-40 - <b>50</b>		CL	SILTY CLAY (LOW PLASTICITY): Moderate ye brown (10YR 5/4), damp, silty clay		IW14-D4	40	20/23/24	0			

	<b>Ivanced</b> G <b>eoEnvironmental, I</b> 81 Thor Place Brea, California 92821	nc.	FIELD BORING LOGBoring No.:MW14Total Depth:70 feet						
PROJECT I	NFORMATION		D	RILLI	NG IN	FORMATIO	N		
Project:	Drilling	; Co.:		(	Cascade Drillin	g, Inc.			
Site Location:	Driller:			Ι	Izzy				
Job No.:	SB 607A7.930	Rig Type:				CME 85			
Logged By:	D. Becker	Method	Method of Drilling:			10 in. Hollow Stem Auger			
Project Manager:	D. Becker	Sampling Methods:			(	California Split Spoon			
Dates Drilled:	04-01-04	Hamme	er Wt.	/Drop	1	40 LB., 30 IN.			
NOTES:	<ul> <li>✓ Water level during drilling</li> <li>✓ Water level in completed well</li> </ul>								
DEPTH SOIL US SYMBOLS	CS SOIL DESCRIPTION	SAMPLE Blows NUMBER /6 in.			PID ppmv	BORING COMPLETION	WELL DESC.		

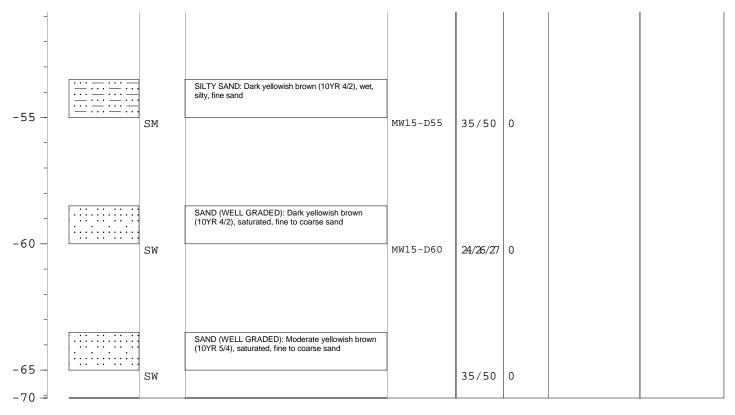


A A A A A A A A A A A A A A A A A A A	<i>dvanced</i> GeoEnvironmental, In 381 Thor Place Brea, California 92821	nc.	<b>FII</b> Bor Tota	ING LOG MW15 70 feet			
PROJECT	<b>INFORMATION</b>		DRILLI	NG INFORM	ATION		
Project:	Eric Realty	Drilling	g Co.:	Cascade	Drilling, Inc.		
Site Location:	495 East Commercial Rd.	Driller:		Izzy	Izzy		
Job No.:	SB 607A7.930	Rig Typ	pe:	<b>CME 85</b>	CME 85		
Logged By:	D. Becker	Method	l of Drilling:	10 in. Ho	llow Stem Auger		
Project Manager:	D. Becker	Samplii	ng Methods:	Californi	a Split Spoon		
Dates Drilled:	04-02-04	Hamme	er Wt./Drop	140 LB.,	30 IN.		
NOTES:		<ul> <li>Water level during drilling</li> <li>Water level in completed well</li> </ul>					
DEPTH SOIL USYMBOLS	JSCS SOIL DESCRIPTION	SAM NUM	ING WELL ETION DESC.				

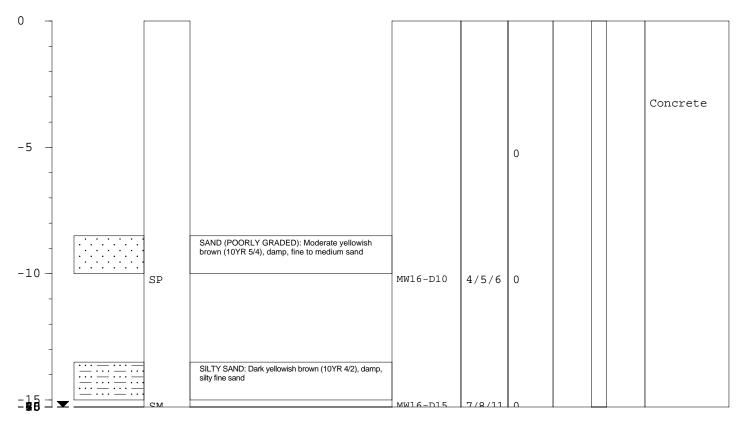


		<i>nced</i> D <b>Environmental, II</b> Thor Place , California 92821	nc.		Bor	E <b>LD</b> ing N al Dep		MV	LOG V15 feet	
	PROJEC	T INF	ORMATION	DRILLING INFORMATION						
Project:		Ε	ric Realty	Drilli	ng Co.	:	(	Cascade D	rilling	g, Inc.
Site Loc	ation:	4	95 East Commercial Rd.	Drille	r:		Ι	zzy		
Job No.:		S	B 607A7.930	Rig T	ype:		(	CME 85		
Logged	By:	D	. Becker	Metho	od of E	Drilling:	1	0 in. Holl	ow St	em Auger
Project N	Manager:	D	. Becker	Samp	ing M	ethods:	(	California	Split	Spoon
Dates Dr	rilled:	0	4-02-04	Hamn	ner Wt	./Drop	1	40 LB., 3	0 IN.	
NOTES:				<ul><li>✓ Water level during drilli</li><li>✓ Water level in completed</li></ul>				0	-	
DEPTH	SOIL     USCS     SOIL DESCRIPTION			MPLE MBER	Blows /6 in.	PID ppmv	BORI COMPLE		WELL DESC.	
-		SC		MW1	5-D25	17/23/25	13.2			
- 30		SC	SANDY CLAY: Dark yellowish brown (10YR 4 damp, fine sandy clay		5-D30	20/24/28	0			
-35 -		SM	SAND (POORLY GRADED): Moderate yellov brown (10YR 5/4), damp, silty, fine sand		5-D35	21/23/30	0			
- 40 - <b>56</b>		SP	SAND (POORLY GRADED): Dark yellowish t (10YR 4/2), damp, fine to medium sand		5-D40	16/22/26	16.2			

381 Th	<b>Deced</b> Denvironmental, In hor Place California 92821	nc.		LOG W15 feet				
PROJECT INFO	ORMATION		D	RILLI	NG IN	FORMATIO	N	
Project: Er	ric Realty	Drilling	; Co.:		(	Cascade Drillin	g, Inc.	
Site Location: 49	Driller:			Ι	Izzy			
Job No.: SE	3 607A7.930	Rig Type:				CME 85		
Logged By: <b>D.</b>	Becker	Method	Method of Drilling:			10 in. Hollow Stem Auger		
Project Manager: D.	Becker	Sampling Methods:			(	California Split Spoon		
Dates Drilled: 04	-02-04	Hamme	er Wt.	/Drop	1	40 LB., 30 IN.		
NOTES:	$\mathbf{\nabla}$		er level du	e	-			
DEPTH SOIL SYMBOLS USCS	SOIL DESCRIPTION				PID ppmv	BORING COMPLETION	WELL DESC.	



	381	<i>nced</i> oEnvironmental, II Thor Place , California 92821	nc.		Bor	E <b>LD</b> ing N al Dep		LOG W16 feet	
PROJE	CT INF	ORMATION		Ι	ORILLI	NG IN	FORMATIC	N	
Project:	Project: Eric Realty					J	DK Drilling		
Site Location:	Driller:				Steve				
Job No.:	S	B 607A7.930	Rig Type:				CME 75		
Logged By:	T	. Smith	Metho	Method of Drilling:			10 in. Hollow Stem Auger		
Project Manager:	Ι	). Becker	Sampli	Sampling Methods:			California Split Spoon		
Dates Drilled:	6	-10-08	Hamm	er Wt.	/Drop	1	40 LB., 30 IN.		
NOTES:	⊻ ¥		er level du		-	-			
DEPTH SOIL SYMBOL	USCS	SOIL DESCRIPTION		APLE MBER	Blows /6 in.		BORING COMPLETION	WELL DESC.	

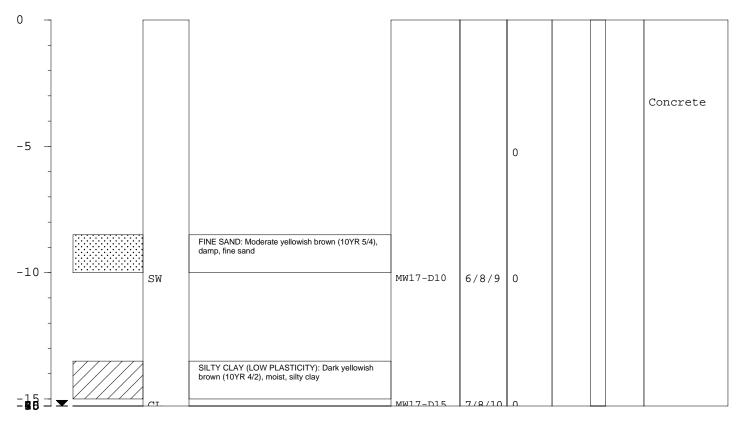


Advanced GeoEnvironmental, Inc. 381 Thor Place Brea, California 92821							Bor	E <b>LD</b> ing N al Dep	0.:	M	LOG W16 feet	
PROJECT INFORMATION					DRILLING INFORMATION							
Project: Eric Realty				Dr	Drilling Co.: JDK Drilling							
Site Loca	ation:	4	95 East Commercial Rd.	Dr	Driller: Steve							
Job No.:		SB 607A7.930		Ri	Rig Type:			<b>CME 75</b>				
Logged 1	By:	Τ	'. Smith	M	ethod o	of D	rilling:	1	10 in. Hollow Stem Auger			
-	Manager:	D	). Becker	Sa	mpling	g Me	ethods:	California Split Spoon				
Dates Dr	rilled:	6	-10-08	Ha	ammer	Wt.	/Drop	140 LB., 30 IN.				
NOTES:	NOTES:				<ul> <li>✓ Water level during drilling</li> <li>✓ Water level in completed well</li> </ul>						-	
DEPTH	SOIL SYMBOLS	USCS	SOIL DESCRIPTION		SAMP NUME		Blows /6 in.	PID ppmv	BC Comp	ORING PLETION	WELL DESC.	
-		SC			MW16-D	025	7/10/12	0			Sch. 40 PVC Blank Casing	
-30 -		SM	SILTY SAND: Moderate yellowish brown (10YF damp, silty fine sand	₹ 5/4),	MW16-D	030	9/12/14	0				
- 35		SP	SAND (POORLY GRADED): Dark yellowish (10YR 6/6), damp, fine to medium sand	orange	MW16-D	035	12/17/23	0				
-40 - - <b>56</b> - ▼		SM	SANDY SILT: Dark yellowish brown (10YR 4/2), fine sandy silt	damp,	MW16-D	040	14/19/24	0			#3 Sand	

Advanced GeoEnvironmental, Inc. 381 Thor Place Brea, California 92821						FIELD BORING LOGBoring No.:MW16Total Depth:75 feet				W16	
PROJECT INFORMATION					DRILLING INFORMATION						
Project:		Ε	ric Realty	Drilling Co.: JDK Drilling							
Site Locatio	on:	4	95 East Commercial Rd.	Dril	Driller: Steve						
Job No.:		S	B 607A7.930	Rig Typ		be:		0	CME 75		
Logged By:		Т	. Smith	Met	thod o	of Di	rilling:	1	10 in. Hollow Stem Auger		
Project Man	nager:	D	. Becker	Sam	npling	g Me	thods:	California Split Spoon			
Dates Drille	ed:	6	-10-08	Han	nmer	Wt./	/Drop	140 LB., 30 IN.			
NOTES:				<ul> <li>Water level during drilling</li> <li>Water level in completed well</li> </ul>							
	SOIL MBOLS	USCS	SOIL DESCRIPTION		SAMP NUME		Blows /6 in.	PID ppmv	BORING COMPLETION	WELL DESC.	
-55 60		SM	SILTY SAND: Dark yellowish brown (10YR 4/2), silty fine sand SANDY SILT: Pale yellowish brown (10YR 6/2), fine to medium sandy silt SANDY SILT: Dark yellowish brown (10YR 4/2),	damp, M	4W16-D 4W16-D		14/17/24			Sch. 40 PVC 0.20- inch Screen	
-65 -		SM	fine sandy silt		4W16-D	065	18/26/35	0			

	Advanced GeoEnvironmental, In 381 Thor Place Brea, California 92821	nc.		Bor			G LOG AW16 75 feet		
PROJ	ECT INFORMATION	DRILLING INFORMATION							
Project:	Drilling Co.:				JDK Drilling				
Site Location:	495 East Commercial Rd.	Driller:			S	Steve			
Job No.:	SB 607A7.930	Rig Type:			(	CME 75			
Logged By:	T. Smith	Method of Drilling:			1	10 in. Hollow Stem Auger			
Project Manager:	D. Becker	Sampling Methods:			(	California Split Spoon			
Dates Drilled:	6-10-08	Hammer Wt./Drop			1	140 LB., 30 IN.			
NOTES:		<ul> <li>✓ Water level during drilling</li> <li>✓ Water level in completed well</li> </ul>							
DEPTH SOIL SYMBO	USCS SOIL DESCRIPTION		IPLE 1BER	Blows /6 in.	PID ppmv	BORING COMPLETIC			

A	nc.		F LOG IW17 5 feet						
PROJECT	DRILLING INFORMATION								
Project: Eric Realty			Drilling Co.:			JDK Drilling			
Site Location:	495 East Commercial Rd.	Driller:		Steve					
Job No.:	SB 607A7.930	Rig Typ	pe:			CME 75			
Logged By:	T. Smith	Method	d of Drilling:			10 in. Hollow Stem Auger			
Project Manager:	Samplii	Sampling Methods:			California Split Spoon				
Dates Drilled:	Hammer Wt./Drop <b>140 LB., 30 IN.</b>								
NOTES:	⊻								
DEPTH SOIL US	SCS SOIL DESCRIPTION	SAM NUM	PLE IBER	Blows /6 in.	PID ppmv	BORING COMPLETIO	N DESC.		



		<b>381</b> T	<i>nced</i> D <b>Environmental, I</b> I Thor Place , California 92821	nc.			Bor	E <b>LD</b> ing N al Dep	0.:	Μ	LOG W17 feet	
	PROJEC	T INF	ORMATION			D	RILLI	NG IN	FOR	MATIO	DN	
Project:		E	ric Realty	Dri	illing (	Co.:	o.: JDK Drilling					
Site Loca	ation:	4	95 East Commercial Rd.	Dri	iller:							
Job No.:		S	B 607A7.930	Rig	g Type	e:		<b>CME 75</b>				
Logged 1	By:	Τ	. Smith	Me	ethod o	of D	rilling:	ng: 10 in. Hollow Stem Aug				
Project N	Manager:	D	. Becker	Sar	npling	g Me	thods:	ods: California Split Spoon				
Dates Dr	rilled:	6	-10-08	Hai	mmer	Wt.	/Drop	1	40 LB	., 30 IN		
NOTES:							er level dui er level in d	0	•		-	
DEPTH	SOIL SYMBOLS	USCS	SOIL DESCRIPTION		SAMP NUME		Blows /6 in.	PID ppmv		ORING PLETION	WELL DESC.	
-		SM		1	MW17-D	25	8/12/14	0				
-30 -		SM	SILTY SAND: Moderate yellowish brown (10YF damp, silty fine sand		MW17-D	30	10/12/18	0			Sch. 40 PVC Blank Casing	
-35		SM	SILTY SAND: Pale yellowish brown (10YR 6/2), silty fine to coarse sand		MW17-D	35	13/17/24	0				
-40 - <b>56 *</b>		SM	SILTY SAND: Moderate yellowish brown (10YF damp, silty fine sand		MW17-D	40	13/19/22	0				

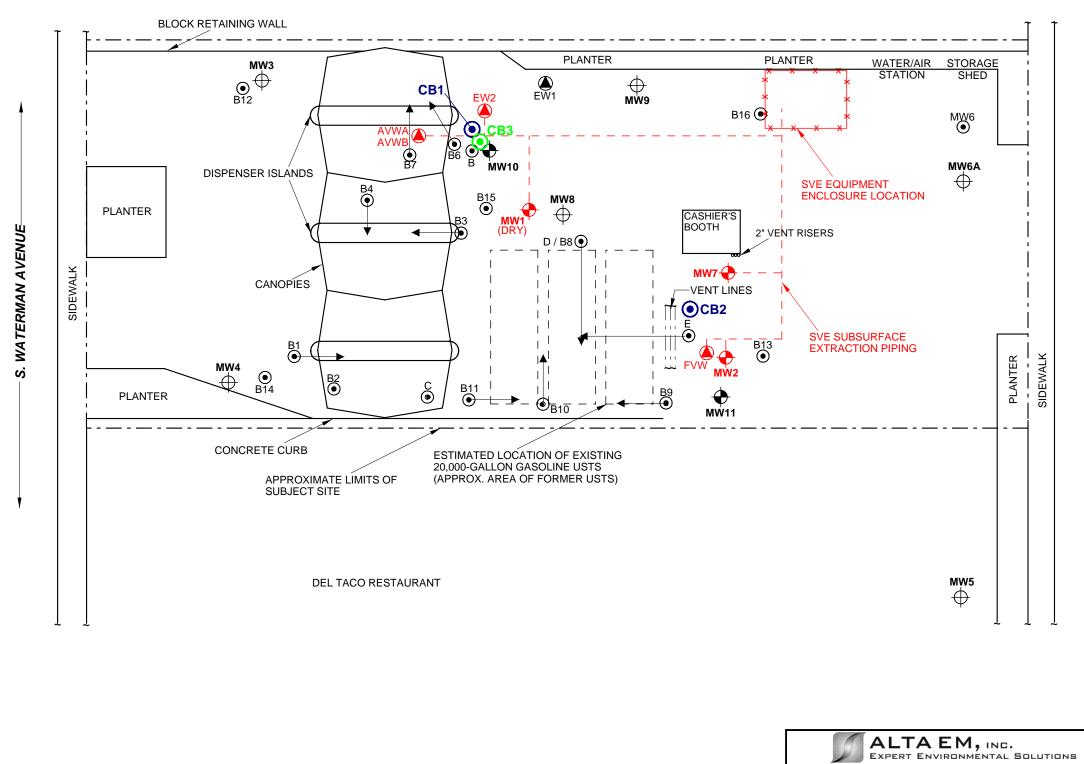
Ge 381	e <b>nced</b> oEnvironmental, II Thor Place , California 92821	nc.		Bor	E <b>LD</b> ing N al Dep		LOG W17 feet
PROJECT INF	ORMATION		D	RILLI	NG IN	FORMATIO	N
Project:	Eric Realty	Drilling	g Co.:		J	DK Drilling	
Site Location: 4	95 East Commercial Rd.	Driller	:		S	Steve	
Job No.:	SB 607A7.930	Rig Ty	pe:		(	CME 75	
Logged By:	<b>Smith</b>	Method	d of D	rilling:	1	0 in. Hollow St	tem Auger
Project Manager:	). Becker	Sampli	ng Me	ethods:	(	California Split	: Spoon
Dates Drilled:	5-10-08	Hamm	er Wt.	/Drop	1	40 LB., 30 IN.	
NOTES:				er level du er level in		-	-
DEPTH SOIL SYMBOLS USCS	SOIL DESCRIPTION		APLE MBER	Blows /6 in.	PID ppmv	BORING COMPLETION	WELL DESC.
-55 - SM	SILTY SAND: Moderate yellowish brown (10Yf damp, silty fine to medium sand SILTY SAND: Dark yellowish brown (10YR 4/2), silty fine to medium sand	MW17		18/25/31			0.20-inch Sch. 40 PVC Screen

	Advanced GeoEnvironmental, In 381 Thor Place Brea, California 92821	nc.		Bor			G LOG MW17 75 feet
PROJI	ECT INFORMATION		D	RILLI	NG IN	FORMAT	ION
Project:	Eric Realty	Drilling	g Co.:		J	DK Drilling	
Site Location:	495 East Commercial Rd.	Driller:			S	teve	
Job No.:	SB 607A7.930	Rig Ty	pe:		(	CME 75	
Logged By:	T. Smith	Method	l of Dr	illing:	1	0 in. Hollow	Stem Auger
Project Manager:	D. Becker	Sampli	ng Me	thods:	(	California Sp	olit Spoon
Dates Drilled:	6-10-08	Hamme	er Wt./	Drop	1	40 LB., 30 I	N.
NOTES:		\ ▼		r level dui r level in d	-	-	-
DEPTH SOIL SYMBO	LS USCS SOIL DESCRIPTION	SAM NUN	IPLE 1BER	Blows /6 in.	PID ppmv	BORING COMPLETIC	



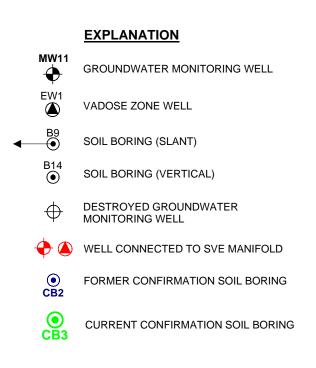
## **APPENDIX C2.11**

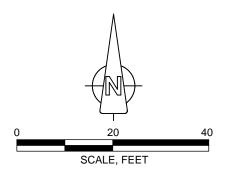
Food N Fuel GeoTracker Case I.D. T0607100528 COMMERCIAL OFFICE BUILDING



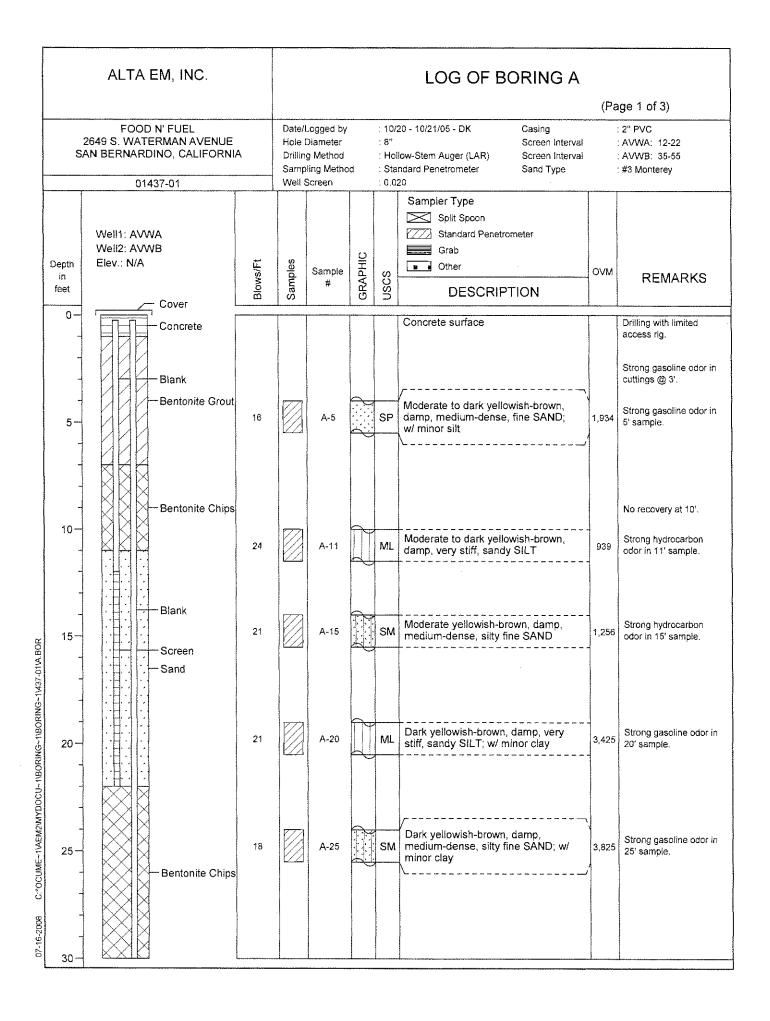
PROJECT:

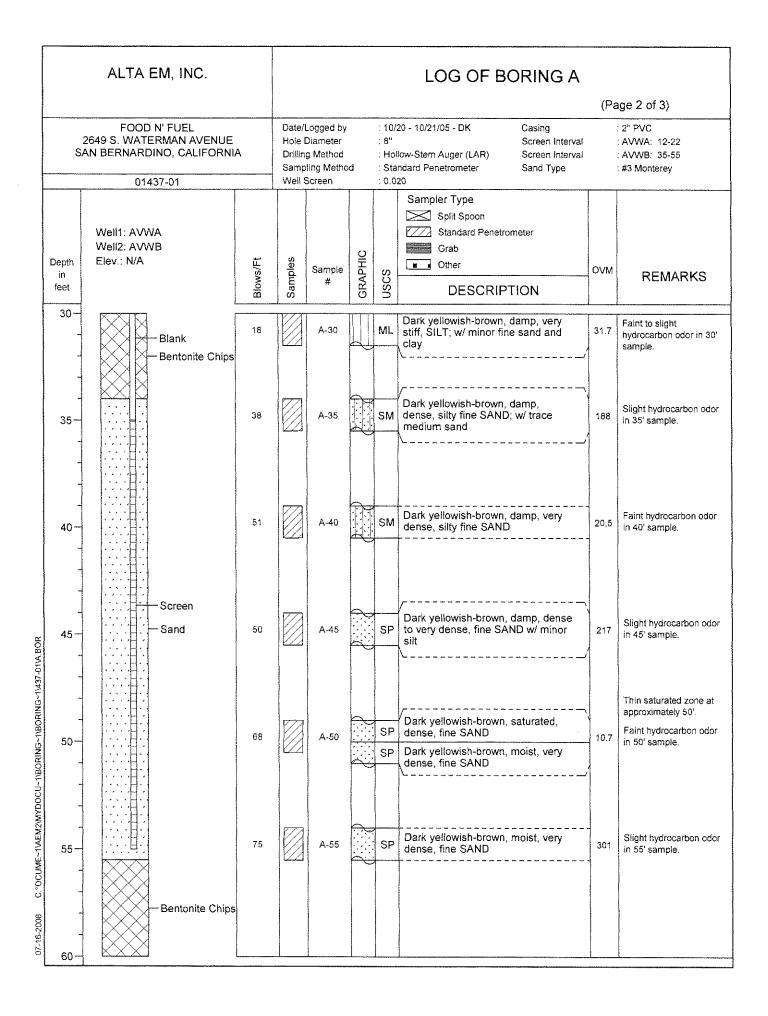
2649 S. WATERMAN AVENUE SAN BERNARDINO, CALIFORNIA

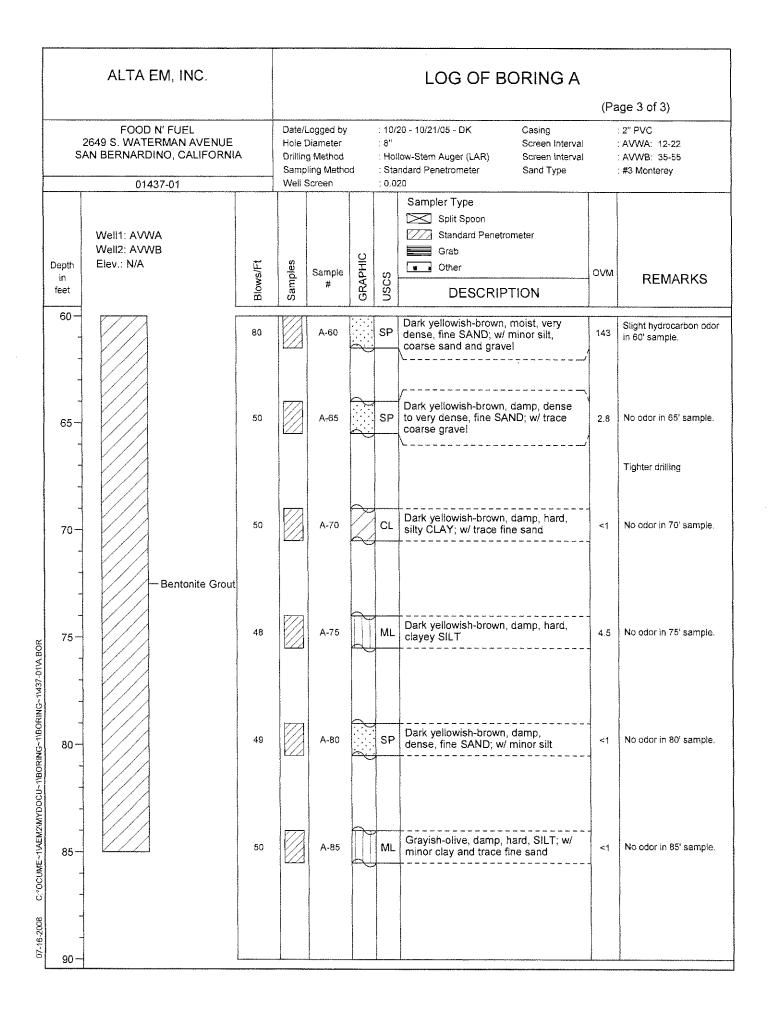




TITLE:	AREA OF INV	/ESTIGATION
DRAWN BY: D. KAWASAKI	CHECKED BY: R. HANSEN	FIGURE NO.:
PROJ. NO.: 01437-01	DATE: FEB 2012	2







		AL <sup>-</sup>	TA EM, INC.				L	OG (	OF BC	ORING B
										(Page 1 of 3)
<u>e</u>		9 S. V	OOD N' FUEL VATERMAN AVENUE JARDINO, CALIFORNIA	Date Drill Boring D Hole Diat Drilling N	epth meter	: 60' : 8''	/02/05 llow-Ster	I Auger		Backfill Material : Bentonite Grout Backfill Interval : 60' - 0' Logged By : R. Stolberg
ļ		T	01437-01	Sampling	Metho	d :Sta	andard Pe	netrome	ter	
Depth in feet	GRAPHIC	uscs	Sampler Type Split Spoon Standard Penetrometer Grab Other DESCRIPTION		Samples	Sample #	Blows/Ft	OVM	Lab Results (TPH)	REMARKS
0-	-		Concrete surface				]			
5-		SM	Dark yellowish-brown, damp, medium-dense, silty fine SAN	ND		В-5	16	66		Slight hydrocarbon odor in 5' sample.
10-		SM	Dark yellowish-brown, damp, medium-dense, silty fine SAN	4D		B-10	22			Slight hydrocarbon odor in 10' sample. Sample is 80% full.
15-		SM	Dark yellowish-brown, damp medium-dense, silty fine SAN	VD		B-15	29	533		Slight hydrocarbon odor in 15' sample.
20-		ML	Dark yellowish-brown, damp, stiff, sandy SILT; w/ minor cla	very ay		B-20	23	64		Slight hydrocarbon odor in 20' sample.
20-		ML	Dark yellowish-brown, damp, stiff, sandy SILT	very		B-25	25	11		Faint to slight hydrocarbon odor in 25' sample.
30-	-			**************************************						

- MARINE WARMAN		AL <sup>-</sup>	TA EM, INC.				L	OG (	OF BC	RING B
,										(Page 2 of 3)
		) S. V	DOD N' FUEL VATERMAN AVENUE ARDINO, CALIFORNIA	Date Drill Boring De Hole Diar Drilling M	epth neter ethod	: 60' : 8" : Hol	02/05 Iow-Stern ndard Pe			Backfill Material     : Bentonite Grout       Backfill Interval     : 60' - 0'       Logged By     : R. Stolberg
<u> </u>			01437-01 Sampler Type	Sampling	Metho	u :sta	noard Pe	netrome	ter	
Depth in feet	GRAPHIC	uscs	Split Spoon Standard Penetrometer Grab Other DESCRIPTION		Samples	Sample #	Błows/Ft	о∨м	Lab Results (TPH)	REMARKS
30-		ML	Dark yellowísh-brown, damp, stiff, sandy SILT	very		B-30	27	6		No odor in 30' sample.
35-		SP	Moderate yellowish-brown, da medium-dense, fine SAND	amp,		B-35	27	15		Faint hydrocarbon odor in 35' sample.
40-		SM	Moderate yellowish-brown, da dense, silty fine SAND	amp,		B-40	31	2		No odor in 40' sample.
437-01\B.BOR		SP	Moderate yellowish-brown, da dense, fine SAND	amp,		B-45	34	5		No odor in 45' sample.
C:"OCUME~1VAEM2MYDDOCU~11BORING~11BORING~11437-011B.BOR		ML	/ Moderate yellowish-brown, da moist, very stiff, SILT; w/ min	amp, or clay		B-50	26	<1		No odor in 50' sample.
		SP	Moderate yellowish-brown, da dense, fine SAND	amp,		B-55	38	<1		No odor in 55' sample.
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		AL <sup>-</sup>	TA EN	I, INC						L	.0G (	OF BC	RING B	
						r veren haarde fan de ser								(Page 3 of 3)
		S. V		IAN AV D, CALI		A	Date Drill Boring De Hole Diar Drilling M Sampling	epth neter lethod	: 60 : 8" : Ho	/02/05 , bliow-Sten andard Pe		tor	Backfill Material Backfill Interval Logged By	: Bentonite Grout : 60' - 0' : R. Stolberg
	1		01437- Samp	ler Typ	e		Sampang	Metro						<u></u>
Depth in feet	GRAPHIC	uscs	$\bowtie$	Split Sp Standar Grab Other	oon d Penet	rometer PTION		Samples	Sample #	Blows/Ft	o∨m	Lab Results (TPH)	R	REMARKS
60-		SP	Light y dense,	ellowist fine SA	ND	, damp,	very	0	B-60	50	<1		No odor in 60' sam	ple.
65-														
70-														
75-	-							Samuel State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State of State						
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	<u> </u>		AL <sup>-</sup>	TA EM, INC.				L	OG (	OF BC	RING C
											(Page 1 of 3)
	S		) S. V	DOD N' FUEL VATERMAN AVENUE ARDINO, CALIFORNIA	Date Drill Boring Di Hole Diar Drilling M	epth meter lethod		low-Sterr	I Auger (I		Backfill Material     : Bentonite Grout       Backfill Interval     : 60' - 0'       Logged By     : R. Stolberg
-		1		01437-01 Sampler Type	Sampling	Metho	id : Sta	ndard Pe	netrome	ter	
	Depth in feet	GRAPHIC	uscs	Sampler Type Split Spoon Standard Penetrometer Grab Other DESCRIPTION		Samples	Sample #	Blows/Ft	ovm	Lab Results (TPH)	REMARKS
ſ	0-		[	Concrete surface							Drilled with limited access rig.
n	- - - 5 -		SM	Olíve green, damp, medium-o silty very fine SAND	iense,		C-5	24	27		Faint to slight hydrocarbon odor in 5' sample.
****	- 10 -		SP	Dark yellowish-brown, damp, medium-dense, fine SAND; w minor silt	/ith		C-10	29	12		Faint hydrocarbon odor in 10' sample.
437-01\C.BOR	- 15 -		SP	Moderate yellowish-brown, da medium dense to dense, fine	amp, SAND		C-15	30	82		Slight hydrocarbon odor in 15' sample.
U-1/BORING-1/BORING-1/	20		ML	Moderate yellowish-brown, da hard, sandy SILT; w/ minor ci	amp, lay		C-20	36	84		Slight hydrocarbon odor in 20' sample.
C:°0CUME~1/AEM2/MYD0CU~1/B0RING~1/B0RING~1/437-01/C.B0	- 25		ML	Moderate yellowish-brown, da hard, sandy SILT; w/ minor c	amp, lay		C-25	36	29		Faint hydrocarbon odor in 25' sample.
07-16-2008	- 30 –										

										(Page 2 of 3)	
S		9 S. W	DOD N' FUEL VATERMAN AVENUE ARDINO, CALIFORNIA	Date Dril Boring D Hole Dia Drilling N	epth meter	: 10/2 : 60' : 8'' : Hol	20/05 low-Sterr	n Auger (	LAR)	Backfill Material : Bentonite Grou Backfill Interval : 60' - 0' Logged By : R. Stolberg	ut
	1		01437-01	Sampling	) Metho	d :Sta	ndard Pe	netrome	ter	T	
epth in eet	GRAPHIC	uscs	Sampler Type Split Spoon Standard Penetrometer Grab Other DESCRIPTION		Samples	Sample #	Blows/Ft	ovm	Lab Results (TPH)	REMARKS	
30		ML	Dark yellowish-brown, damp, stiff to hard, sandy SILT; w/ s clay	very ome		C-30	32	12		Faint hydrocarbon odor in 30' sample.	
35		ML	Dark yellowish-brown, damp, clayey SILT; w/ trace fine san	hard, d		C-35	47	<1		No odor in 35' sample.	
- - 40		. SP	Moderate yellowish-brown, da dense, fine SAND	amp,		C-40	34	3		No odor in 40' sample.	
45-		. SM	Moderate yellowish-brown, da medium dense, silty fine SAN minor clay	amp, ID; w/		C-45	24	<1		No odor in 45' sample.	
50		SP	Moderate yellowish-brown, da very dense, fine SAND; w/ tra	amp, ace silt		C-50	51	<1		No odor in 50' sample.	
55		SP	Moderate yellowish-brown, da very dense, fine SAND; w/ tra	amp, ace silt		C-55	75	<1		No odor in 55' sample.	
55		SP	very dense, fine SAND; w/ tra	ace silt		C-55	75	<1		No odor in 55' sample.	

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		S. W		FUEL MAN AVI			Date Dril Boring D Hole Dia	epth	: 10/ : 60' : 8''	20/05		····	Backfill Material Backfill Interval Logged By	(Page 3 of 3) : Bentonite Grout : 60' - 0' : R. Stolberg
			01437				Drilling N Sampling	lethod	: Ho	llow-Sterr Indard Pe				
Depth in feet	GRAPHIC	uscs		Split Spo Standard Grab Other	noo			Samples	Sample #	Blows/Ft	o∨m	Lab Results (TPH)	R	EMARKS
60-		SP	Moder dense sand	ate yello , fine SA	wish-bro ND; w/ r	own, da ninor c	imp, :oarse		C-60	50	<1		No odor in 60' sam;	ole.
65-														
70 -														
75-														
00 100 100 100 100 100 100 100 100 100														· ·
C. 000ME-1MEMZIMMD0C0-1B0HNNG-1B0HNNG-1M9C-011C B0H														
	+													
90 -	-													s.

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*****			AL <sup>-</sup>	TA EM, INC.			L	OG (	DF B	ORIN	G D (SLANT)
											(Page 1 of 3)
	S		) S. V	OOD N' FUEL VATERMAN AVENUE ARDINO, CALIFORNIA	Date Dril Boring D Hole Dial Drilling M Sampling	epth meter Nethod	: 60* : 8.0" : Holl	02/05 low-Stem			Backfill Material : Bentonite Grout Backfill Interval : 60' - 0' Logged By : D. Kawasaki
				01437-01 Sampler Type	Samburg	j wietno		ngaro Pe	netrome	ter	
Stade, Wildowski w produktion – 1. – Stade State  Depth in	GRAPHIC	uscs	Split Spoon Standard Penetrometer Grab Other		Samples	Sample #	Blows/Ft	ovm	Lab Results	REMARKS	
-	feet	1 5	ŝ	DESCRIPTION		Sa		E		(TPH)	
A DATE OF A DATE OF A DATE OF A DATE OF A DATE OF A DATE OF A DATE OF A DATE OF A DATE OF A DATE OF A DATE OF A	0	and and a second second second second second second second second second second second second second second se		Concrete surface							Boring drilled at 21 degree angle from vertical.
	5			Void from 0 to 16 feet (condu	ctor						Boring drilled through existing 10-inch diameter
	-  10 	ter en ante en ante en ante en ante en ante en ante en ante en ante en ante en ante en ante en ante en ante en		casing)							PVC conductor casing.
-01°.BOR	- 15— -	· · · · · · · · (									
ING~1\437	-		SP	Dark grey, damp, coarse SAN minor gravel and fine sand	1D; w/		D-18		15		Slight hydrocarbon odor in 18' sample.
C:°OCUME~1/AEMZ/MYDOCU~1/BORING~1/BORING~1/437-01°,BOR	- 20— -		SP	Dark yellowish-brown, damp, SAND	fine		D-20		38		Slight hydrocarbon odor in 20' sample.
MYDOCU-1	-		ML	Dark yellowish-brown, damp, SILT	sandy		D-23		5		No odor in 23' sample.
DCUME-1/AEM2/	- 25		SM	Moderate yellowish-brown, m wet, silty fine SAND	oist to		D-25		1		No odor in 25' sample.
	-										
07-16-2008	- 30—	-									

		AL <sup>-</sup>	ΓΑ ΕΜ, INC.			L	OG (	OF B	ORIN	G D (SLANT)
1										(Page 2 of 3)
		9 S. V	DOD N' FUEL VATERMAN AVENUE ARDINO, CALIFORNIA	Date Drill Boring D Hole Dial Drilling M	epth meter lethod	: 60' : 8.0' : Hol	low-Stem	-	·~·	Backfill Material : Bentonite Grout Backfill Interval : 60' - 0' Logged By : D. Kawasaki
			01437-01 Sampler Type	Sampling		a sta	ndard Pe	netrome	ter	
Depth in feet	GRAPHIC	USCS	Split Spoon Standard Penetrometer Grab Other DESCRIPTION		Sampies	Sample #	Biows/Ft	ovm	Lab Results (TPH)	REMARKS
30-		ML	Moderate yellowish-brown, m sandy SILT	oist,		D-30		<1		No odor in 30' sample.
35-		ML	/ Dark yellowish-brown, moist t SILT; w/ minor clay and trace sand	o wet, fine		D-35		<1		No odor in 35' sample.
40-		ML	Dark yellowish-brown, damp moist, SILT; w/ minor clay an fine sand	to d trace		D-40		<1		No odor in 40' sample.
45-		SM	Moderate to dark yellowish-b damp, silty fine SAND	rown,		D-45		<1		No odor in 45' sample.
50-	( <u>,,,,,</u> ,) ( <u>,,,,,,</u> )	SM	Moderate yellowish-brown, di silty fine SAND	amp,		D-50		<1		No odor in 50' sample.
55-		SP	Light yellowish-brown, damp, SAND; w/ minor silt	fine		D-55		<1		No odor in 55' sample.
60-										

07-16-2008 C:\*OCUME~\*1AEM2WYDOCU~\*1BORING~11BORING~1437-01\* BOR

		AL <sup>-</sup>	TA EM, INC.			L	OG (	DF B	ORIN	G D (SLANT)
										(Page 3 of 3)
s		9 S. V	OOD N' FUEL VATERMAN AVENUE IARDINO, CALIFORNIA	Date Drill Boring Do Hole Diar Drilling M	epth meter lethod	: 60' : 8.0' : Hol	low-Stem		,	Backfill Material : Bentonite Grout Backfill Interval : 60' - 0' Logged By : D. Kawasaki
	1		01437-01 Sampler Type	Sampling	Metho	d :Sta	ndard Pei	retromet	ter	
Depth in feet	GRAPHIC	USCS	Sampler Type Split Spoon Standard Penetrometer Grab Other DESCRIPTION		Samples	Sample <sup>.</sup> #	Blows/Ft	OVM	Lab Results (TPH)	REMARKS
60-		SP	Dark yellowish-brown, damp, SAND; w/ minor silt	fine		D-60		<1		No odor in 60' sample.
65-										
70-										
75-	-									
80 -										
85-										
90										

											Page 1 of 3)
5		S. V	DOD N' FUEL VATERMAN AVENUE ARDINO, CALIFORNIA	Date Drill Boring De Hole Diar Drilling M	epth meter	: 11/2 : 60' : 8''	2/05 low-Sterr	Auger		Backfill Material Backfill Interval Logged By	: Bentonite Grout : 60' - 0' : D. Kawasaki
			01437-01	Sampling			ndard Pe		ter		
epth in feet	GRAPHIC	uscs	Sampler Type Split Spoon Standard Penetrometer Grab Other DESCRIPTION		Sampies	Sample #	Blows/Ft	o∨m	Lab Results (TPH)	R	EMARKS
-0 -			Concrete surface							Drilled at 22 degree	angle from vertical.
- - 5 -		SP	Moderate yellowish-brown, di fine SAND; w/ minor silt	amp,						No odor in 5' cutting	S.
- 10- -		SP	Dark yellowish-brown, damp, SAND; w/ minor silt	fine		E-10		12		Faint hydrocarbon c	dor in 10' sample.
15-		SP	Dark yellowish-brown, damp, SAND; w/ minor silt	 fine		E-15		57		Slight hydrocarbon i	odor in 15' sample.
20-		SP	Moderate yellowish-brown, d. fine SAND; w/ some silt	amp,		E-20		10		Faint hydrocarbon c	dor in 20' sample.
- 25		SM	Moderate yellowish-brown, d silty fine SAND	amp,		E-25		22		Faint hydrocarbon o	odor in 25' sample.

		۸L٦	ΓΑ ΕΜ, INC.			L	OG (	OF B	ORIN	G E (SLAN⁻	Г)
										(	Page 2 of 3)
s		) S. W	DOD N' FUEL /ATERMAN AVENUE ARDINO, CALIFORNIA 01437-01	Date Drill Boring D Hole Diar Drilling M Sampling	epth meter lethod		2/05 Iow-Stem ndard Pe		er	Backfill Material Backfill Interval Logged By	: Bentonite Grout : 60' - 0' : D. Kawasaki
Depth in feet	GRAPHIC	USCS	Sampler Type Split Spoon Standard Penetrometer Grab Other DESCRIPTION	,	Samples	Sampie #	Blows/Ft	OVM	Lab Results (TPH)	RI	EMARKS
30-		ML	Dark yellowish-brown, damp, SILT	sandy		E-30		<1		No odor in 30' samp	le.
35-		ML	Dark yellowish-brown, damp, SIL7	sandy		E-35		<1		No odor in 35' samp	le.
40-		SP	Dark yellowish-brown, damp, SAND w/ minor silt and trace sand	fine coarse		E-40		<1		No odor in 40' samp	ie.
437-01/E BOR		ŚP	Moderate yellowish-brown, d fine SAND w/ trace gravel	amp,		E-45		<1		No odor in 45' samp	e.
-11180RING-1180RING-11- 0 -		SP	Moderate yellowish-brown, d fine SAND; w/ trace gravel	amp,		E-50		<1		No odor in 50' samp	sie.
C.ºOCUME-1MEM2MMYDOCU-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-1180RING-118		SP	Moderate yellowish-brown, d fine SAND; w/ minor silt	lamp,		E-55		2		No odor in 55' sam;	ole.
09 07-16-2008	-										

1		AL7	TA EM, INC.			L	OG (	DF B	ORIN	G E (SLANT)
										(Page 3 of 3)
		S. W	OOD N' FUEL VATERMAN AVENUE VARDINO, CALIFORNIA	Date Drill Boring De Hole Diar Drilling M	epth neter lethod		ow-Stem			Backfill Material : Bentonite Grout Backfill Interval : 60' - 0' Logged By : D. Kawasaki
			01437-01 Sampler Type	Sampling	Metho	d :Stai	ndard Pe	netrome	ter	** + *********************************
Depth in feet	GRAPHIC	nscs	Split Spoon Standard Penetrometer Grab Other DESCRIPTION	1	Sampies	Sample #	Blaws/Ft	о∨м	Lab Results (TPH)	REMARKS
60-		SP	Dark yellowish-brown, damp SAND	, fine		E-60		<1		No odor in 60' sample.
65-										
70-										
75-					N FY CATTORN AND A LAND AND A LAND AND A LAND AND A LAND AND A LAND AND A LAND AND A LAND AND A LAND AND A LAND					
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		AL	TA EM, INC.			<u>, , , , , , , , , , , , , , , , , , , </u>	LC	G O	F BOF	RING CB1
										(Page 1 of 3)
		9 S. V	OOD N' FUEL VATERMAN AVENUE IARDINO, CALIFORNIA	Date Drill Boring De Hole Dial Drilling M	epth meter	: 75' : 8"	17/08 low-Sterr	n Auger		Backfill Material : Bentonite Grout Backfill Interval : 75' - 0' Logged By : D. Kawasaki
	,	r	01437-01	Sampling			Split Spor			·
Depth in feet	GRAPHIC	USCS	Sampler Type Split Spoon Standard Penetrometer Grab Other DESCRIPTION		Samples	Sample #	Blows/Ft	ovm	Lab Results (TPH)	REMARKS
0-			Concrete surface							
5		SM	Moderate yellowish-brown, da moist, medium dense, silty fi	amp to te SAND		CB1-5	21	240		Slight to moderate hydrocarbon odor in 5' sample.
10-		SM	Dark yellowish-brown, moist, dense, silty fine SAND	medium		CB1-10	21	25		Faint hydrocarbon odor in 10' sample.
15-		SM	Moderate yellowish-brown, da moist, medium dense, silty fi	amp to he SAND		CB1-15	25	95		Faint to slight hydrocarbon odor in 15' sample.
20-		SP	Moderate yellowish-brown, da dense, fine to medium SAND trace coarse sand	amp, ; with		CB1-20	47	8		No odor in 20' sample.
25 -		ML	Dark yellowish-brown, damp hard, sandy SILT; with minor sand	to moist, medium		CB1-25	56	<b>T</b>		No odor in 25' sample.
30-										

		AL <sup>-</sup>	TA EM, INC.				LC	G O	F BOF	RING CB1
										(Page 2 of 3)
		) S. V	DOD N' FUEL VATERMAN AVENUE ARDINO, CALIFORNIA	Date Drill Boring De Hole Diar Drilling M	epth meter	: 75 <sup>+</sup> : 8"	17/08 low-Sterr	n Auger		Backfill Material: Bentonite GroutBackfill Interval: 75' - 0'Logged By: D. Kawasaki
	<b>T</b>		01437-01	Sampling	Metho	d : 2" 8	Split Spoo	n		
Depth in feet	GRAPHIC	USCS	Sampler Type Split Spoon Standard Penetrometer Grab Other DESCRIPTION		Samples	Sample #	Blows/Ft	o∨m	Lab Results (TPH)	REMARKS
30-		ML	Dark yellowish-brown, damp, stiff, SILT; with minor fine sar	very nd		CB1-30	47	3		No odor in 30' sample.
35-		SP	Moderate yellowish-brown, da very dense, fine SAND	amp,	$\square$	CB1-35	56	<b>Y</b>		No odor in 35' sample.
40-		SM	Moderate yellowish-brown, da very dense, silty fine SAND	amp,		CB1-40	52	<b>1</b>		No odor in 40' sample.
45-		SM	Moderate yellowish-brown, da very dense, silty fine SAND	amp,		CB1-45	63	<b>4</b>		No odor in 45' sample.
10-11BORING-11BORING-10-10-10-10-10-10-10-10-10-10-10-10-10-		SP	Light yellowish-brown, damp, dense, fine SAND	very		CB1-50	60	<1		No odor in 50' sample.
C:*0CUME~114EM2WYDOCU-11BORING~11BORING~11437-011CB1.BO	-	SP	Light yellowish-brown, damp, dense, fine to medium SAND trace coarse sand			CB1-55	82	<1		No odor in 55' sample.
07-16-2008	-									

			ΓΑ ΕΜ, INC.					00		(Page 3 of 3)
		S. V	DOD N' FUEL VATERMAN AVENUE ARDINO, CALIFORNIA	Date Dril Boring D Hole Dia Drilling N	epth meter	: 75' : 8''	17/08 low-Sterr	Auger		Backfill Material : Bentonite Grout Backfill Interval : 75' - 0' Logged By : D. Kawasaki
			01437-01	Sampling			Split Spor	-		
Depth ín feet	GRAPHIC	uscs	Sampler Type Split Spoon Standard Penetrometer Grab Other DESCRIPTION		Samples	Sample #	Biows/Ft	OVM	Lab Results (TPH)	REMARKS
60 -		SP	Light yellowish-brown, damp dense to very dense, fine to r SAND; with trace gravel	to moìst, nedium		CB1-60	50	<1		No odor in 60' sample.
- 65 -		SP	Light yellowish-brown, damp, SAND; with some medium to SAND; and minor gravel	fine coarse		CB1~65	85	<1		No odori n 65' sample.
- - 70 — -		ML	Moderate yellowish-brown, d hard, SILT; with minor clay	amp,	$\square$	CB1-70	87	<1		No odor in 70' sample.
- - 75 -		ML	Moderate olive-brown, damp hard, SILT	to moist,	$\square$	CB1-75	49	<1		No odor in 75' sample.
- - 80 -					er regelen andere en verste forste en verste en ve					
- - 85 -		And the second second second second second second second second second second second second second second second			na – 1994. – Annual d <sup>a k</sup> ara ang kanalang kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na kanala na					
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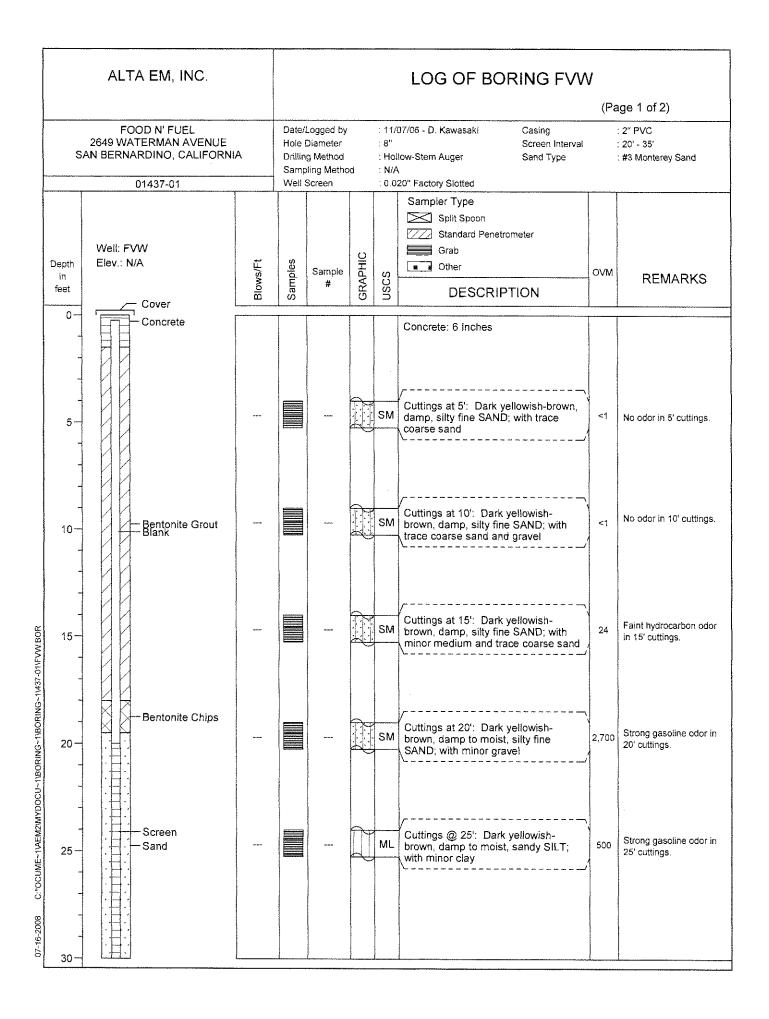
		AL.	TA EM, INC.				LO	GO	F BOF	RING CB2
5		) S. V	OOD N' FUEL VATERMAN AVENUE IARDINO, CALIFORNIA	Date Drille Boring De Hole Dian Drilling Me Sampling	epth neter ethod	: 75' : 8" : Hol	17/08 Iow-Stem	-		(Page 1 of 3) Backfill Material : Bentonite Grout Backfill Interval : 75' - 0' Logged By : D. Kawasaki
Depth in feet	GRAPHIC	uscs	01437-01 Sampler Type Split Spoon Standard Penetrometer Grab Other DESCRIPTION	Sampung	Samples	Sample #	Blows/Ft	o ovm	Lab Results (TPH)	REMARKS
0		SP	Concrete surface Moderate yellowish-brown, da dense, fine SAND; with minor	imp, silt	$\square$	CB2-5	48	<1		No odor in 5' sample.
- - 10		ML	Dark yellowish-brown, damp hard, SILT	to moist,	$\square$	CB2-10	40	3		No odor in 10' sample.
- - 15 -		SP	Moderate yellowish-brown, m dense, fine SAND	oist,	$\square$	CB2-15	39	5		No odor in 15' sample.
- 20- -		SP	Dark yellowish-brown, damp dense, fine SAND	to moist,	$\square$	CB2-20	38	<1		No odor in 20' sample.
- 25-		SM	Dark yellowish-brown, moist, silty fine SAND; with some m sand	dense, edíum	$\square$	CB2-25	38	<1		No odor in 25' sample.
- 30										

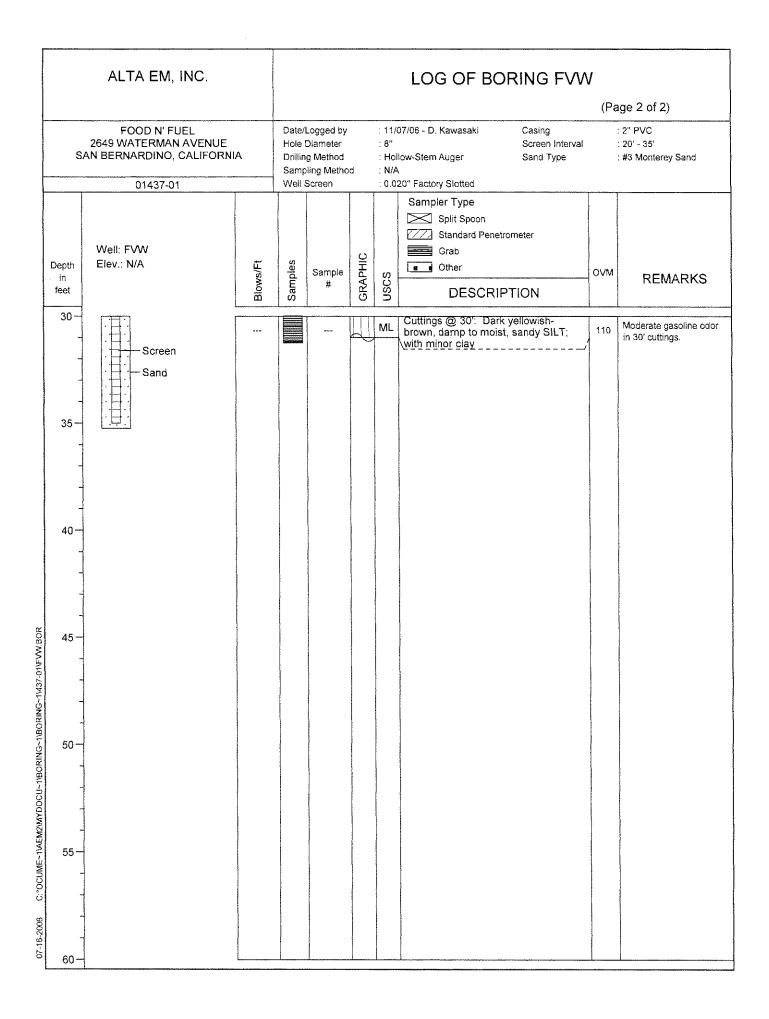
		А	LT/	A EM, INC.				LO	GO	F BOF	RING CB2
											(Page 2 of 3)
	s	2649 S	. WA RNA	OD N' FUEL ATERMAN AVENUE RDINO, CALIFORNIA	Date Drill Boring De Hole Diar Drilling M	epth neter ethod		low-Stem	-		Backfill Material : Bentonite Grout Backfill Interval : 75' - 0' Logged By : D. Kawasaki
-			<u> </u>	01437-01 Sampler Type	Sampling	Metho	.28	Split Spoo			
f	epth in eet	GRAPHIC		Split Spoon Split Spoon Standard Penetrometer Grab Other DESCRIPTION		Samples	Sample #	Blows/Ft	o∨m	Lab Results (TPH)	REMARKS
	30	M	IL s	Dark yellowish-brown, moist, stiff, SILT	very	X	CB2-30	30	1		No odor in 30' sample.
and the second second second second second second second second second second second second second second secon	- 35 — -		11	Dark yellowish-brown, moist, stiff, SILT	very		CB2-35	26	<1		No odor in 35' sample.
1977 (A.A.)	-  40 	N		Dark yellowish-brown, moist, stiff, SILT; with minor fine sar	very nd	$\square$	CB2-40	28	<1		No odor in 40' sample.
137-01/CB2.BOR	- 45- -	s	;P	Dark yellowish-brown, moist, dense, fine SAND; with mino	very r silt	$\boxtimes$	CB2-45	54	<1		No odor in 45' sample.
U~1/BORING~1/BORING~1/4	- 50	s	SP	Dark yellowish-brown, moist, fine SAND; with minor silt	dense,		CB2-50	48	<1		No odor in 50' sample.
C*0CUME~14EM2MYD0CU~1/BORING~1/BORING~1437-01/CB2	- 55 - -	s	SP	Moderate yellowish-brown, da moist, very dense, fine SANE some medium sand, minor ca sand and trace gravel	; with	$\square$	CB2-55	58	<1		No odor in 55' sample.
07-16-2008	- 60 -										

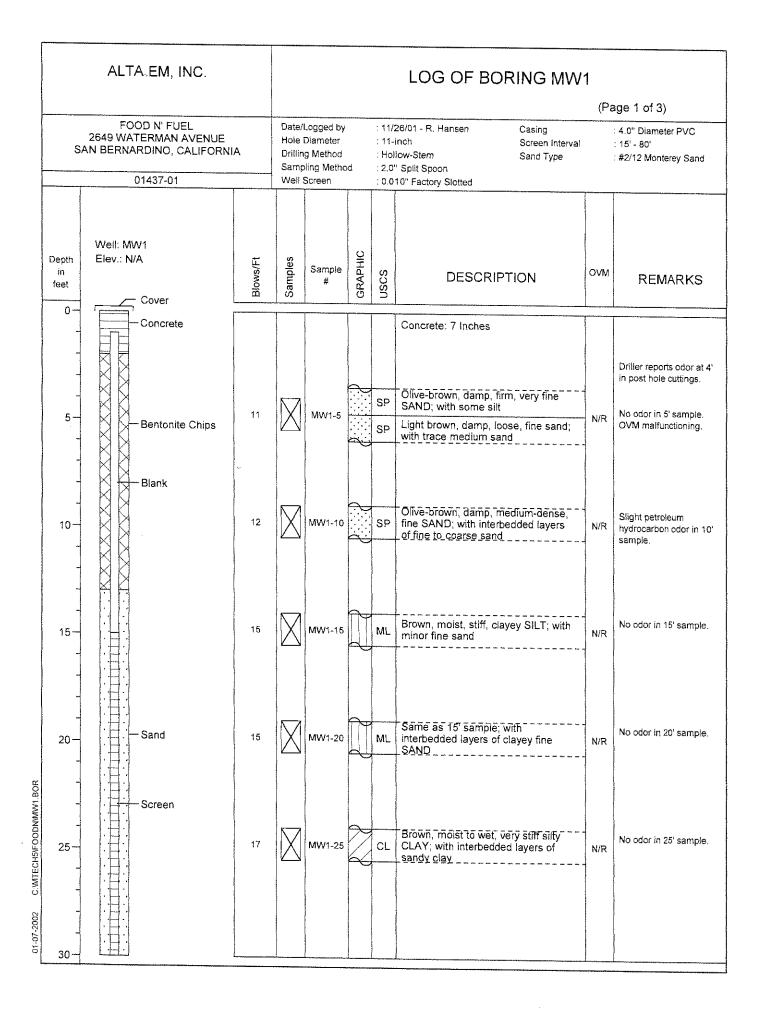
		ALT	ΓΑ ΕΜ, INC.				LO	GO	F BOF	RING CB2
-										(Page 3 of 3)
S		) S. W	DOD N' FUEL VATERMAN AVENUE ARDINO, CALIFORNIA	Date Drill Boring De Hote Diar Drilling M	epth neter ethod		ow-Stem			Backfill Material : Bentonite Grout Backfill interval : 75' - 0' Logged By : D. Kawasaki
	1		01437-01 Sampler Type	Sampling	Metho	d : 2" S	plit Spoo	n 		
Depth in feet	GRAPHIC	nscs	Sampler Type Split Spoon Standard Penetrometer Grab Other DESCRIPTION		Samples	Sample #	Blows/Ft	оум	Lab Results (TPH)	REMARKS
60-		SP	Moderate yellowish-brown, da moist, very dense, fine SAND some medium sand, minor co sand and trace gravel	); with		CB2-60	79	<1		No odor in 60' sample. Driller reports gravelly zone at 60'. Much slower drilling.
65		GP SP		amp, mínor		CB2-65	87	<1		No odori n 65' sample.
70-		SP	Dark yellowish-brown, damp very dense, fine SAND; with medium to coarse sand and g	some	$\boxtimes$	CB2-70	85	<1		No ador in 70' sample.
1437-01/CB2.B0R		ML	Moderate olive-brown, moist, SILT	hard,		CB2-75	64	<1		No odor in 75' sample.
-08 -08 -08					na da anti da anti da anti da anti da anti da anti da anti da anti da anti da anti da anti da anti da anti da a					
C.°OCUME-1/AEM2MYDOCU-1/BORING-1/BORING-1/437-01/CB2.BOR - 08 - 08										
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		,	TA EM, INC.				LC	00		(Page 1 of 2)
		9 S. V	OOD N' FUEL VATERMAN AVENUE ARDINO, CALIFORNIA	Date Dril Boring D Hole Dia Drilling N	epth meter 1ethod	: 40' : 1.2 : Dire	ect Push		be 6600)	Backfill Material: Bentonite ChipsBackfill Interval: 40' - 0'Logged By: D. Kawasaki
			01437-01	Sampling	g Metho	od : 1.0'	' Piston S	Sampler		
Depth in feet	GRAPHIC	USCS	Sampler Type Split Spoon Standard Penetrometer Grab Other DESCRIPTION		Samples	Sample #	Blows/Ft	OVM	Lab Results (TPH)	REMARKS
0-	-		6" Concrete Surface							
5-	- - - - - - - - - -	SM	Moderate yellowish-brown, da silty fine SAND	amp,		CB3-5		<1		No odor in 5' sample.
10-	- - 	SM	Moderate yellowish-brown, da silty fine SAND	amp,		CB3-10		<1		No odor in 10' sample.
15-		ML	Dark yellowish-brown, damp moist, sandy SILT			CB3-15		<1		No odor in 15' sample.
20-		ML	Dark yellowish-brown, damp sandy SILT	to moist		CB3-20		<1		No odor in 20' sample.
25-	- - - - - - - - - - - - - - - - - - -	SM	Moderate yellowish-brown, da moist, silty fine SAND	amp to	<b>1 1</b>	CB3-25		<1		No odor in 25' sample.
30-	-									

			AL	TA EM, INC.				LO	G O	F BOF	RING CB3
											(Page 2 of 2)
-	S		9 S. V	OOD N' FUEL VATERMAN AVENUE IARDINO, CALIFORNIA	Date Drill Boring D Hole Diar Drilling N	epth meter lethod	: 40' : 1.2 : Dire	ect Push		be 6600)	Backfill Material: Bentonite ChipsBackfill Interval: 40' - 0'Logged By: D. Kawasaki
ł				01437-01 Sampler Type	Sampling	Metho	ba : 1.0	" Piston S	sampier		
-	Depth in feet	GRAPHIC	NSCS	Split Spoon Split Spoon Standard Penetrometer Grab Other DESCRIPTION		Samples	Sample #	Blows/Ft	OVM	Lab Results (TPH)	REMARKS
	30- - -		ML	Dark yellowish-brown, damp moist, sandy SILT	to 		CB3-30		<1		No odor in 30' sample.
	- 35 — -		SP	Dark yellowish-brown, damp moist, fine SAND; with minor medium to coarse sand	to silt and		CB3-35		1		No odor in 35' sample.
	- - 40 -		SM	Dark yellowish-brown, damp moist, silty fine SAND; with m medium to coarse sand	to inor		CB3-40		<1		No odor in 40' sample.
G~1\437-01\CB3.BOR	- 45 — - -	-									
C:°OCUME~1\USER\MYDOCU~1\BORING~1\BORING~1\437-01\CB3.BOR	- 50 — - -	-									
02-28-2012 C:ºOCUME~1\USEF	55 — - - - - 60 —	-									





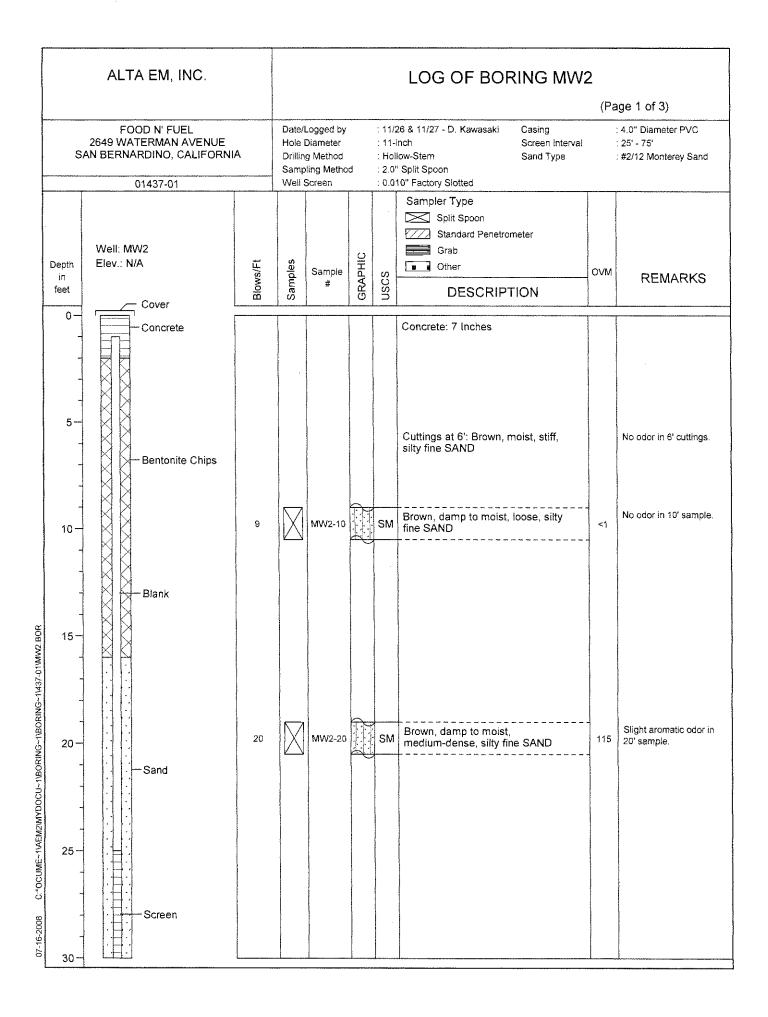


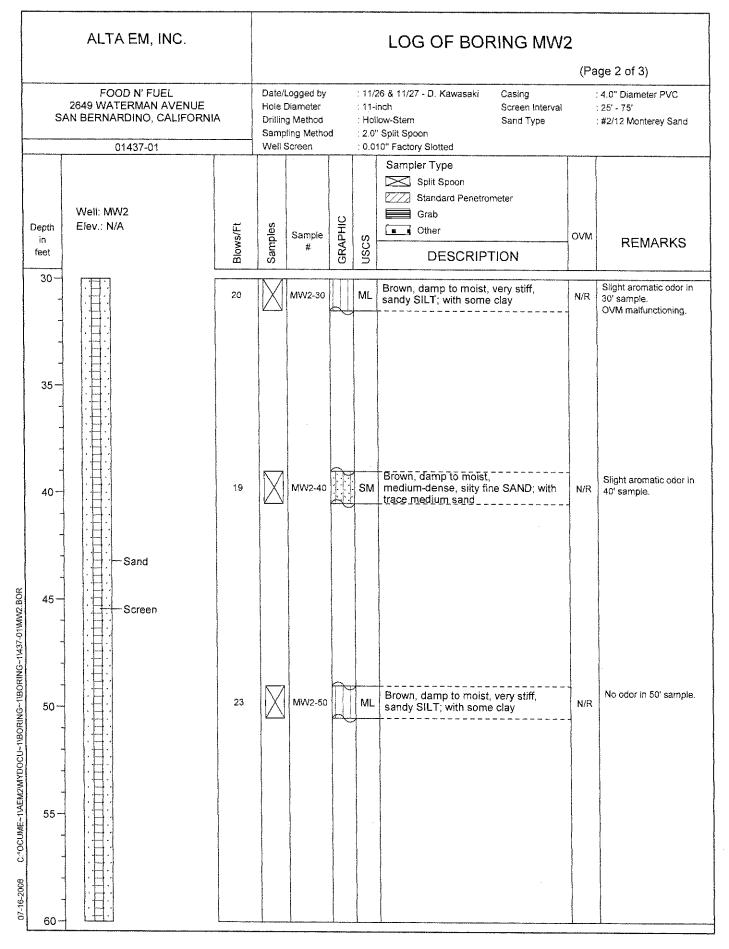
ALTA EM, INC.			LOG OF BORING MW1 (Page 2 of 3)								
FOOD N' FUEL 2649 WATERMAN AVENUE SAN BERNARDINO, CALIFORNIA				Date/Logged by Hole Diameter Drilling Method Sampling Method		: 11/26/01 - R. Hansen Casing : 11-inch Screen Interval : Hollow-Stem Sand Type : 2.0" Split Spoon		: 4.0" Diameter PVC : 15' - 80' : #2/12 Monterey Sand			
	01437-01		Well S	Screen	: 0	010" Factory Slotted					
Depth in feet	Well: MW1 Elev.: N/A	Blows/Ft	Samples	Sampie #	GRAPHIC	DESCRIP		<sup>4</sup> REMARKS			
30		17	X	MW1-30	M	Brown, damp to moist clayey SILT	, very stiff, N/F	No odor in 30' sample.			
35-		17	$\boxtimes$	MW1-35	MI	Same as 30' sample	N/F	No odor in 35' sample.			
40		20	$\square$	MW1-40	MI	Brown, damp to moist SILT	, very stiff, N/F	No odor in 40' sample.			
45	Screen	19	$\square$	MW1-45	SI SI	Brown, damp to moist medium-dense, fine S minor sitt	AND; with N/F	No odor in 45' sample.			
- 50  		22		MW1-50	Sh	Brown, wet, medium-c very fine SAND	Jense, silty N/F	No odor in 50' sample.			
- - - -		27	$\square$	MW1-55	SF	Light-brown, damp, m fine SAND; no fines, t sand	race medium N/F	No odor in 55' sample.			
60-											

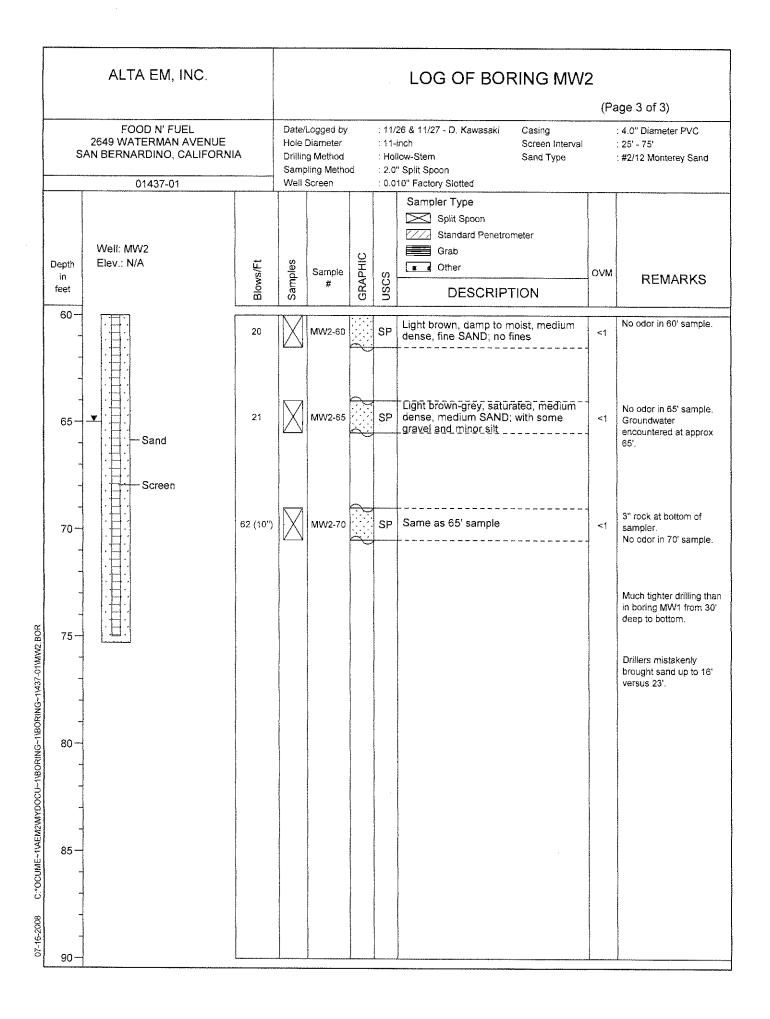
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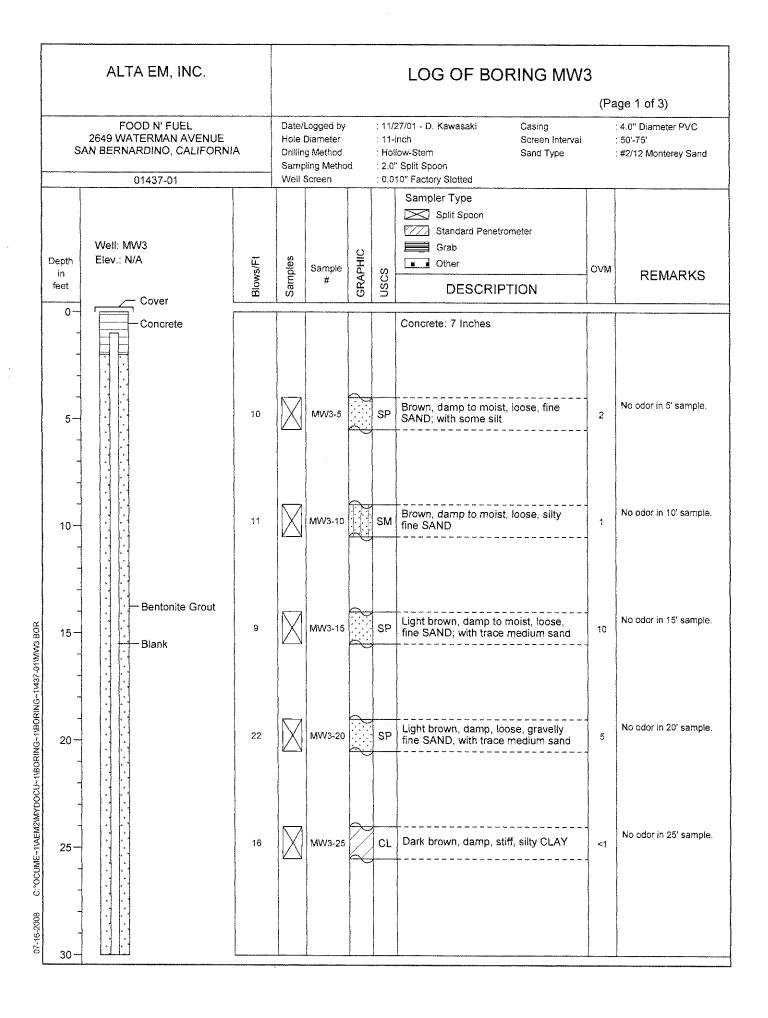
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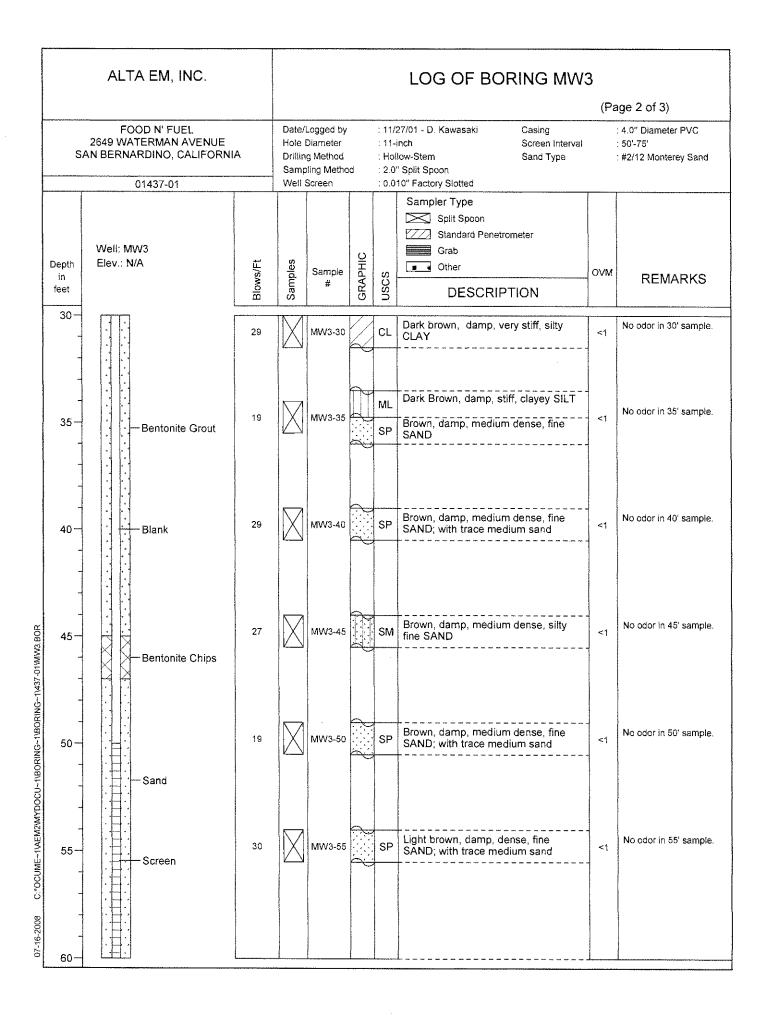
ALTA EM, INC. FOOD N' FUEL 2649 WATERMAN AVENUE SAN BERNARDINO, CALIFORNIA 01437-01			LOG OF BORING MW1								
			Date/Logged by Hole Diameter Drilling Method Sampling Method Well Screen		: 11/26/01 - R. Hansen Casing : 11-inch Screen Inter : Hollow-Stem Sand Type : 2.0" Split Spoon : 0.010" Factory Slotted		creen Interval	(Page 3 of 3) : 4.0" Diameter PVC : 15' - 80' : #2/12 Monterey Sand			
Well: MW1 Depth Elev.: N/A feet	Blows/Ft	Sampies	Sample #	GRAPHIC	uscs	DESCRIPTIO	N	o∨M	REMARKS		
65- <b>•</b>	50 (1") 59 (10") 67 (8")		MW1-65		SP	Grey-brown, saturated, ver gravelly fine to medium SA is fine to coarse and suban Brown, saturated, very den gravelly fine SAND; mucky	ND; gravel     gular se sitv	N/R	No recovery at 60' Gravel at 65'. No odor in 65' sample. A few inches of water on 65' sample. Groundwater encountered at approx 65'. Gravel at 70'. No odor in 70' sample. B' of water on down-hole hammer.		

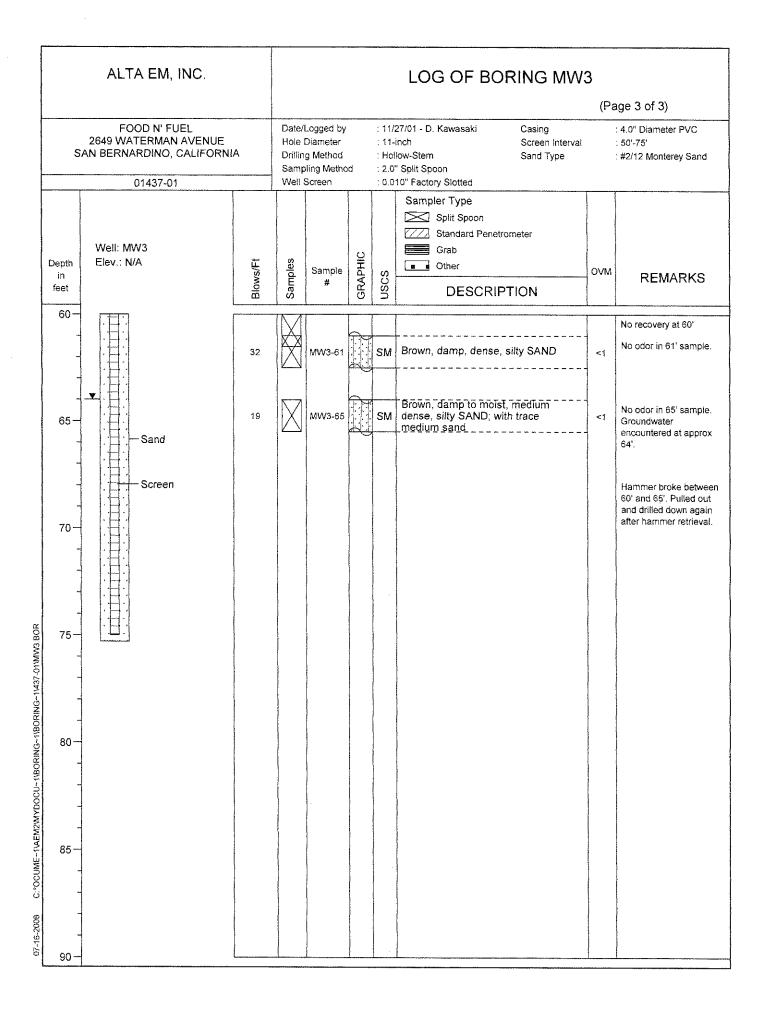


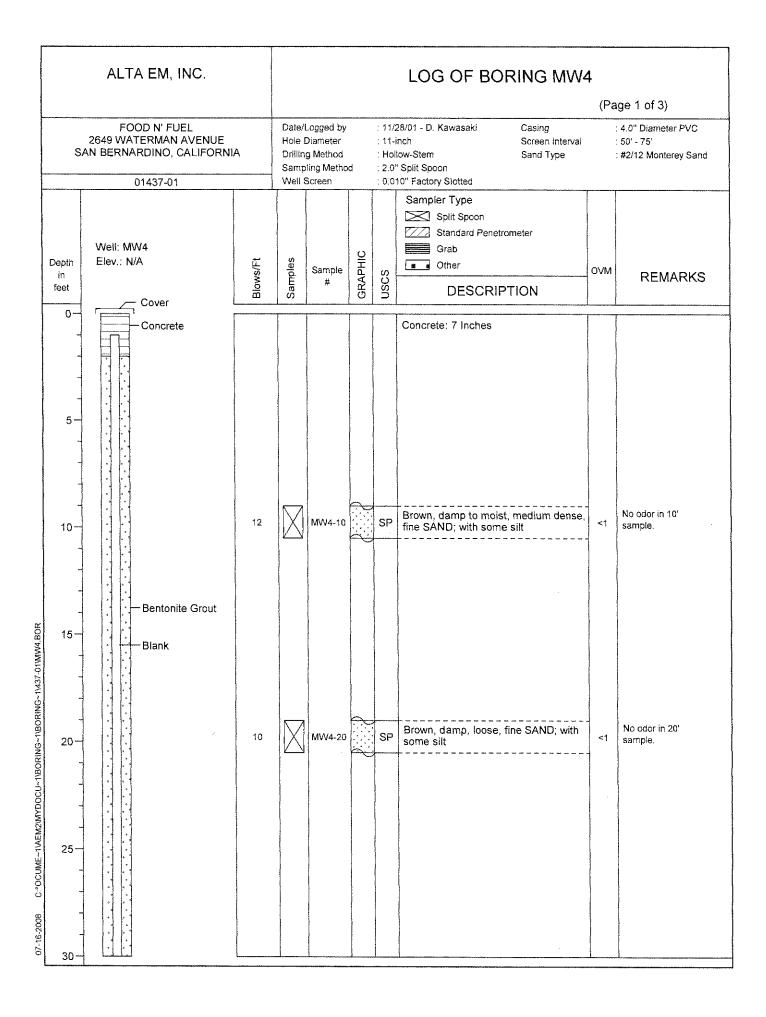


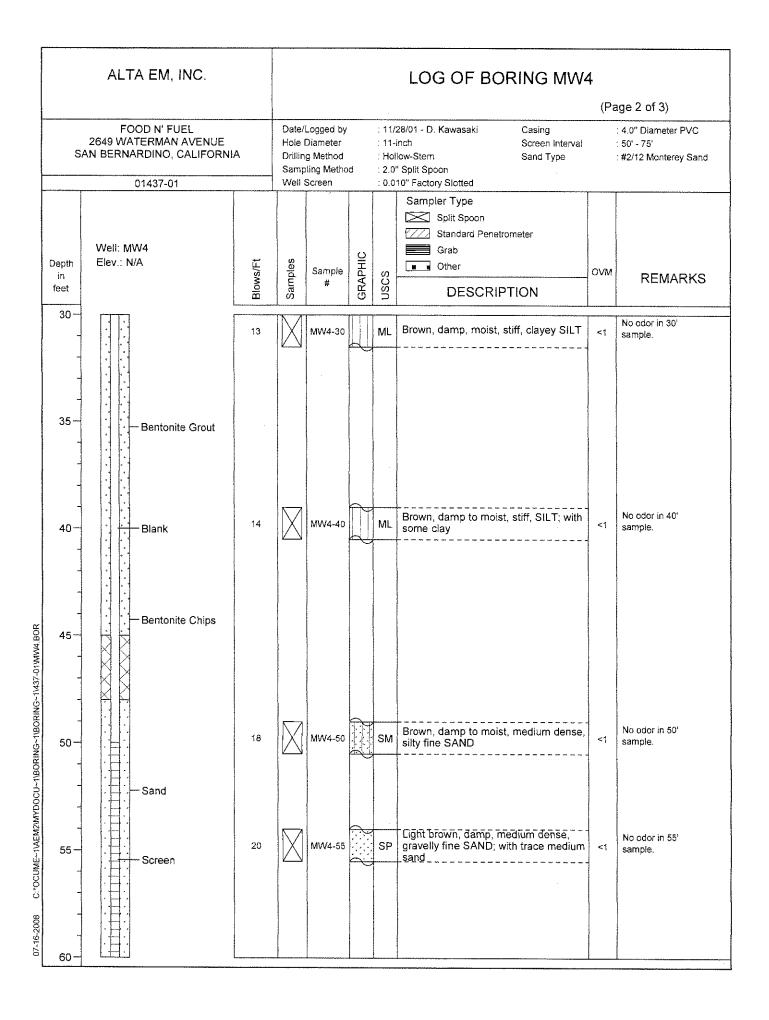


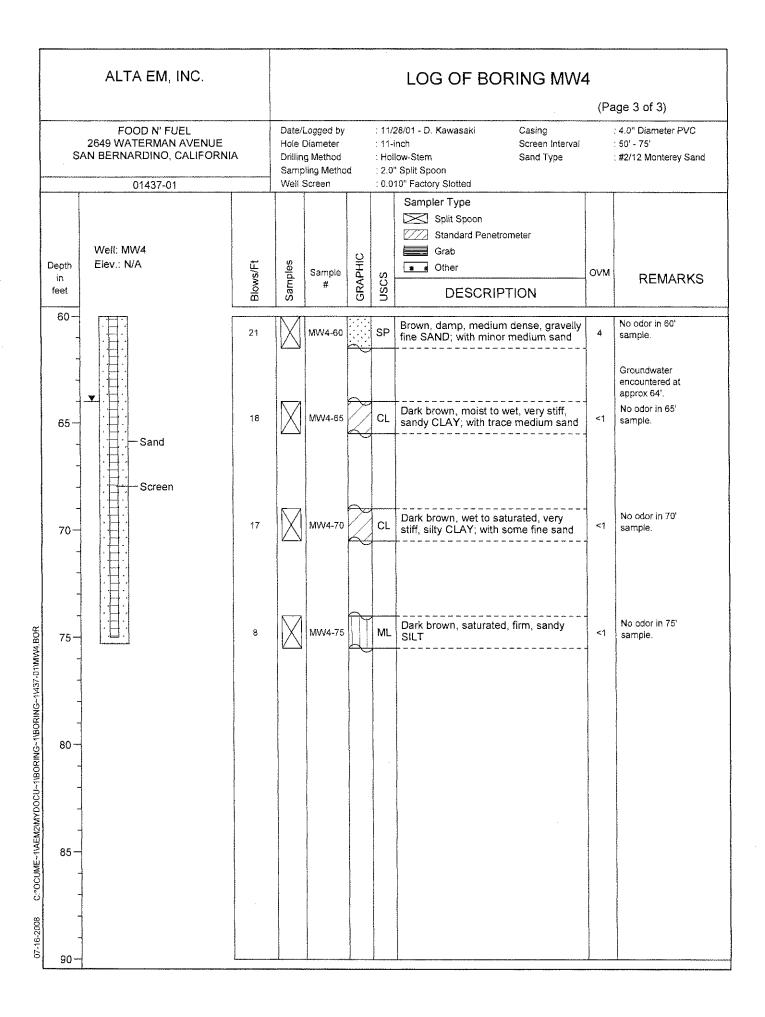


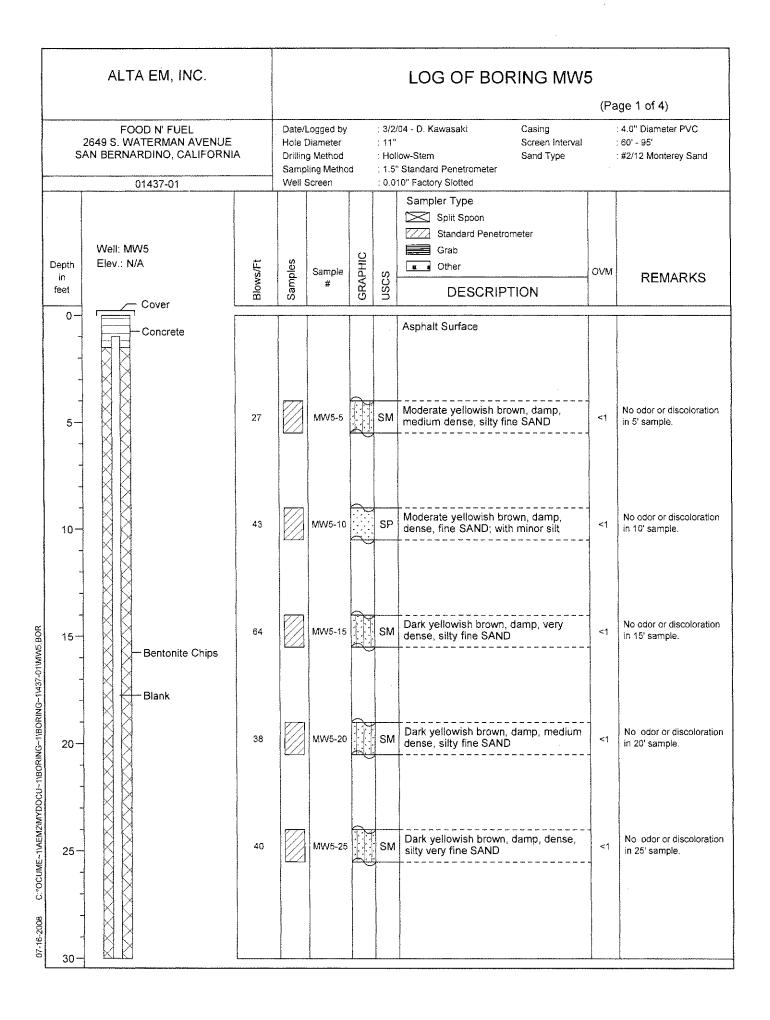


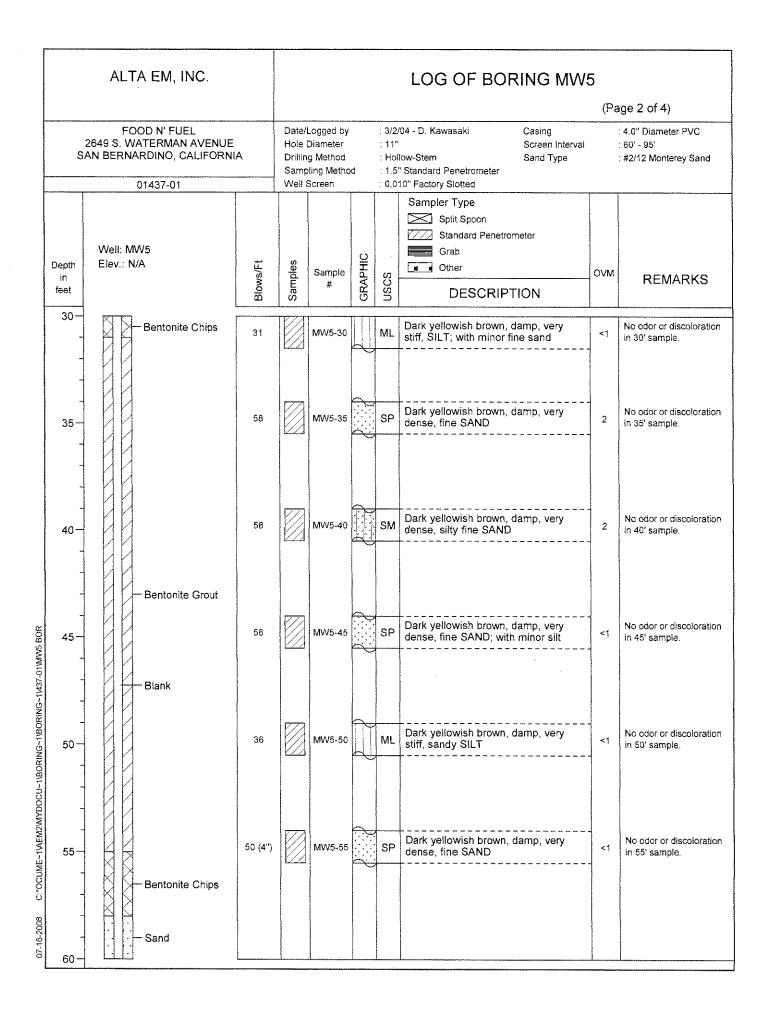


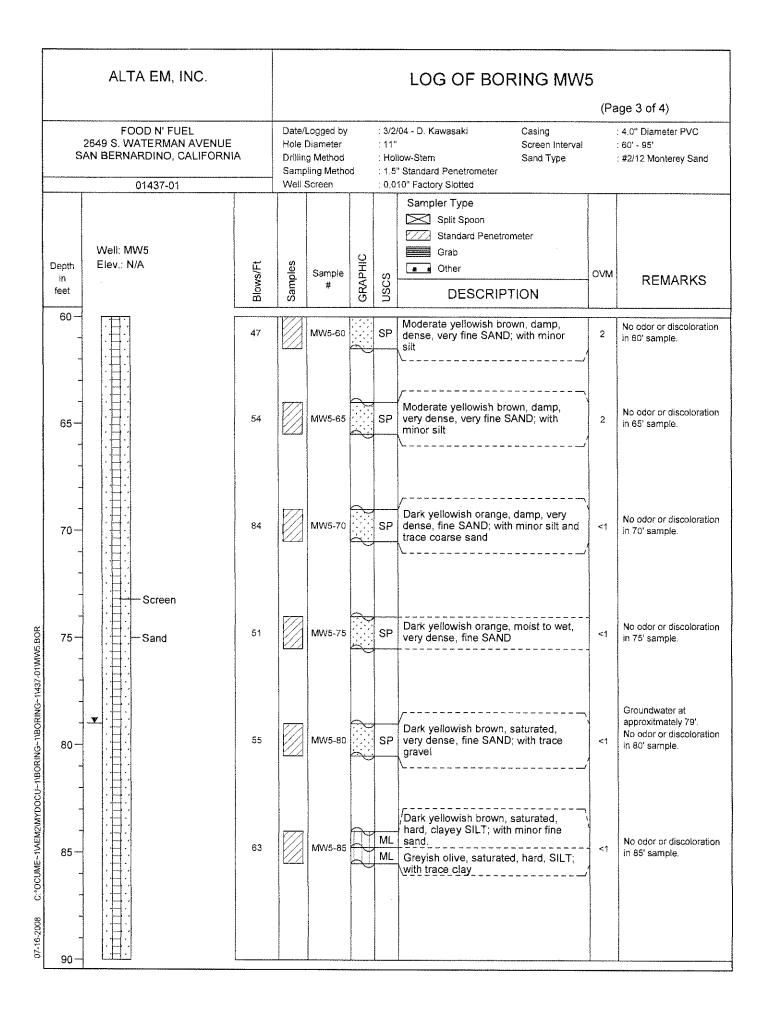


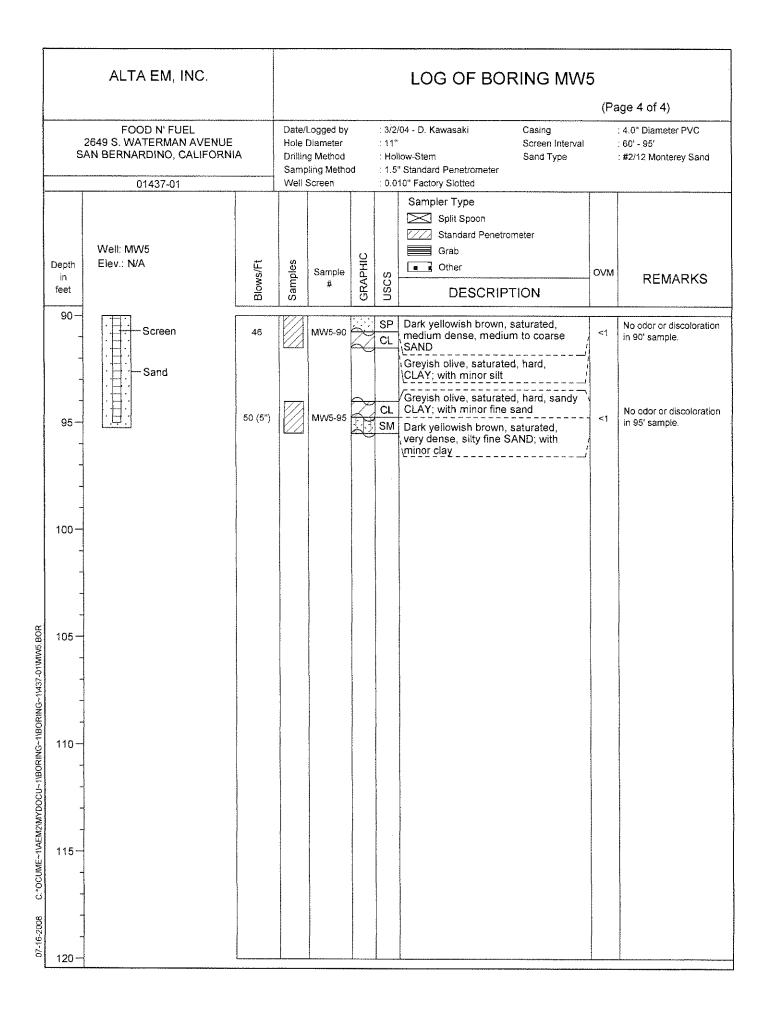








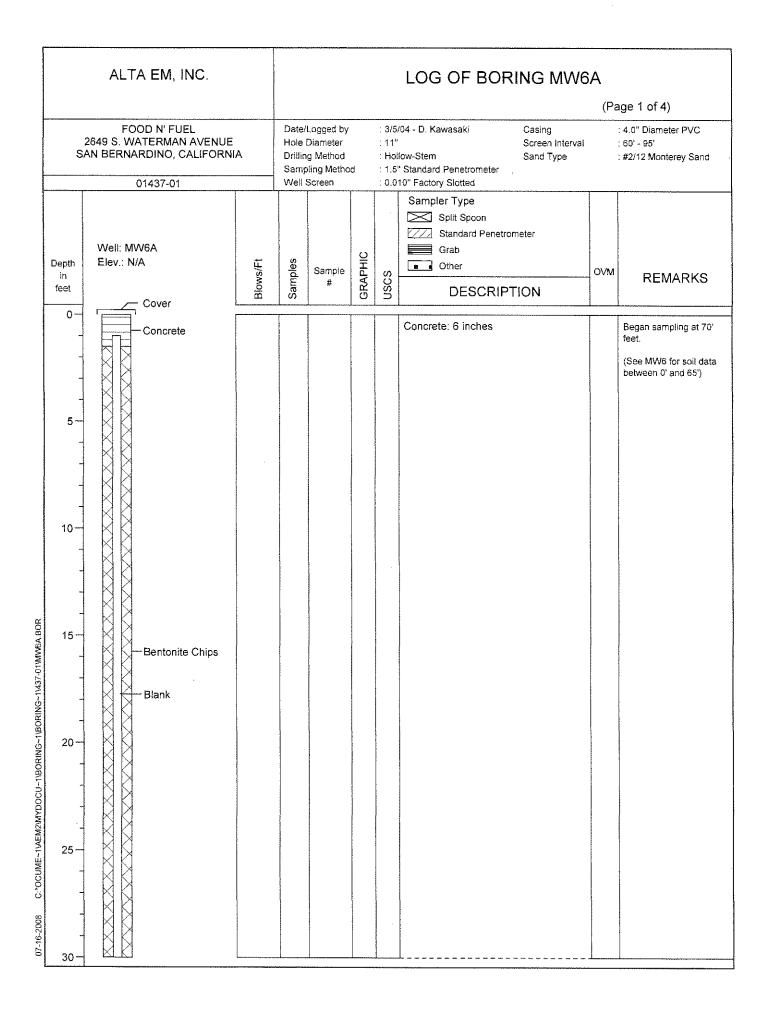


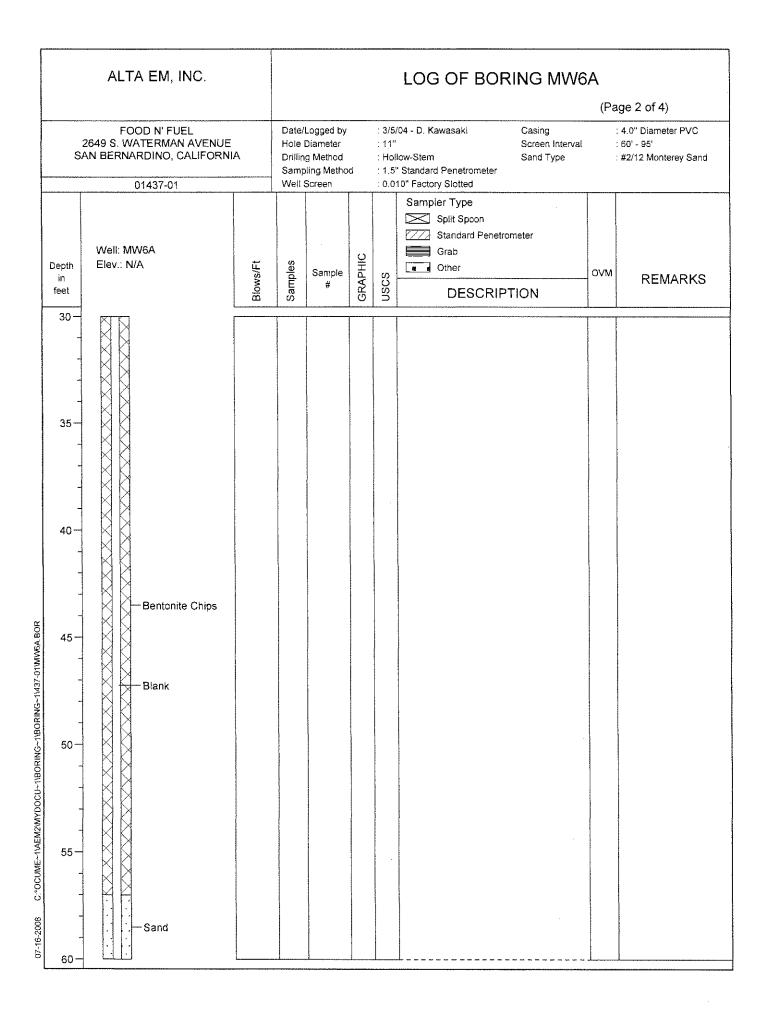


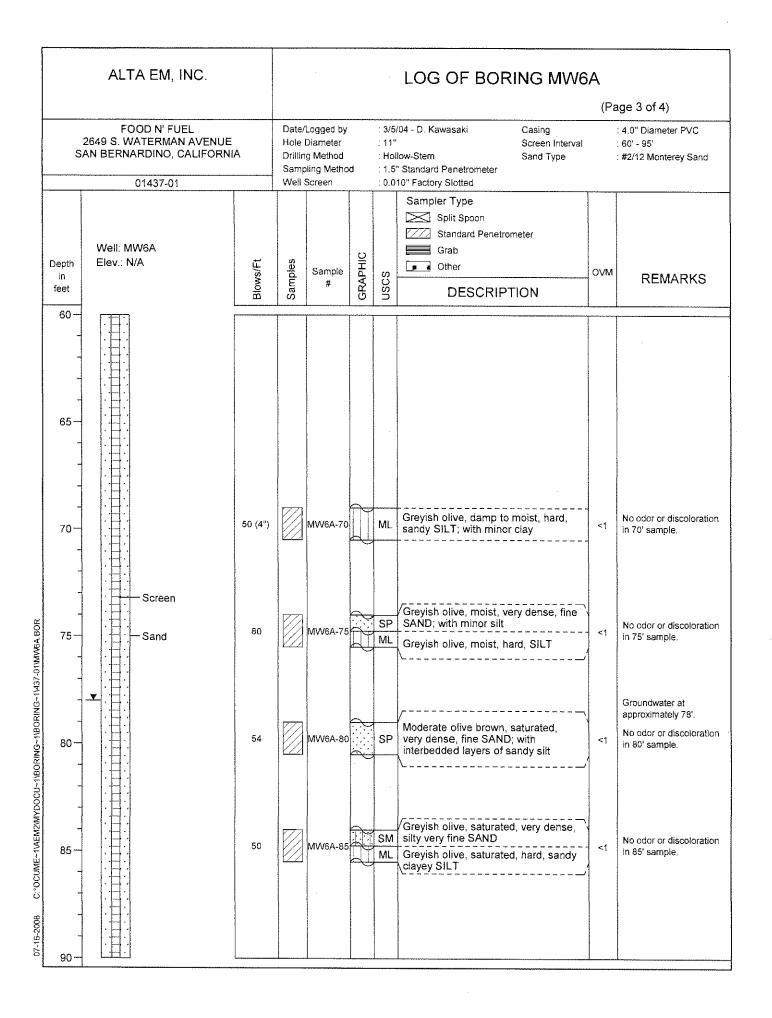
				-	(Page 1								
FOOD N' FUEL 2649 S. WATERMAN AVENUE SAN BERNARDINO, CALIFORNIA			Date Dril Boring D Hole Dia Drilling N	epth meter Aethod		low-Stem	Backfill Material : Bentonite Chips Backfill Interval : 68' - 0' Logged By : R. Hansen						
Depth in feet	GRAPHIC	uscs	01437-01 Sampler Type Split Spoon Standard Penetrometer Grab Other DESCRIPTION	Sampling	o ditemples	d : 1.5' Sample #	Standar	OVM	Lab Results (TPH)	F	REMARKS		
0-			Concrete: 6 inches										
- - 5 -		SM	Brown, moist, medium dens fine SAND, with interbedded layers	e, silty silt		MW6-5	32	<1		No odor or discolo	ration in 5' sample.		
- - 10- -		ML	Brown, moist, stiff, clayey S	ILT 		MW6-10	14	3		No odor or discolo	ration in 10' sample.		
- 15-		SP	Brown, damp, medium dens SAND; with some silt	e, fine		MW6-15	29	<1		No odor or discolo	ration in 15' sample.		
- 20 -		SM	Brown, damp, dense, silty fi SAND; with some coarse sa fine gravel; with interbedded of clayey silt	nd to		MW6-20	54	2		No odor or discolo	pration in 20' sample.		
25-		ML	Brown, damp, very stiff, SIL minor clay	T; with		MW6-25	24	3		No odor or discolo	oration in 25' sample.		

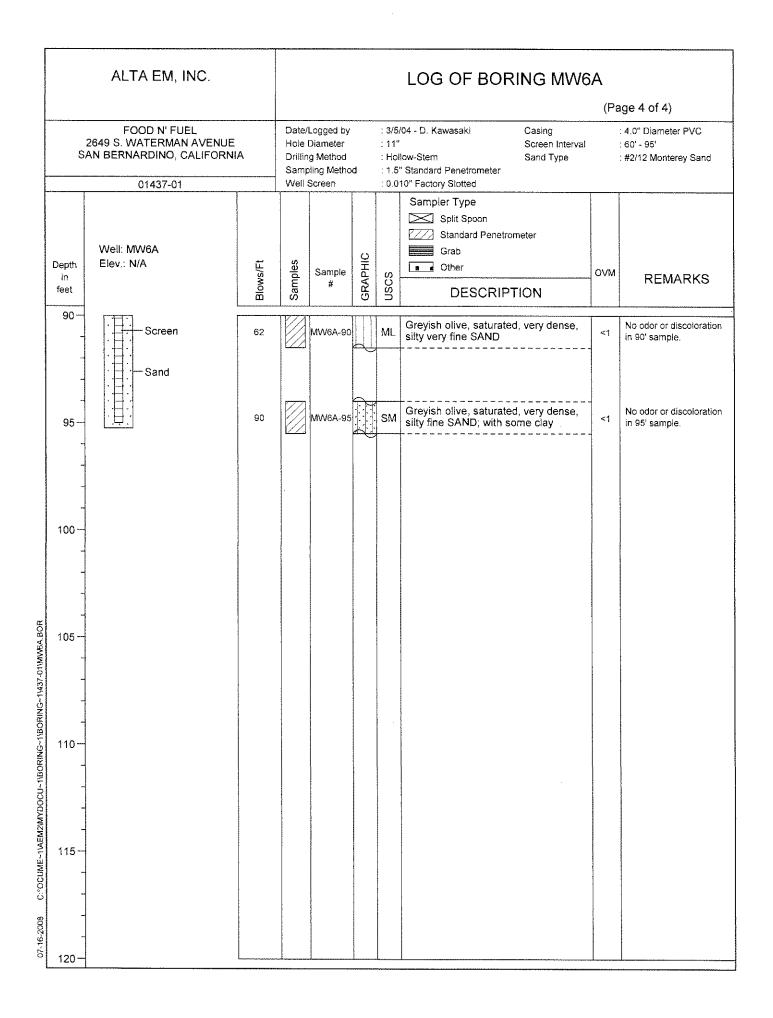
	ALTA EM, INC.			LOG OF BORING MW6									
									(Page 2 of 3)				
	2649 S.	FOOD N' FUEL WATERMAN AVENUE NARDINO, CALIFORNIA	Date Drilled Boring Dept Hole Diame Drilling Meth	th eter hod		low-Stern			Backfill Material : Bentonite Chips Backfill Interval : 68' - 0' Logged By : R. Hansen				
		01437-01 Sampler Type	Sampling M	lethoo	1 : 1.5'	' Slandaro	I Penetro	ometer					
Depth in feet	GRAPHIC	Split Spoon ZZZ Standard Penetrometer Grab		Samples	Sample #	Blows/Ft	OVM	Lab Results (TPH)	REMARKS				
30-		L Brown, damp, very stiff, claye	y SILT		MW6-30	25	2	-	No odor or discoloration in 30' sample.				
35-	SI -	Yellowish brown, damp, dens fine SAND	e, very		MW6-35	41	<1		No odor or discoloration in 35' sample.				
40-	SI	Yellowish brown, damp, dens SAND; no fines and trace me sand	e, fine dium		MW6-40	57	14		No odor or discoloration in 40' sample.				
437-011MW6.BOR	- SI	Brown, damp, very dense, sil M SAND; with trace coarse san clay	ty fine d and		MW6-45	50 (5")	<1		No odor or discoloration in 45' sample.				
C.ºOCUME-1/AEM2MYYDOCU-1/BORING-1/BORING-1/437-01MWB	SI	Light brown, damp, very dens to medium SAND; with some fine gravel, no fines	se, fine angular		MW6-50	71	<1		No odor or discoloration in 50' sample.				
C.ºOCUME~1VEM2MYDOC	SI SI	Light brown, damp, very dens to medium SAND; with some fine gravel, no fines	se, fine angular		MW6-55	63	<1		No odor or discoloration in 55' sample.				
07-16-2008	-												

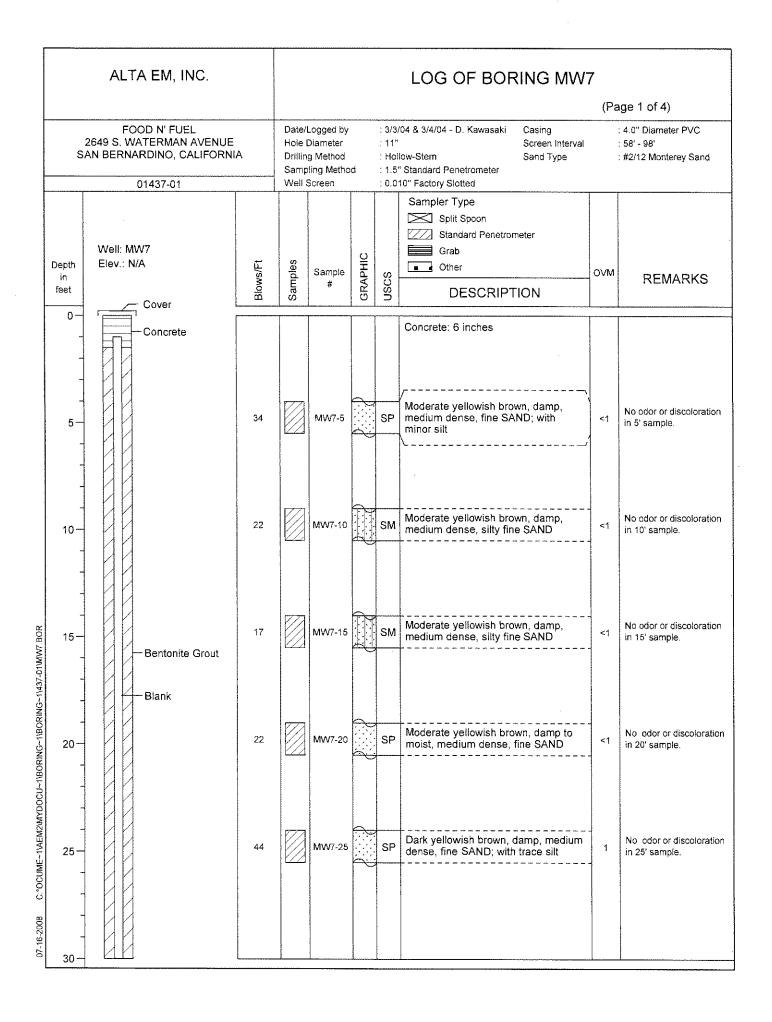
		۸L	TA EM, INC.				LO	g of	- BOR	ING MW6
										(Page 3 of 3)
		es. v	DOD N' FUEL VATERMAN AVENUE ARDINO, CALIFORNIA	Date Drill Boring De Hole Diar Drilling M Sampling	epth meter lethod				omotor	Backfill Material : Bentonite Chips Backfill Interval : 68' - 0' Logged By : R. Hansen
			01437-01 Sampler Type	Samping		G. 1.0	Standar	Peneu	ometer	
Depth in feet	GRAPHIC	USCS	Split Spoon Standard Penetrometer Grab Other DESCRIPTION		Samples	Sample #	Blows/Ft	o∨m	Lab Results (TPH)	REMARKS
60-		SP	Grey brown, damp, very dens gravelly fine sand; gravel is fi coarse and subangular	se, ne to /		MW6-60	50 (4")	<1		No odor or dicoloration in 60' sample. Driller reports abundant gravel at ~60'.
65-		SP	Grey brown, damp, very dens gravelly fine sand; gravel is fi coarse and subangular	se, ne to		MW6-65	70 (5")			No odor or discoloration in 65' sample.
70.		****								Cannot advance past 68' due to coarse gravel and likely cobbles. Pulled out augers to check bit. Bit OK. Driller indicates that they cannot proceed in this hole. Moved to new location 12 feet away and began drilling MW6A.
Y00-75										
80		An ann an								
907-91-/0 90	-									

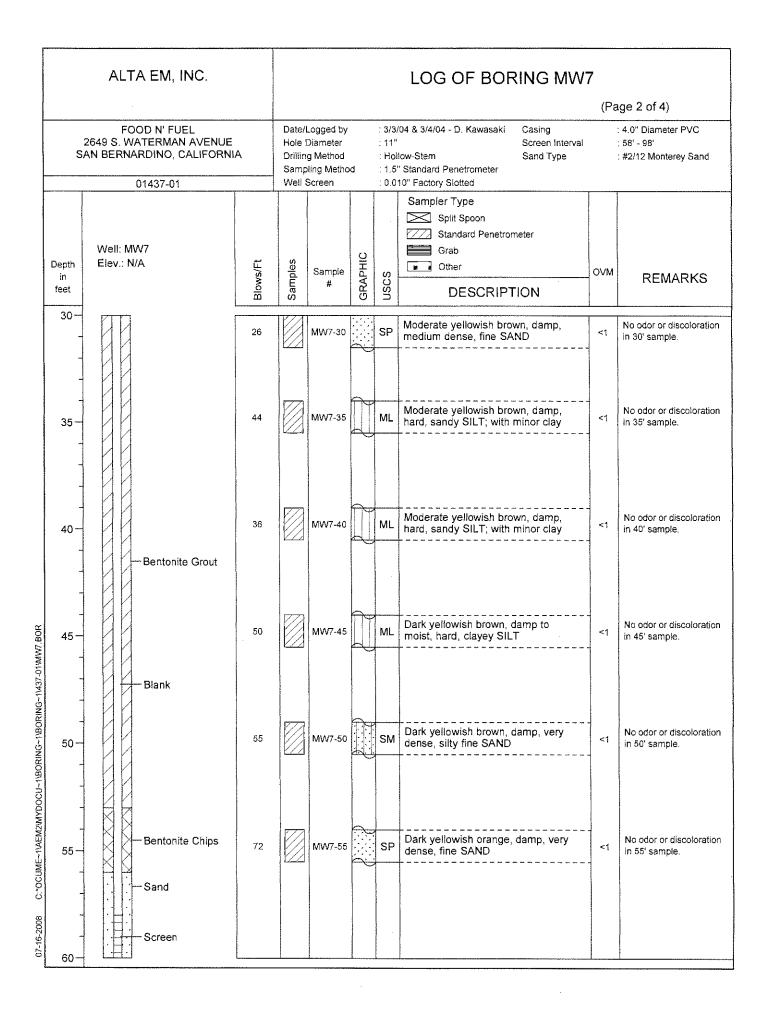


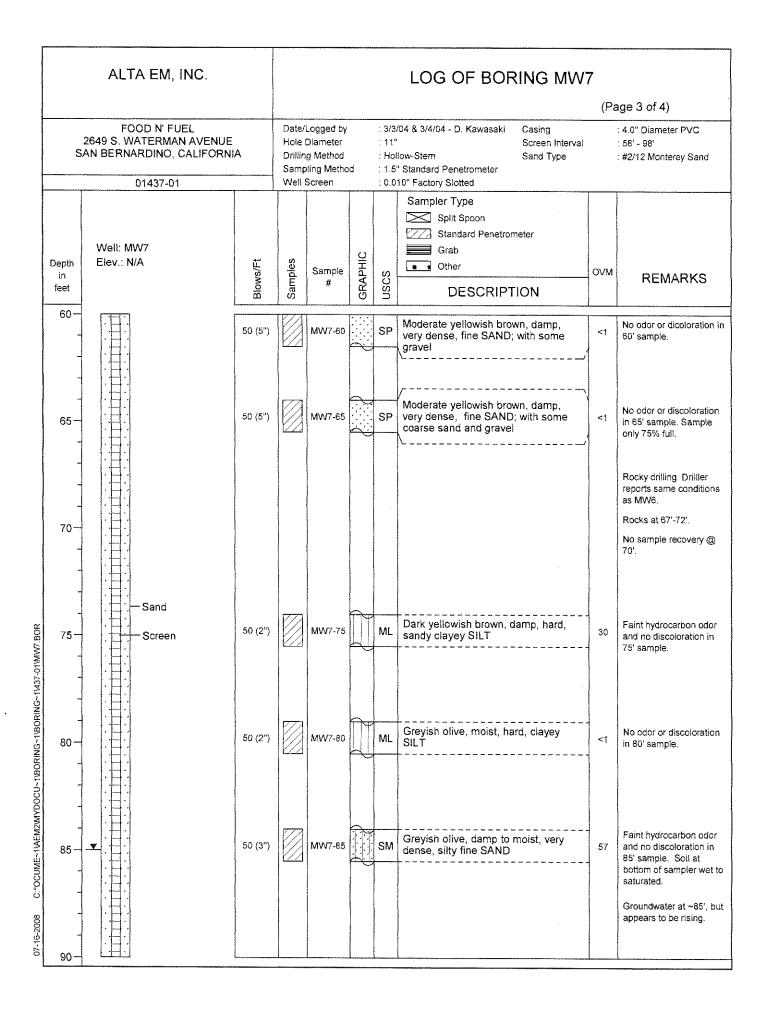


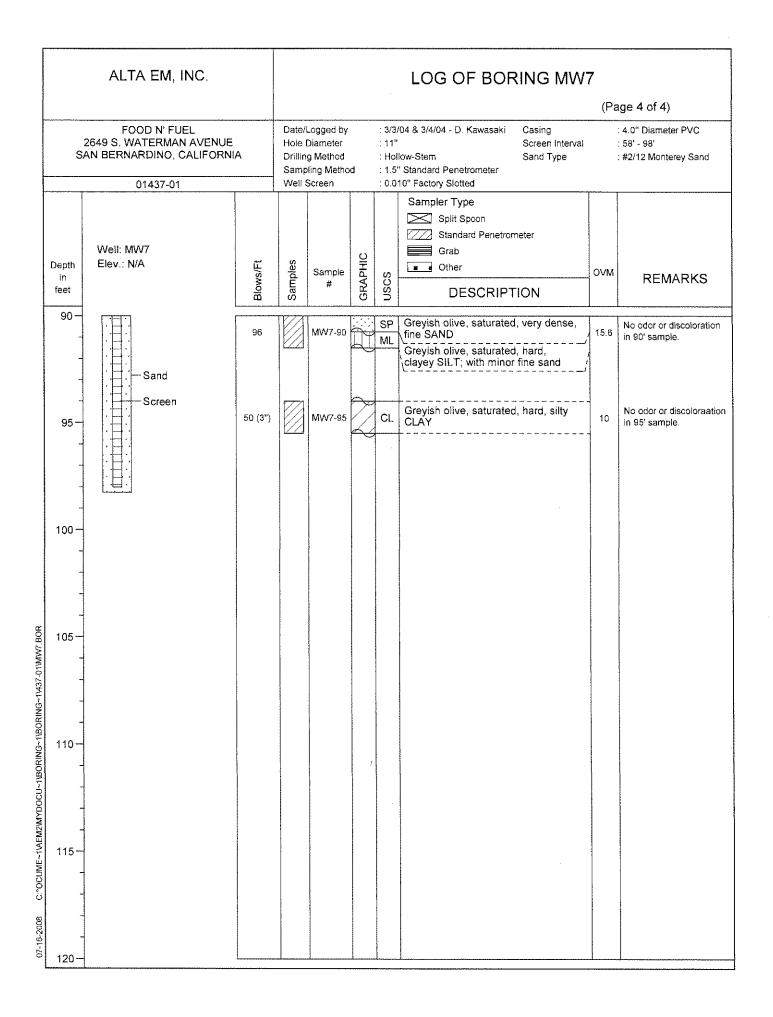


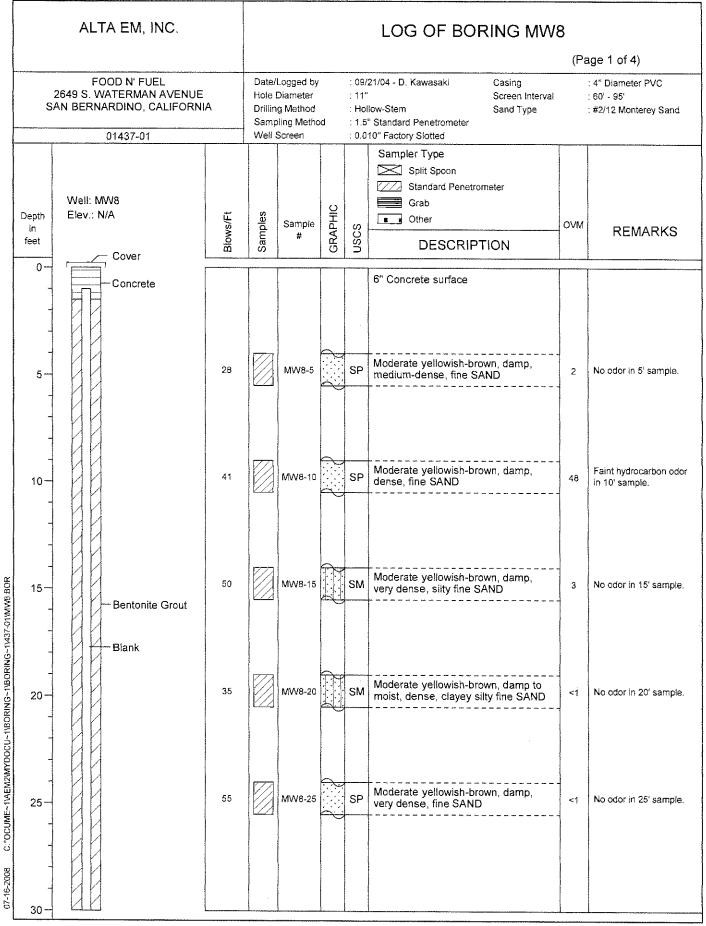




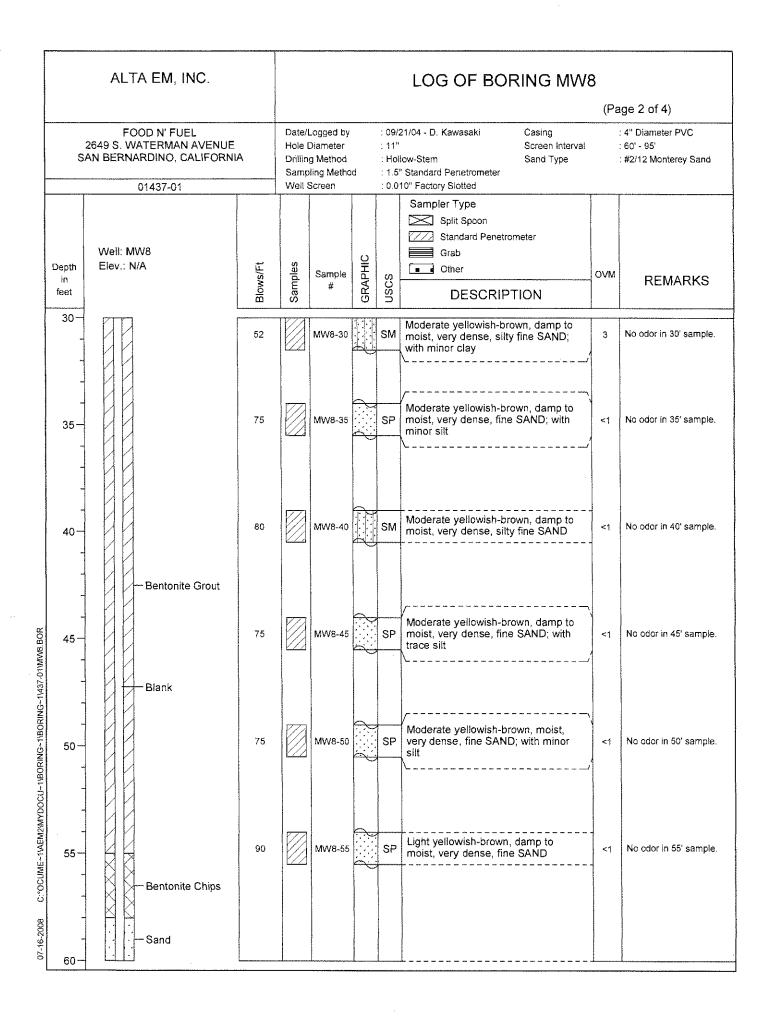


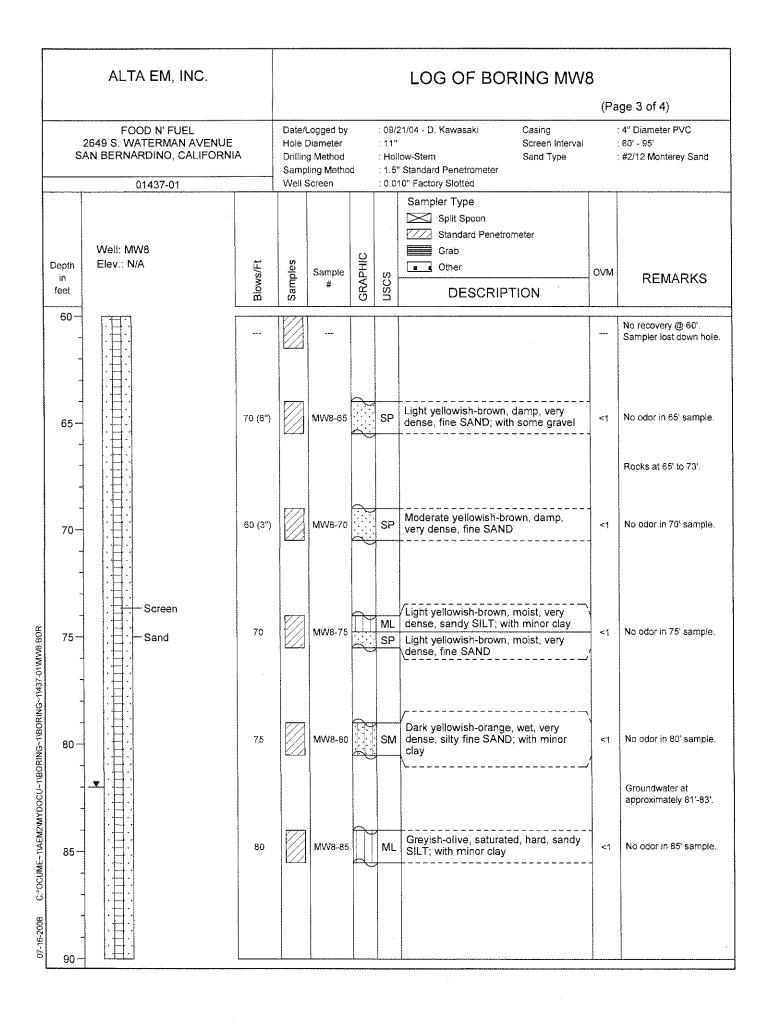




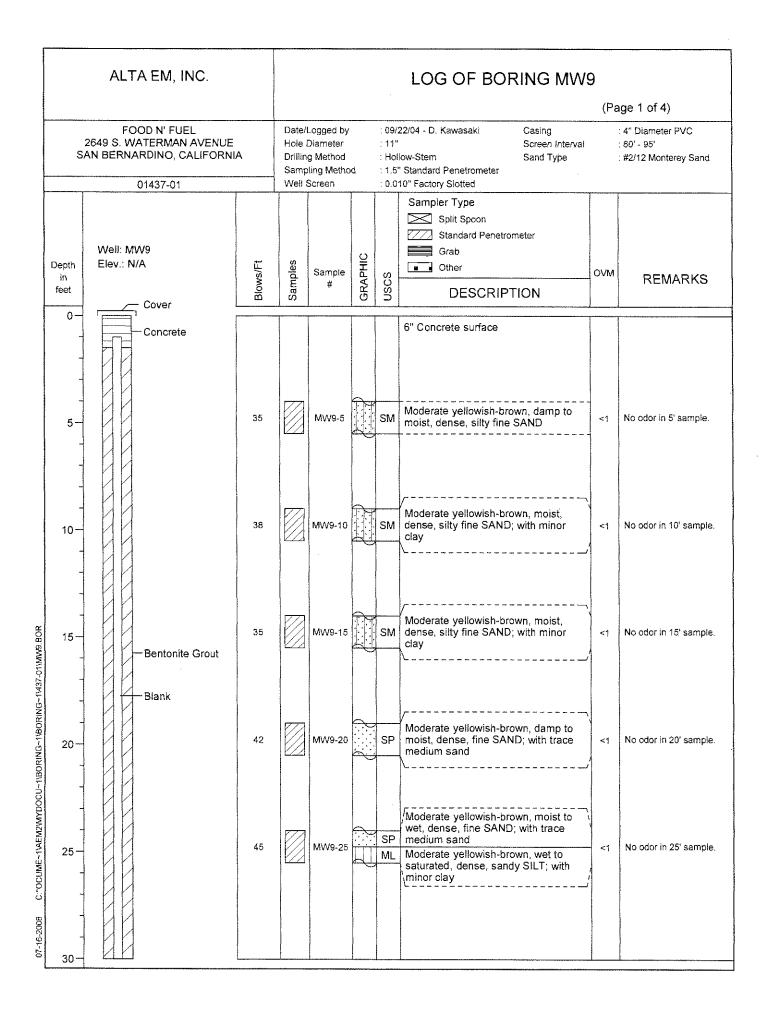


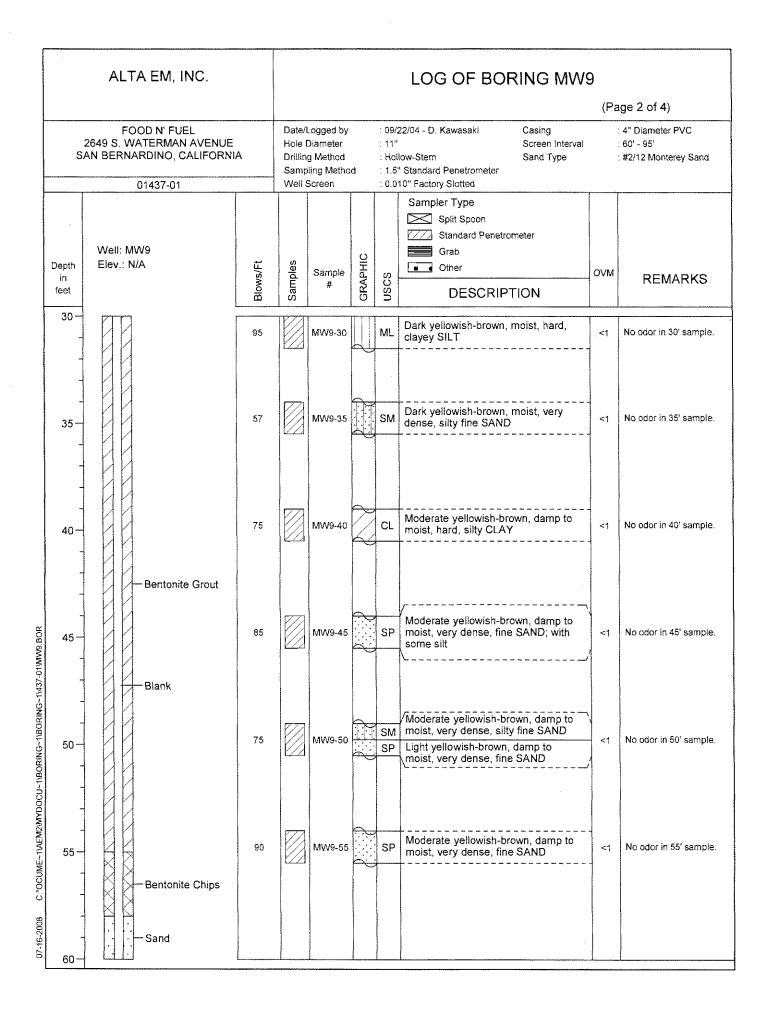
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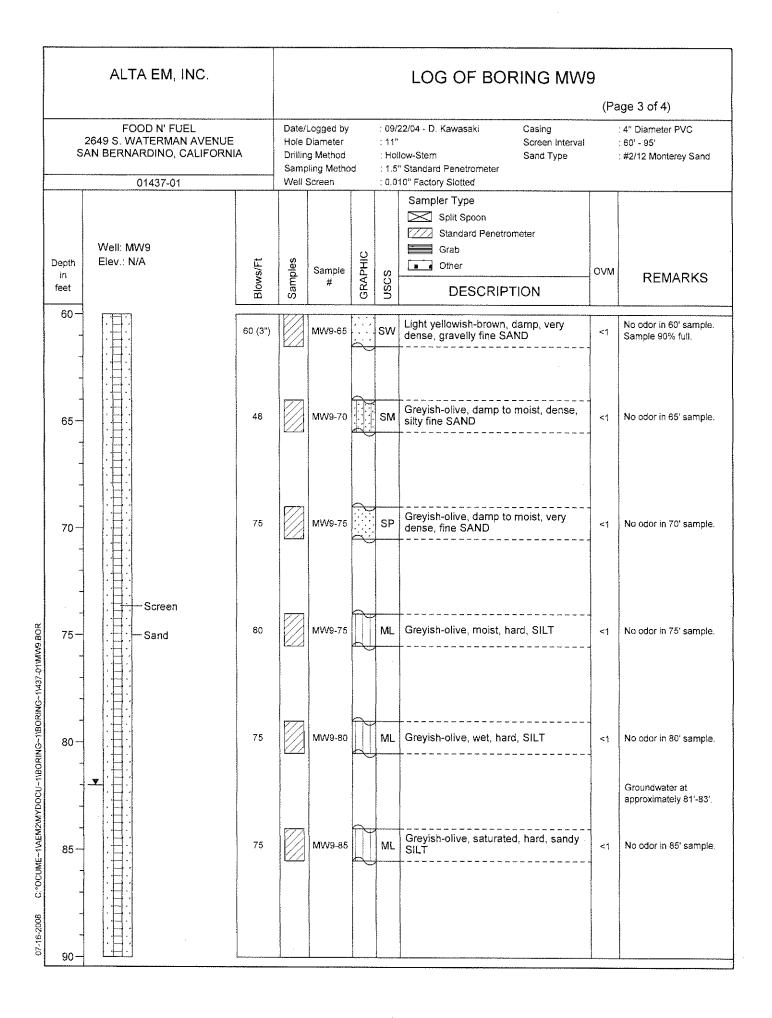


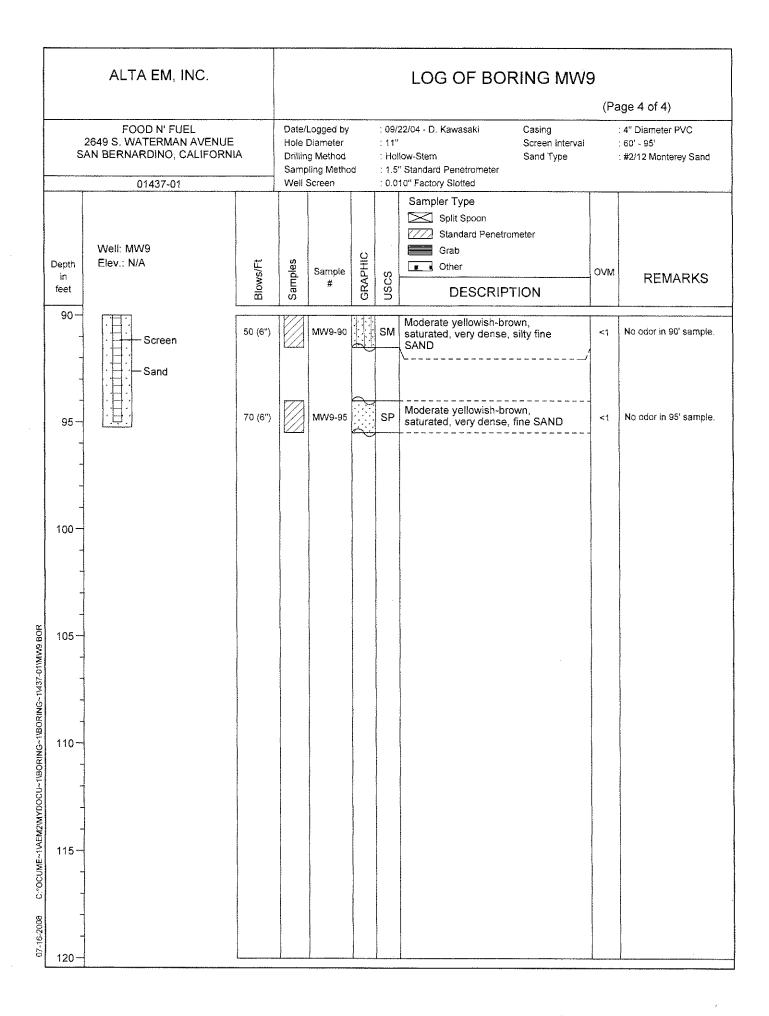


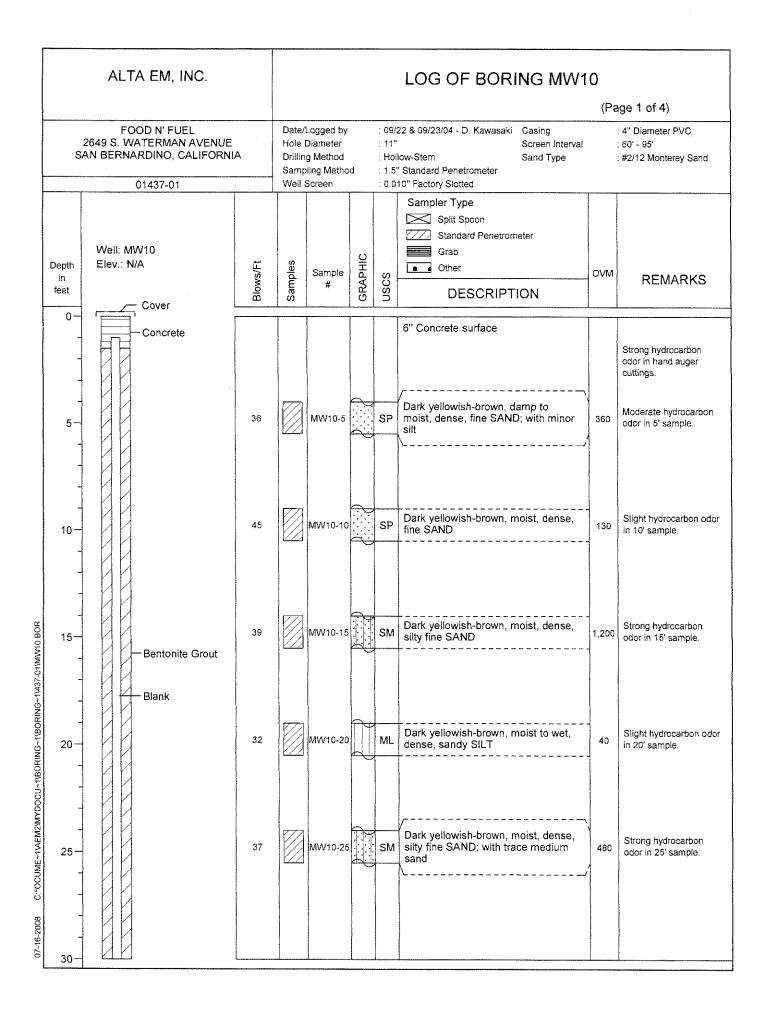
	-	ALTA EM, INC.						LOG OF BORING MW	8	
	s,	FOOD N' FUEL 2649 S. WATERMAN AVENUE AN BERNARDINO, CALIFORNI 01437-01	A	Hole Drillin Samp	Logged by Diameter g Method bling Metho Screen		: 11" : Hol : 1.5	21/04 - D. Kawasaki Casing Screen Interval low-Stem Sand Type ' Standard Penetrometer 10'' Factory Slotted		age 4 of 4) : 4" Diameter PVC : 60' - 95' : #2/12 Monterey Sand
	Depth in feet	Well: MW8 Elev.; N/A	Blows/Ft	Samples	Sample #	GRAPHIC	uscs	Sampler Type Split Spoon Standard Penetrometer Grab Other DESCRIPTION	_ OVM	REMARKS
	90	Screen Screen Sand	80		MW8-90		SP	Greyish-olive, saturated, very dense, fine SAND	<1	No odor in 90' sample.
	95 -		80		MW8-95		SM	Greyish-olive, saturated, very dense, silty fine SAND		No odor in 95' sample.
						and a second second second second second second second second second second second second second second second			and a second and a second and a second and a second and a second and a second and a second and a second and a s	
-1437-011MW8.BOR										
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08 C.*OCUME~11AEM21MYDOCU~11BORING~11BORING~14437-011MW8.BOR	1 1									
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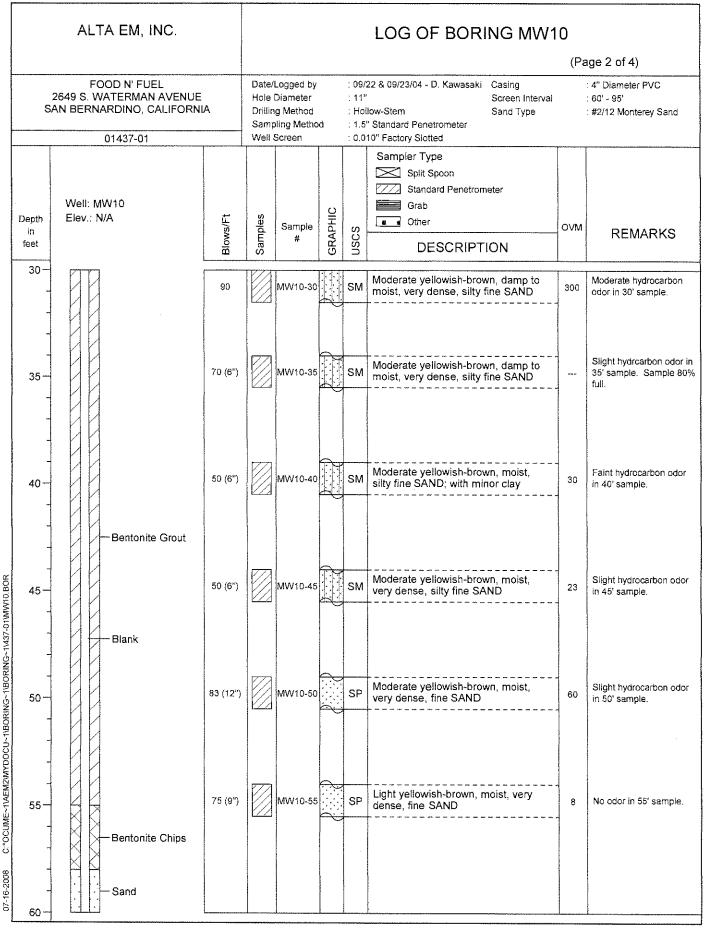




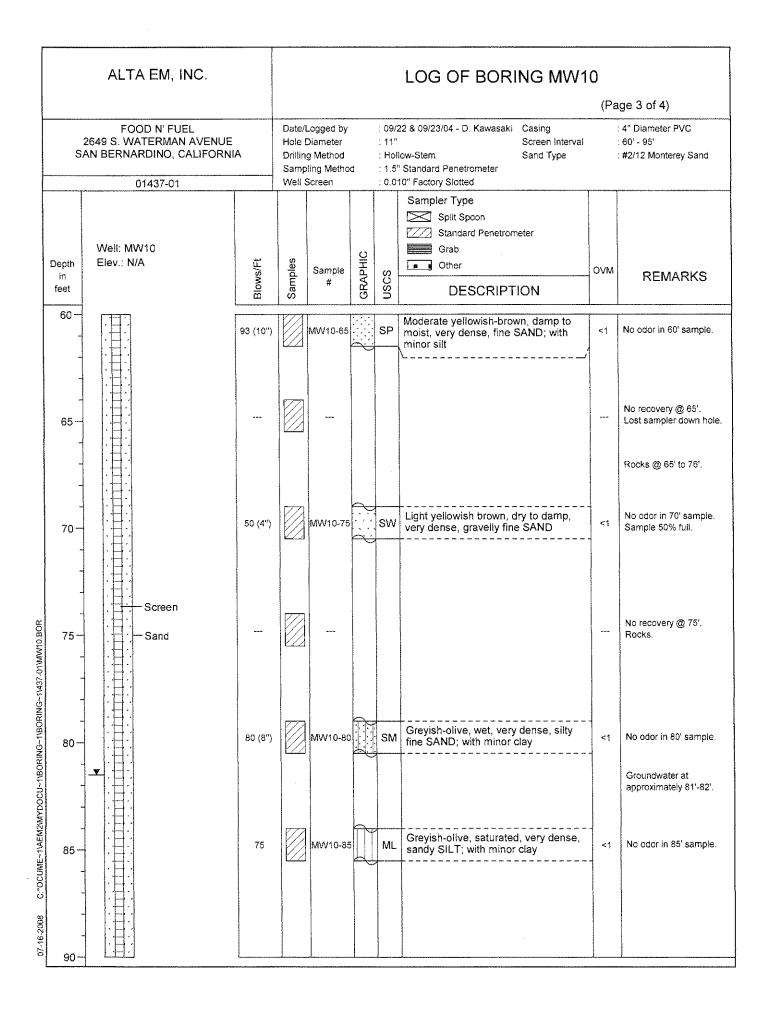


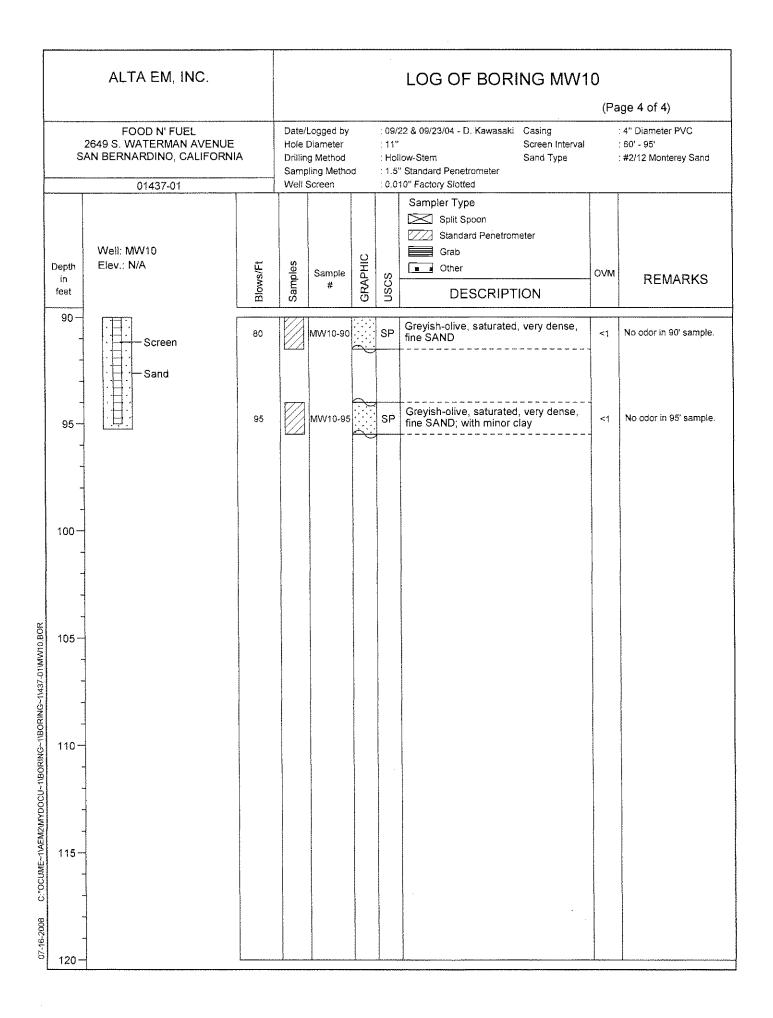


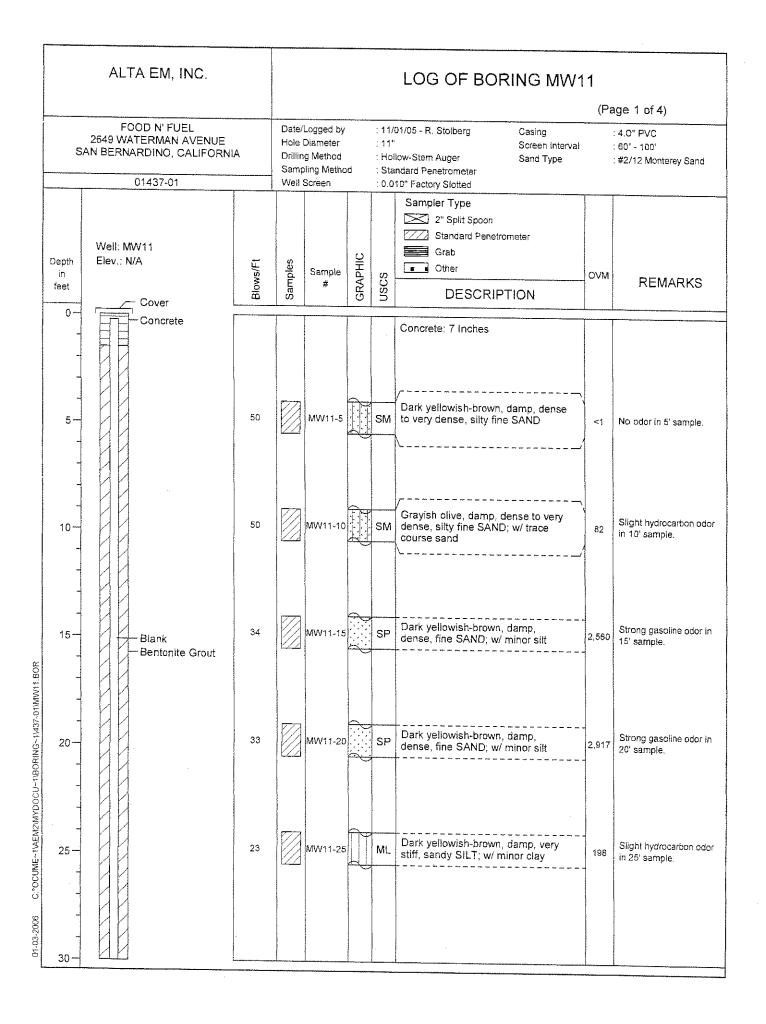


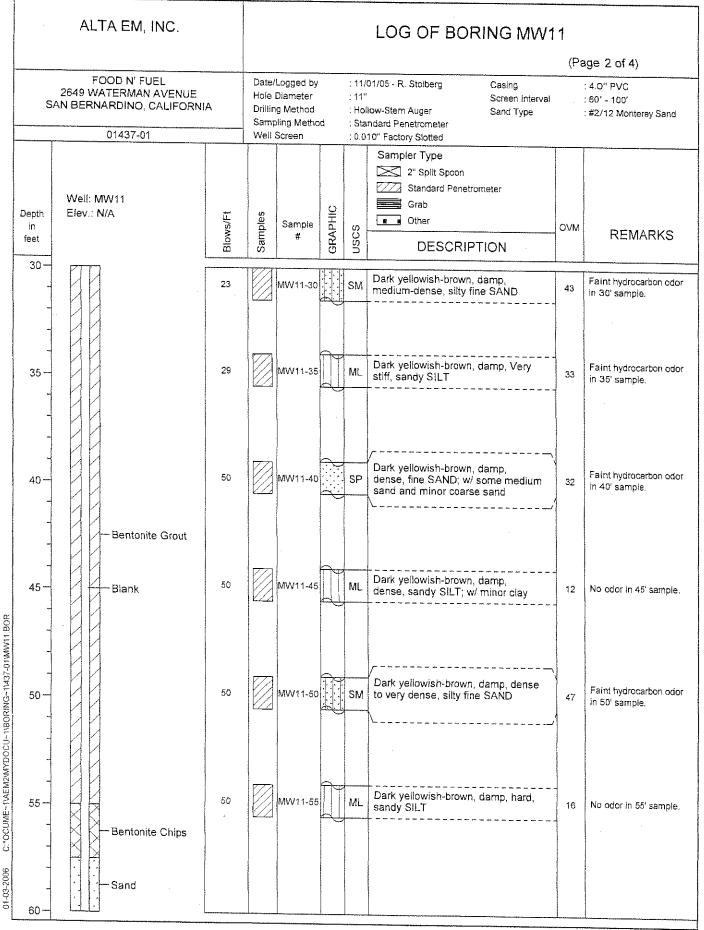


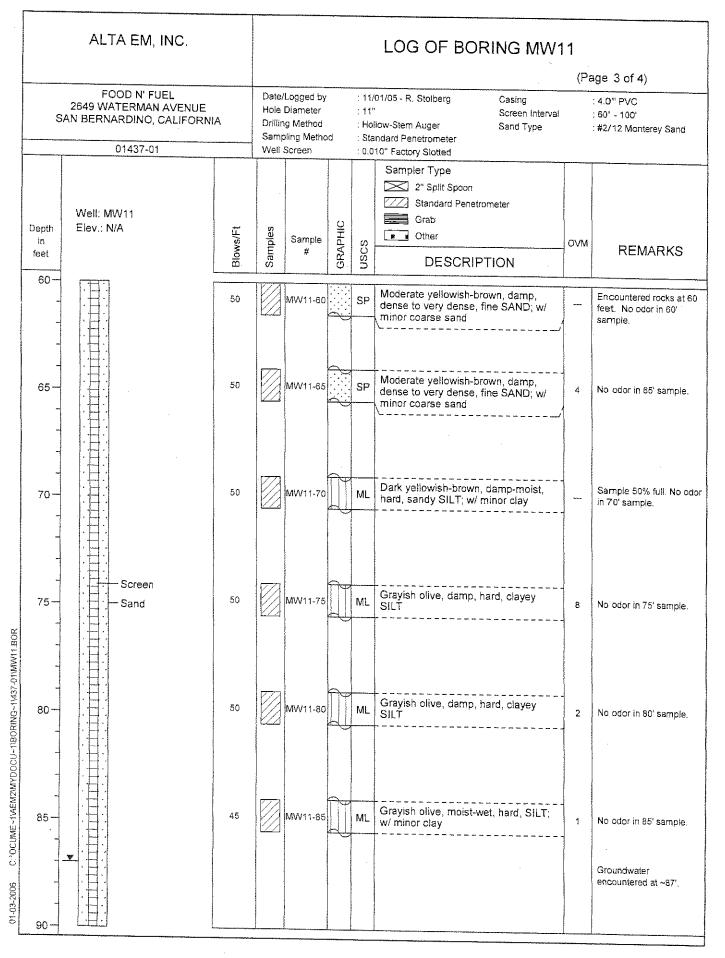
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S	FOOD N' FUEL 2649 WATERMAN AVENUI AN BERNARDINO, CALIFOR 01437-01	Date/Logged by Hole Diameter Drilling Method Sampling Method Well Screen			: 11/01/05 - R. Stolberg Casing : 4.0" PVC : 11" Screen Interval : 60' - 100' : Hollow-Stem Auger Sand Type :#2/12 Monterey Sa : Standard Penetrometer : 0.010" Factory Slotted							
Depth in feet	Well: MW11 Elev.: N/A	Blows/Ft	Samples	Sampie #	GRAPHIC	uscs	Sampler Type 2" Split Spoon CCC Standard Penetron Grab CTC Other DESCRIPT		ovm	REMARKS		
90		50		MW11-90		SM	Dark yellowish-brown, s dense to very dense, si SAND	paturated, Ity fine	<1	No odor in 90' sample.		
95	Screen	46		MVV11-95		SM	Dark yellowish-brown, s dense, silty fine SAND	aturated,		No odor in 95' sample.		
100-		50		MW11-100		SM	Dark yellowish-brown, s dense to very dense, si SAND	aturated, Ity fine	<1	No odor in 100' sample		
105 -												
110												
115												